

Mtg 28: Fri, 31 Oct 08. EAS 4200c (28-1)

With 4 zero stress comp. on p. 24-2
obtain Eq. (2) on p. 27-1

HWS: Do the other 2 eqs of equil, i.e.,
 $j = 2, 3$ in Eq. (4), p. 27-2.

Derivation of Eq. (4), p. 27-2. (3-D case)
Do 1-D model pb 1st.

$$\text{p. 24-3: } \Sigma F_x = 0 = -\sigma(x)A + \sigma(x+dx)A + f(x)dx$$

$$\Rightarrow 0 = A [\sigma(x+dx) - \sigma(x)] + f(x)dx$$

Taylor Series \longrightarrow

$$\frac{d\sigma(x)}{dx}$$

dx

+ h.o.t.

higher order terms

Recall: $f(x+dx) = f(x) + \frac{df(x)}{dx}dx + \frac{1}{2} \frac{d^2f(x)}{dx^2}(dx)^2 + \dots$

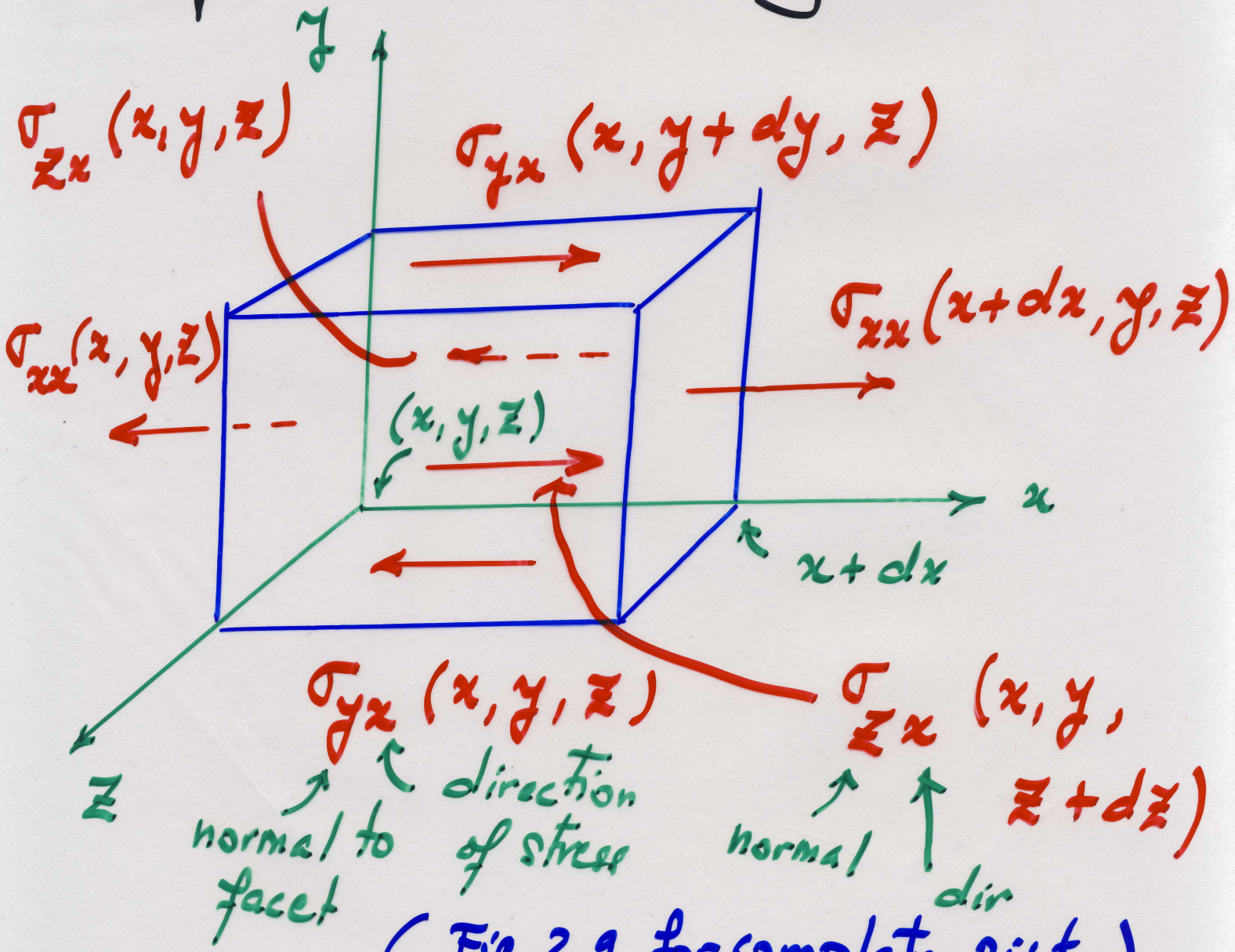
Now, neglect h.o.t.

$$\Rightarrow \frac{d\sigma}{dx} + \frac{f(x)}{A} = 0$$

body force = force/vol. (Tared)

$f(x)$ = force/length. \uparrow applied load.

Now, non uniform stress field in 3-D, but w/o applied load, and focusing on the x dir. only (i.e., w/o the other stress comp. to avoid cluttering fig.)



(Fig. 2.9 for complete pict.)

$$\begin{aligned}
 \Sigma F_x = 0 = & dydz \left[-\sigma_{xx}(x, y, z) \right. && \text{(28-3)} \\
 & \left. + \sigma_{xx}(x+dx, y, z) \right] && \text{facets w/ normal } x \\
 + dzdx \left[-\sigma_{yx}(x, y, z) + \right. && \left. \sigma_{yx}(x, y+dy, z) \right] && \left. \begin{array}{l} \text{facets} \\ \text{w/ normal} \\ y \end{array} \right\} \\
 + dxdy \left[-\sigma_{zx}(x, y, z) \right. && \left. + \sigma_{zx}(x, y, z+dz) \right] && \left. \begin{array}{l} \text{facets} \\ \text{w/ normal} \\ z \end{array} \right\}
 \end{aligned}$$

$$\Rightarrow 0 = (dxdydz) \left[\frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{yx}}{\partial y} \right.$$

$$\Rightarrow \text{Eq. (3) p. 27-2.} \quad \left. + \frac{\partial \sigma_{zx}}{\partial z} \right]$$

Roadmap: p. 16-2

D. Prandtl stress function ϕ .

Mtg 29: Mon, 3 Nov 08

29-1

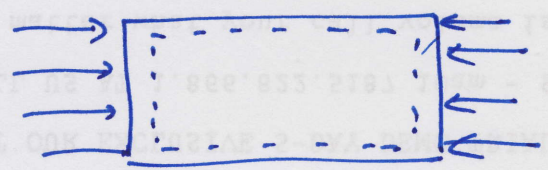
⇒ Plate buckling + dimen. Anal. + HW6.

- Dimensional analysis for eq. of equil in terms of stresses

- Continue elasticity theory for torsional Anal

Roark's, formula for stress and strain, 7th ed., Young & Budynas, p. 736:

$$\sigma' = K \frac{E}{1-\nu^2} \left(\frac{t}{b}\right)^2$$



For $\frac{a}{b} = 1 \Rightarrow K = 3.29$.

MIT: + my wiki:

$$\sigma_{cr} = k_c \frac{\pi^2 D}{b^2 h} = \left(\frac{mb}{a} + \frac{a}{mb}\right)^2 \pi^2$$

$$\left(\frac{1}{b^2 h}\right) \left(\frac{E h^3}{1-\nu^2}\right)$$

$$= \left(\frac{mb}{a} + \frac{a}{mb}\right)^2 \frac{\pi^2}{12} \left(\frac{E}{1-\nu^2}\right) \left(\frac{h}{b}\right)^2$$

$$\text{For } \frac{a}{b} = 1, m=1 \Rightarrow \underbrace{\frac{(1+1)^2 \pi^2}{12}}_{\frac{\pi^2}{3}} = K = 3.29$$

Mtg 28: Mon, 3 Nov 08. EAS 4200c (28-1)

Eq. of equil (cont'd)

Recall 1-D case p. 28-2 : Dimen. Anal.

$$\left. \begin{aligned} [f] &= \frac{F}{L} \quad \left\{ \begin{array}{l} \text{Force} \\ \text{length} \end{array} \right. \\ [A] &= L^2 \end{aligned} \right\} \left[\frac{f}{A} \right] = \frac{F}{L^3}$$

"Dimen. of"

$$[\sigma] = \frac{F}{L^2} \Rightarrow \left[\frac{d\sigma}{dx} \right] = \frac{[d\sigma]}{[dx]} = \frac{F/L^2}{L}$$

$$[dx] = L = \frac{F}{L^3}$$

force / vol
or body force.

$$\left. \begin{aligned} \epsilon &= \frac{du}{dx} \\ \epsilon &= \frac{\Delta L}{L} \end{aligned} \right\} [\epsilon] = \frac{[du]}{[dx]} = \frac{L}{L} = 1 \text{ non-dim}$$

Stress

$$\nu = - \frac{\epsilon_{yy}}{\epsilon_{xx}} \Rightarrow [\nu] = \frac{[\epsilon_{yy}]}{[\epsilon_{xx}]} = 1 \text{ Jash}$$

HW6: Plate buckling wiki page.

Mtg 30: Wed, 5 Nov 08. EAS 4200c (30-1)

Torsional Anal. Cont'd.

$$\left. \begin{array}{l} \text{p. 27-2} \\ \text{p. 29-1} \end{array} \right\} \left[\frac{\partial \sigma_{ij}}{\partial x_i} \right] = \frac{F}{L^3} = \text{force/vol.}$$

HW6: Read & Report (R&R)

(3.14) p. 27-1

Roadmap p. 16-2: D. Prandtl stress func. ϕ

$$(1) \quad \sigma_{yz} = \frac{\partial \phi}{\partial z}, \quad \sigma_{zx} = -\frac{\partial \phi}{\partial y} \quad \text{cf. (3.15)}$$

ϕ plays role of a potential func.,
(σ_{yz}, σ_{zx}) comp. of "gradient" of ϕ
wrt (y, z).

Recall: Scalar func. $f(x, y, z)$

$$\text{grad } f(x, y, z) = \frac{\partial f}{\partial x} \vec{i} + \frac{\partial f}{\partial y} \vec{j}$$

a vector

$$(Math) \quad f = \text{potential func.} + \frac{\partial f}{\partial z} \vec{k} \equiv$$

Eq. (1) on p. 30-1 satisfies automatically - (30.2)
Eq. (1) p. 27-1.

Math:

$$\frac{\partial}{\partial y} \left(\frac{\partial \phi}{\partial x} \right) + \frac{\partial}{\partial z} \left(- \frac{\partial \phi}{\partial y} \right)$$

$\underbrace{\hspace{10em}}_{\sigma_{yx}} \qquad \qquad \qquad \underbrace{\hspace{10em}}_{\sigma_{zx}}$

$$= \frac{\partial^2 \phi}{\partial y \partial z} - \frac{\partial^2 \phi}{\partial z \partial y} = 0, \text{ since}$$

ϕ cont. & smooth \Rightarrow 2nd mixed deriv. interchangeable.

i.e.,
$$\frac{\partial^2 \phi}{\partial y \partial z} = \frac{\partial^2 \phi}{\partial z \partial y}$$

HW6: R & R (3.15) \rightarrow (3.19)

$$\frac{\partial^2 \phi}{\partial y^2} + \frac{\partial^2 \phi}{\partial z^2} = -2G\theta$$

$\nabla^2 \phi$ (Laplacian of ϕ)

R & R until end of Sec. 3.2.

Need to explain (derive) : (30-3)

(2.30) $\{t\} = [\sigma] \{n\}$ (eq. below (3.19) Chap. 2)

$\underbrace{\quad}_{3 \times 1}$ $\underbrace{\quad}_{3 \times 3}$ $\underbrace{\quad}_{3 \times 1}$

comp. of traction force \vec{t} comp. of stress tensor comp. of normal vector \vec{n}

2-D case first: \vec{t} p. 29 \vec{n} (normal)

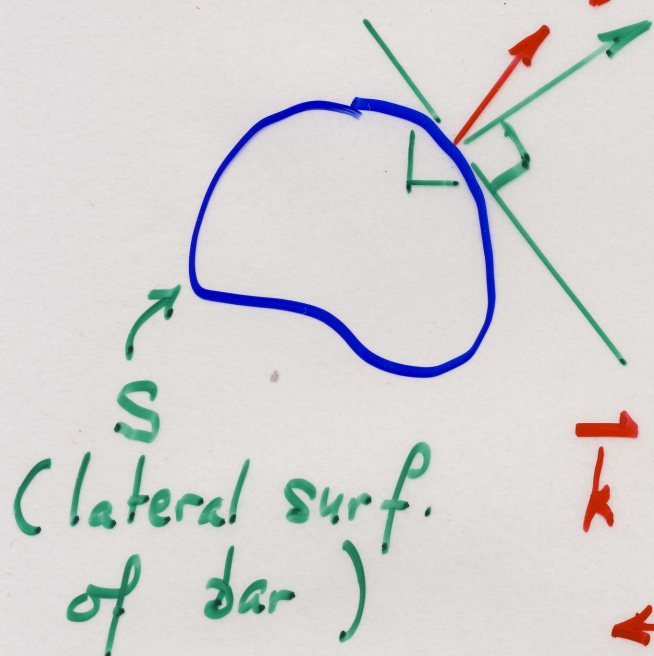
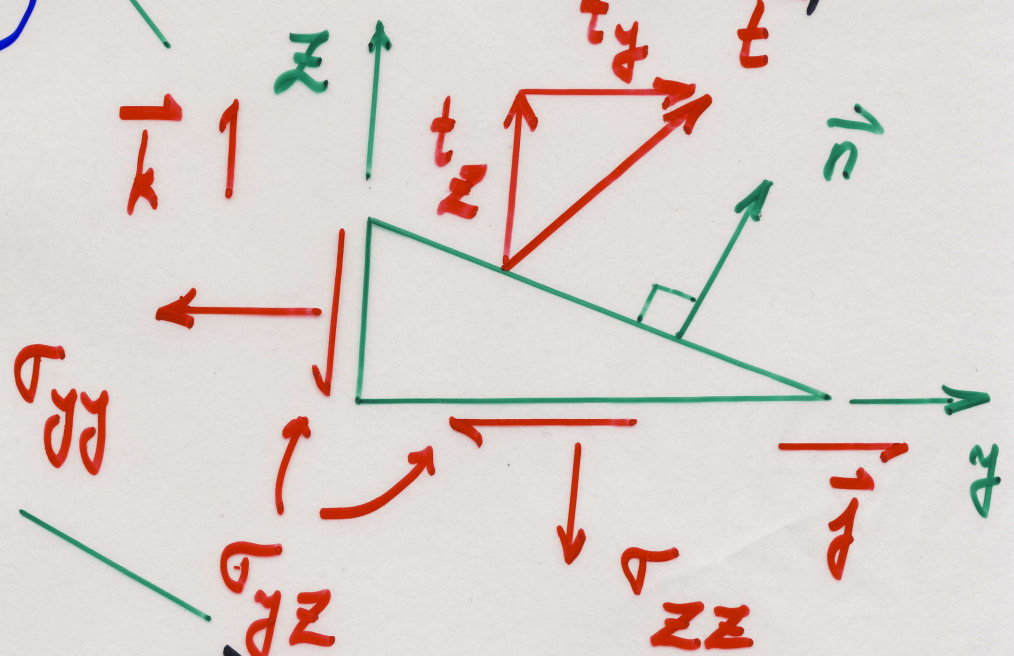
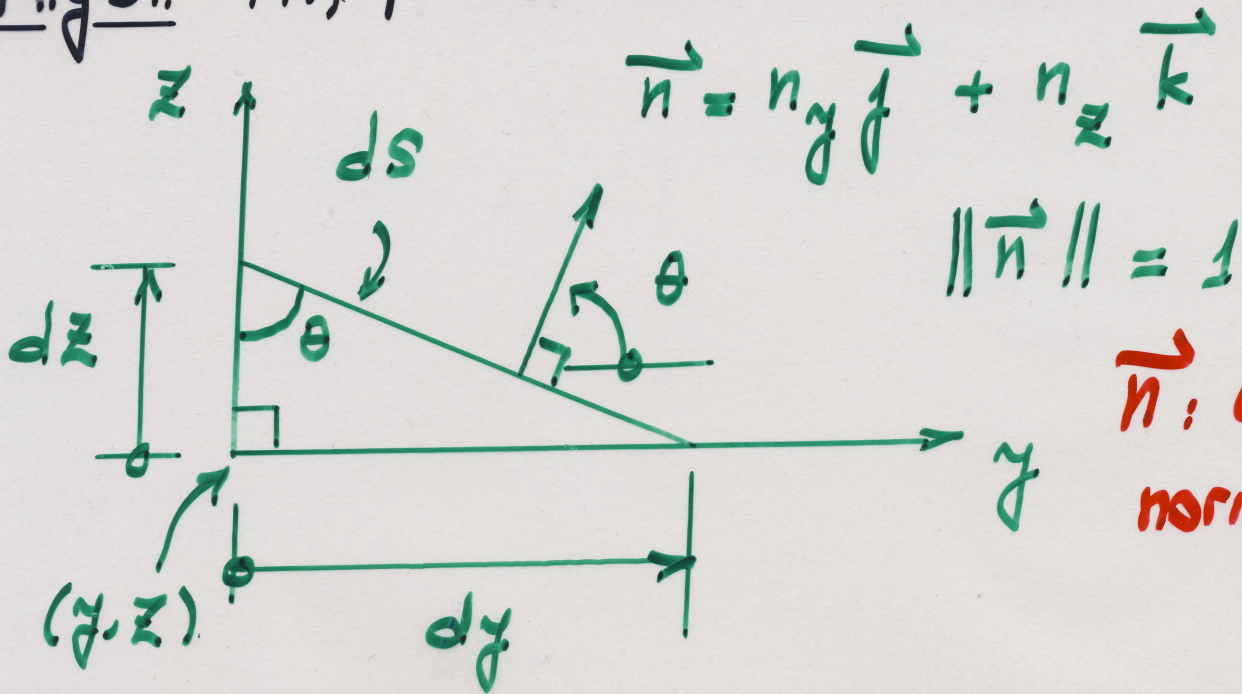


Fig. 26-1: Example of \vec{t} in aircraft.



$$\vec{t} = t_y \vec{j} + t_z \vec{k}$$

Mtg 31: Fri, 7 Nov 08. EAS 4200c (31-1)



$$\begin{cases} dz = ds \cdot \cos \theta \\ dy = ds \cdot \sin \theta \end{cases} \quad \left| \quad \begin{cases} n_y = \cos \theta \\ n_z = \sin \theta \end{cases} \right.$$

$$\sum F_y = 0 = -\sigma_{yy} \cdot (dz \cdot 1)$$

↑ unit depth along x axis

$$- \tau_{yz} \cdot (dy \cdot 1)$$

$$+ t_y \cdot (ds \cdot 1)$$

$$\Rightarrow 0 = -\sigma_{yy} \cdot \cancel{ds} \cdot n_y - \tau_{yz} \cdot \cancel{ds} \cdot n_z + t_y \cdot \cancel{ds}$$

$$\Rightarrow \boxed{t_y = \sigma_{yy} \cdot n_y + \tau_{yz} \cdot n_z} \quad (1)$$

Note: $[t_y] = \frac{F}{L^2}$ force/area |31-2

\vec{F} : traction vector (distr. surf. force)

$$[t_y] = [\tau]$$



$$\sum F_{xz} = 0 \quad \text{HW6}$$

$$\Rightarrow t_{xz} = \sigma_{yz} \cdot n_y + \sigma_{zz} \cdot n_z \quad (2)$$

(1) & (2):

$$\begin{Bmatrix} t_x \\ t_y \\ t_z \end{Bmatrix} = \begin{bmatrix} \sigma_{xx} & \sigma_{xy} & \sigma_{xz} \\ \sigma_{xy} & \sigma_{yy} & \sigma_{yz} \\ \sigma_{xz} & \sigma_{yz} & \sigma_{zz} \end{bmatrix} \begin{Bmatrix} n_x \\ n_y \\ n_z \end{Bmatrix} \quad (3)$$

Generalization to 3D:

$$\begin{Bmatrix} t_1 \\ t_2 \\ t_3 \end{Bmatrix} = \begin{bmatrix} \sigma_{11} & \sigma_{12} & \sigma_{13} \\ \sigma_{21} & \sigma_{22} & \sigma_{23} \\ \sigma_{31} & \sigma_{32} & \sigma_{33} \end{bmatrix} \begin{Bmatrix} n_1 \\ n_2 \\ n_3 \end{Bmatrix} \quad (4)$$

$$t_i = \sum_{j=1}^3 \sigma_{ij} n_j, \quad i = 1, 2, 3 \quad \underline{(3.1-2)}$$

↑ row ↑ col.

$$\Rightarrow \left\{ \begin{array}{l} \{t_i\} \\ 3 \times 1 \\ \text{Col. mat} \end{array} \right. = \left[\begin{array}{l} [\sigma_{ij}] \\ 3 \times 3 \\ \text{Col. mat.} \end{array} \right] \left\{ \begin{array}{l} \{n_j\} \\ 3 \times 1 \end{array} \right.$$

Note: row mat.

$$\{t_i\}^T = \left\{ \begin{array}{l} t_1 \\ t_2 \\ t_3 \end{array} \right\}^T = [t_1 \quad t_2 \quad t_3] \quad 1 \times 3$$

Roadmap p. 16-2:

G. Bound. cond. for ϕ (3.24)

$\phi = \text{const}$ on lateral surf. of the bar.

↑ (3.24a)

- Rube-Goldberg device: How to run an air plane; Quotations (inspiring); Narrative
 - Best of HWS: Team VQ Crew's narrative of computed results
 - Plate buckling wiki page: (Sun Sec 7.7)
 - * continue to improve / add material
 - * k_c^* , m^*
 - * clamped rect. plate: Roark's formula
- HW 6: Verify for SS plate, K for $\frac{a}{b} = 1$ and $m = 1$.

- Torsional analysis of bar w/ circular cross section (Sec. 3.3)
 - * no warpy: HW 6.

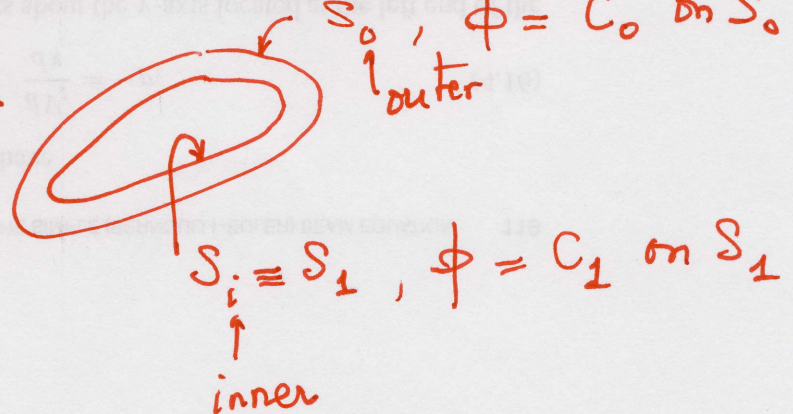
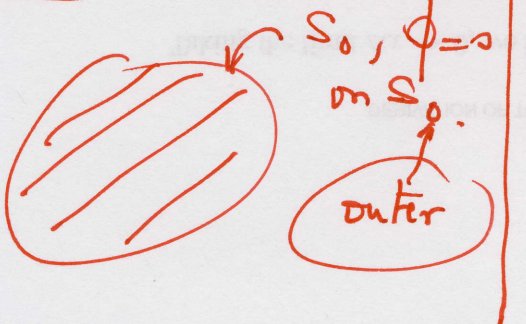
Roadmap p. 16-2:

G. Bound. cond. for ϕ

Case 1: Thin-walled cross section

S_0 , $\phi = C_0$ on S_0
↑
outer

Case 2: Solid cross section



- Rube-Goldberg device

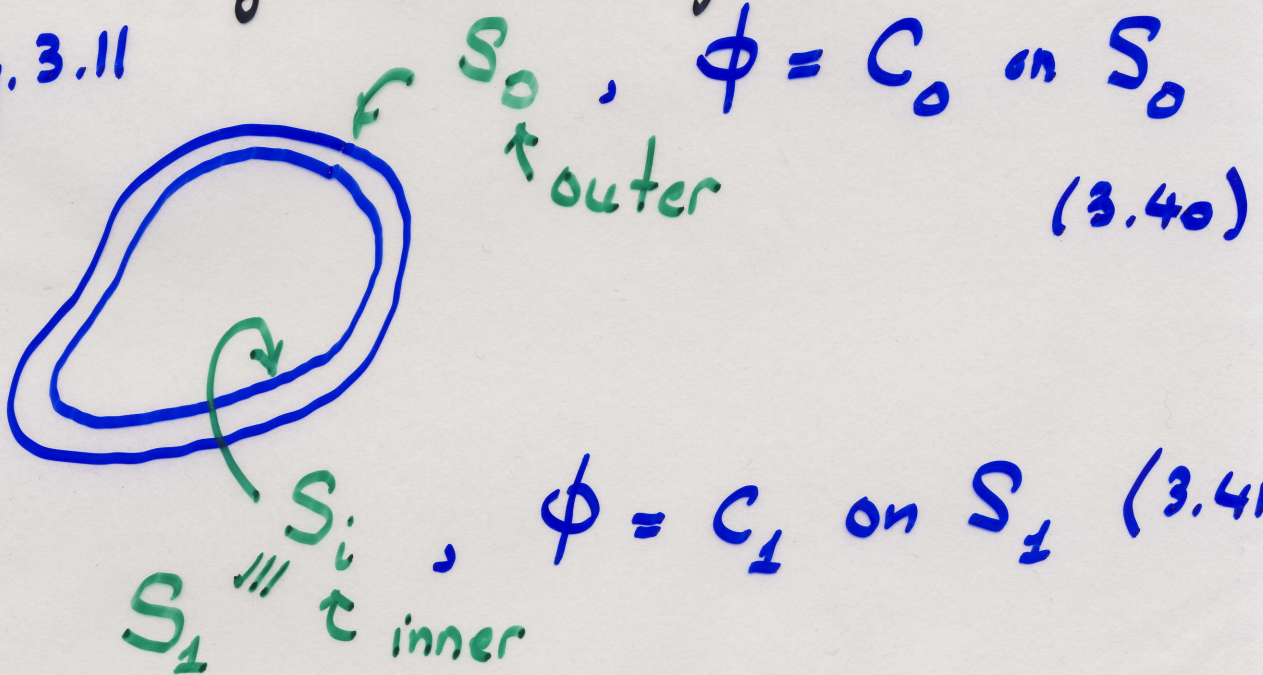
- Plate buckling: cont'd

HW6: Find rel. betw. K and k_c
Verify K for $a/b = 1, m = 1$.

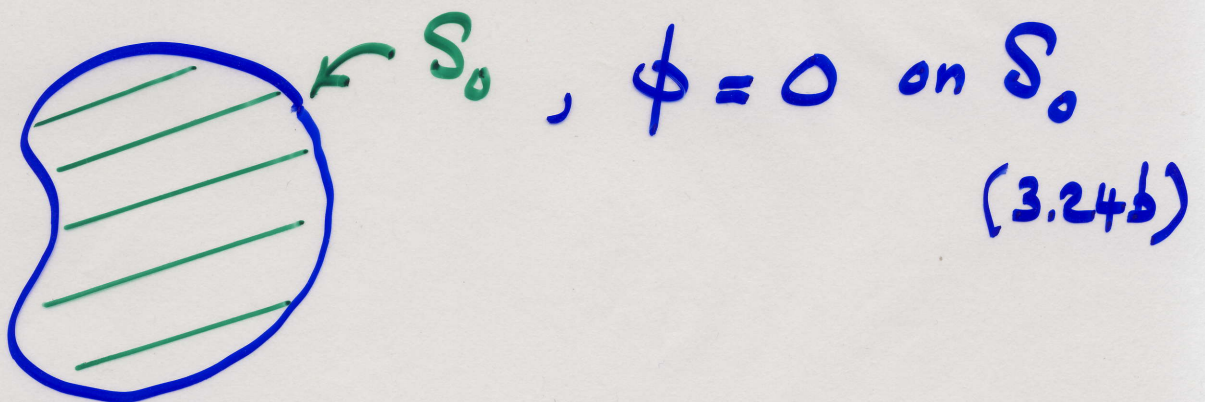
Torsional Anal: cont'd p. 31-3

Case 1: Thin-walled cross section (closed)
e.g. NACA airfoil

cf. Fig. 3.11



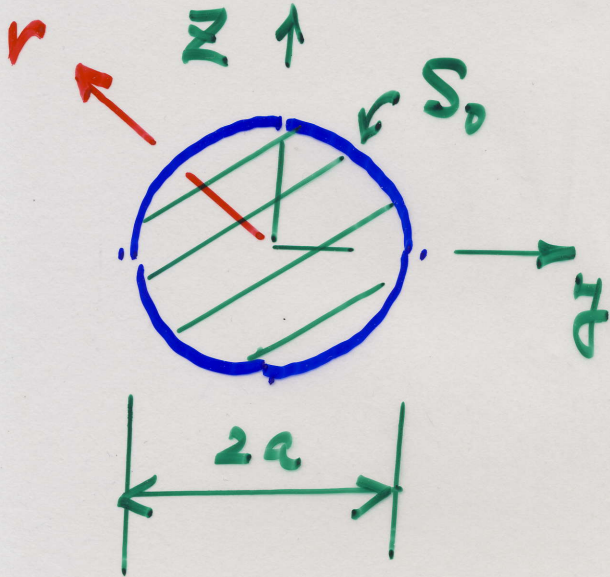
Case 2: Solid cross section



Uniform bar w/ solid, circular cross sect (32-2)
 p. 16-3: Roadmap, expr. for (Sec 3.3)
 T and J in terms of ϕ .

$$\phi(x, z) = C \left(\frac{x^2}{a^2} + \frac{z^2}{b^2} - 1 \right)$$

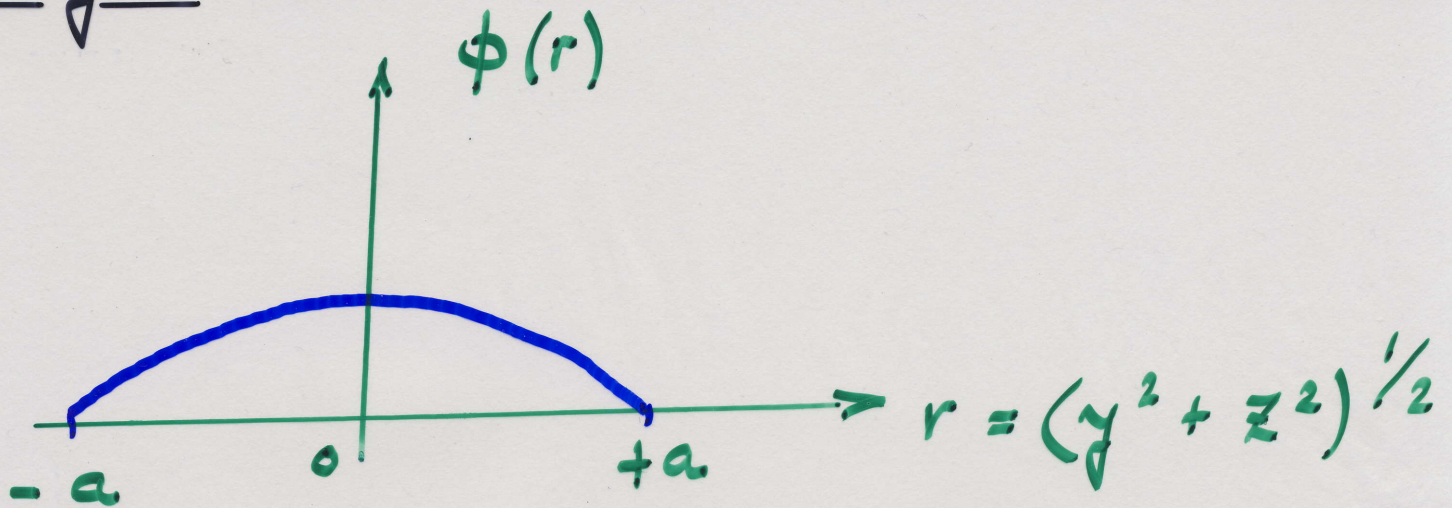
($b=a$) (circle)



$$\Rightarrow \phi = 0 \text{ on } S_0 \quad \text{HW6}$$

$$\Rightarrow \nabla^2 \phi = -2G\theta$$

$$\Rightarrow C = -\frac{1}{2} a^2 G\theta$$



$$T = 2 \int_A \phi \, dA = 2C \left(\frac{J}{a^2} - \frac{A}{\pi a^2} \right)$$

$$J = \int_A r^2 \, dA = \frac{1}{2} \pi a^4$$

$$\Rightarrow \boxed{T = GJ\theta} \quad (3.28)$$

$$\left\{ \begin{array}{l} \sigma_{yz} = \frac{\partial \phi}{\partial z} = -G\theta z \end{array} \right. \quad (3.29)$$

$$\left\{ \begin{array}{l} \sigma_{zx} = -\frac{\partial \phi}{\partial y} = G\theta y \end{array} \right. \quad (3.30)$$

R + R thru end See 3.3.

$$\text{Also: } \tau = \frac{Tr}{J} \quad p. 14.2$$

Using (3.16), (3.29), (3.30) and (3.3-2)
 $\sigma - \epsilon$ rel., show that $u_x(y, z) = 0$

i.e., no warping, (ad hoc assump. p.14-2)

Hint:

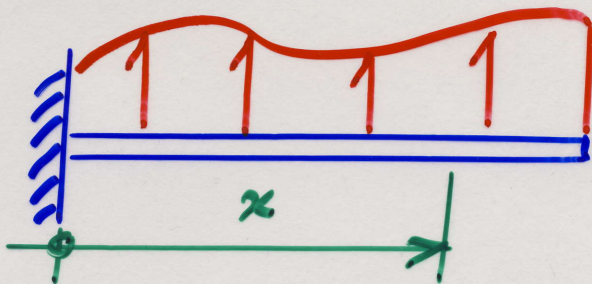
$$(3.16ab) \quad \gamma_{yx} = \frac{\sigma_{yx}}{G} = \frac{\partial u_x}{\partial y} - \theta z$$

$$\gamma_{zx} = \frac{\sigma_{zx}}{G} = \frac{\partial u_x}{\partial z} + \theta y$$

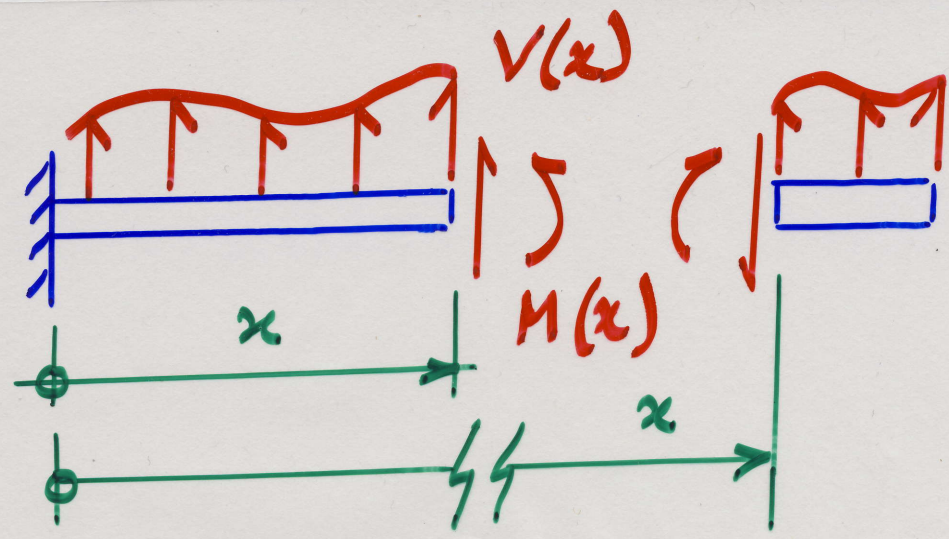
$$(3.29) \quad \sigma_{yx} = -G\theta z$$

$$(3.30) \quad \sigma_{zx} = G\theta y$$

Flexural Shear flow in thin-walled Sections
(Chap. 5) 5.1, 5.1.1, 5.1.2, 5.1.3, 5.3, 5.3.1,
5.3.2, 5.4



p. 26-1



- HW5: Effect of $M(x)$ on NACA air foil
- HW6: Plate buckling, compressive in-plane load
NACA air foil
- HW7: Effects of $V(x)$ on NACA air foil
Plate buckling, Shear load.

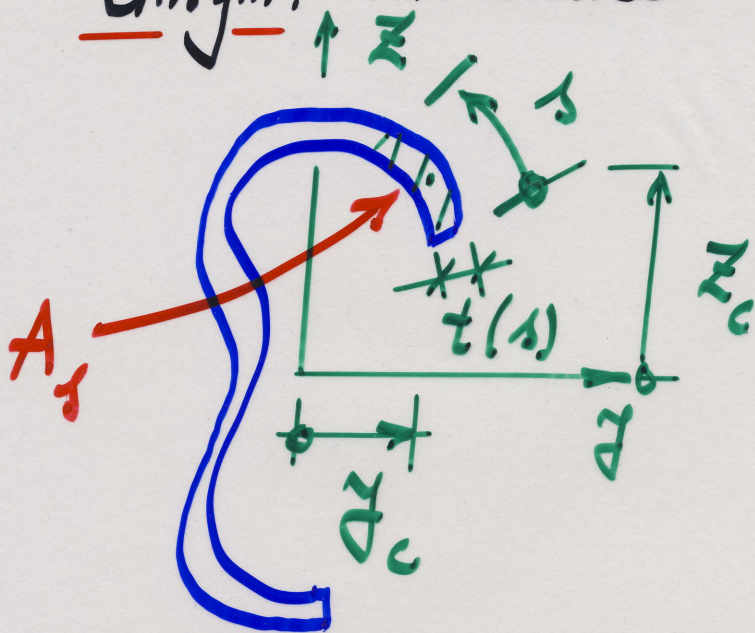
Mtg 34: Fri, 14 Nov 08. EAS 4200c

34-1

Secs 5.1, 5.1.1, 5.1.2

Unsym. thin walled cross sect.

$q(s)$



$$\int_{A_s} \frac{d\sigma_{xx}}{dx} dA = -\frac{q}{t_s} \quad (5.1)$$

general for unsym. cross sect.

Sym. about y axis

$$\sigma_{xx} = \frac{M_y z}{I_y}$$

$$q(s) = -\frac{V_z Q_z}{I_y} \quad (5.2)$$

$$Q_z = \int_{A_s} z dA = z_c A_s$$

Unsym. cross sect.

$$\sigma_{xx} = (k_y M_z - k_{yz} M_y) y + (k_z M_y - k_{yz} M_z) z \quad (5.3)$$

$$k_y = \frac{I_y}{D} \quad p. 26-2$$

$$k_{yz} = I_{yz} / D$$

$$k_z = I_z / D$$

$$\sigma_{xx} = \underbrace{L}_{1 \times 1} \underbrace{\begin{bmatrix} y & z \\ z & y \end{bmatrix}}_{2 \times 2} \underbrace{\begin{bmatrix} k_y & -k_{yz} \\ -k_{yz} & k_z \end{bmatrix}}_{2 \times 2} \underbrace{\begin{Bmatrix} M_z \\ M_y \end{Bmatrix}}_{2 \times 1} \quad (34-2)$$

$$\begin{Bmatrix} k_y M_z - k_{yz} M_y \\ -k_{yz} M_z + k_z M_y \end{Bmatrix} \quad 2 \times 1$$

$$\sigma_{xx} = \underbrace{L}_{1 \times 1} \underbrace{\begin{bmatrix} z & y \\ y & z \end{bmatrix}}_{2 \times 2} \underbrace{\begin{bmatrix} k_z & -k_{yz} \\ -k_{yz} & k_y \end{bmatrix}}_{2 \times 2} \underbrace{\begin{Bmatrix} M_y \\ M_z \end{Bmatrix}}_{2 \times 1}$$

Particularize to sym. cross sect. $I_{yz} = 0$

Consider $M_x = 0$.

Why? HW6

$$I_{yz} = 0 \Rightarrow D = I_y I_z$$

$$k_y = \frac{1}{I_y}, \quad k_{yz} = 0, \quad k_z = \frac{1}{I_z}$$

$$\Rightarrow \sigma_{xx} = \frac{z M_y}{I_y}$$

Sym.

Non-sym. I_y

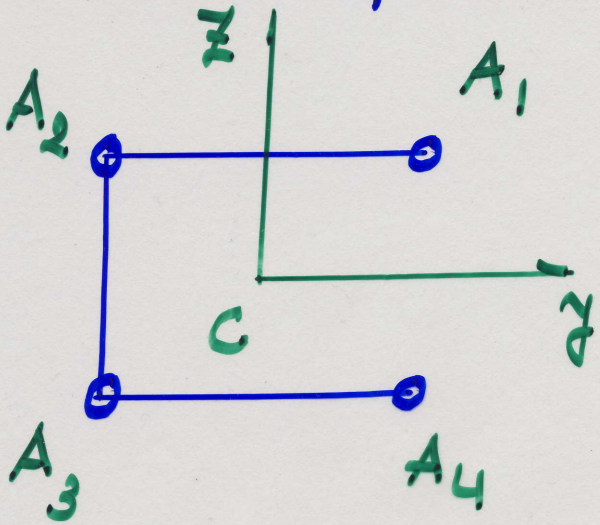
$$q(z) = - (k_y v_y - k_{yz} v_z) Q_z - (k_z v_z - k_{yz} v_y) Q_y \quad (5.5)$$

$$Q_z = \int_{A_s} y dA, \quad Q_y = \int_{A_s} z dA$$

HW6: Put (5.5) in matrix form and recover (5.2) as particular case.

Stringer-web sections: thickness t of skin and spar webs very small \Rightarrow neglect in comp. $I_y, I_z, I_{yz}, Q_y, Q_z$ (Use only areas of stringers).

Sym. wrt y axis
Ex 5.2 p. 154



$$A_3 = A_2$$

$$A_4 = A_1$$

Non-Sym

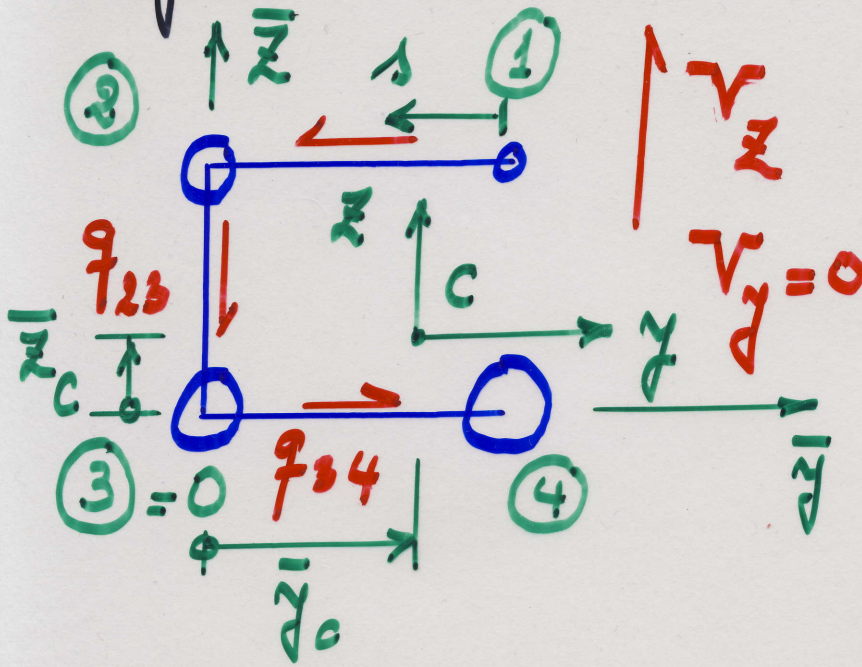
34-4

$$A_1 = A$$

$$A_2 = 2A$$

$$A_3 = 3A$$

$$A_4 = 4A$$



Mean Value Thm
"Average" (MVT)

$$\int_A \bar{y} dA = \bar{y}_c \int_A dA = \bar{y}_c A$$

$$\int_A \bar{z} dA = \bar{z}_c A$$

"Average"

$A = \sum_{i=1}^4 A_i$ (neglect skin + spar webs)
cf. HWS

(5.5) p. 34.3 $q(s) = (k_{yz} Q_z^{(s)} - k_z Q_y^{(s)}) V_z$

- Since :
- 1) V_z
 - 2) k_{yz}, k_z
 - 3) $Q_z^{(s)}, Q_y^{(s)}$
- } indep. of s
} const betw 2 Stringers
- all areas concentr. on stringers.

\Rightarrow shear flow $q(s)$ const. betw. 2 stringers

but $q(s)$ would increment (jump) [35.2]
when crossing a stringer. \rightarrow and (\bar{y}_i, \bar{z}_i) ,

Step 1: Find (\bar{y}_c, \bar{z}_c) HW 7 $i=1, \dots, 4$

Step 2: Find I_y, I_z, I_{yz} (Gabriel)

Step 3: Find k_y, k_z, k_{yz}

Step 4: Follow path "s" to find

$$q_{12}, q_{23}, q_{34}$$

Shear flow in
skin panel 12

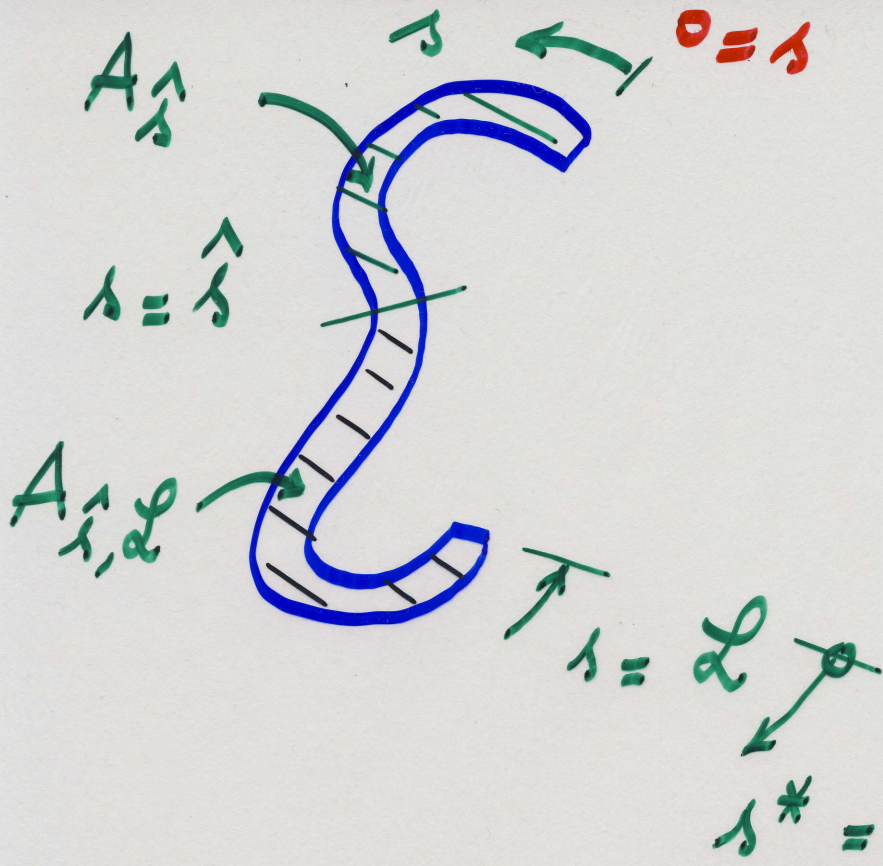
$$q_{12} = (k_{yz} Q_z^{12} - k_z Q_y^{12}) V_z$$

$$Q_z^{12} = \bar{y}_1 A_1$$

\uparrow y coord. of stringer 1

$$Q_y^{12} = \bar{z}_1 A_1$$

Note: $Q_z^{23} = \bar{y}_1 A_1 + \bar{y}_2 A_2$



$$A = A_{\hat{s}} + A_{\hat{s}, \alpha}$$

$\underbrace{\hspace{2em}}_{=}$
 $A_{0, \hat{s}}$

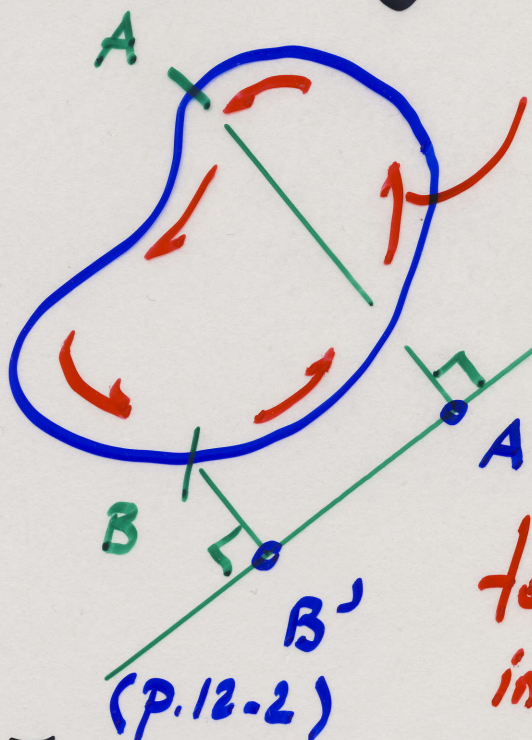
(jared)

To be cont'd soon

- Mini plan: S) Single-cell sections
 S.1) Without stringers
 S.2) w/ stringers

- M) Multi-cell sections
 M.1) w/o stringers
 M.2) w/ stringers

- S) Single-cell
 S.1) w/o stringers



$q = \text{const. shear flow}$
 \underline{Q} : Can this set up resist V_z ?

$$R^z = R_{AB}^z + R_{BA}^z$$

↑ result. of q in AB ↑ result. of q in BA

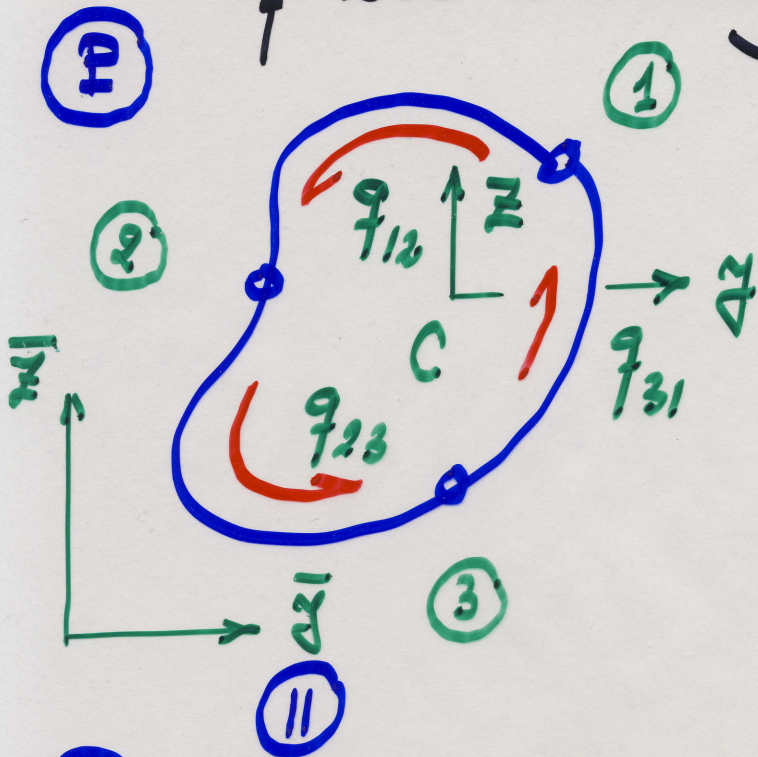
total result. in z dir.

(p.12-2)

$$R_{AB}^z = -q \quad \overline{A'B'} = -R_{BA}^z$$

$$\Rightarrow R^z = 0$$

S.2) w/ stringers (neglect contrib. $\underline{L36-2}$ of web to bending.)

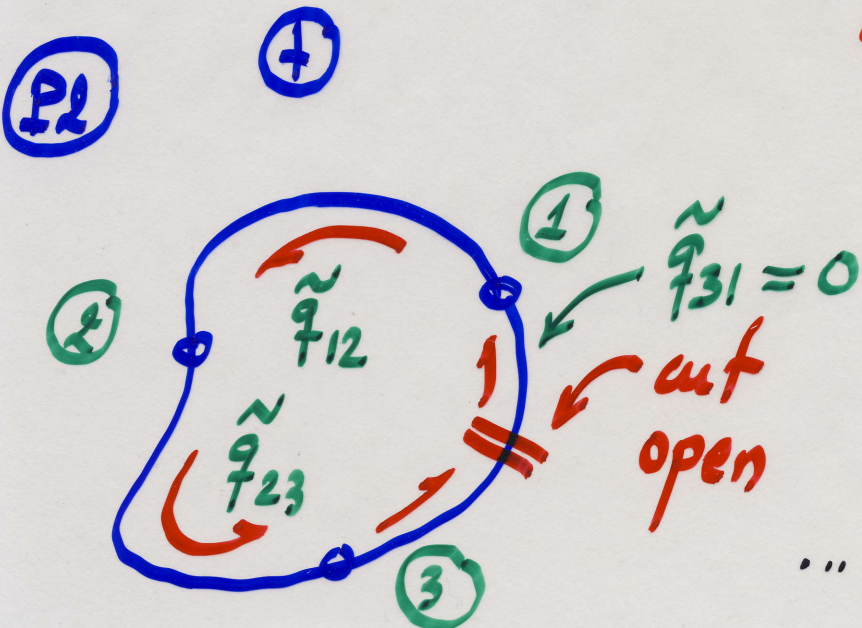
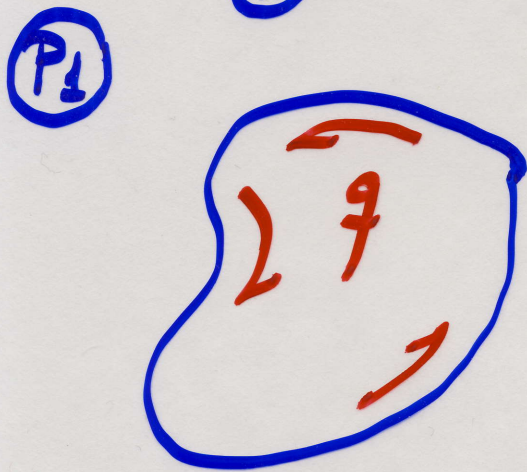


$q_{12} \neq q_{23} \neq q_{31}$
 but $q_{ij}(s) = \text{const}$
 within each panel.
 $\Rightarrow q(s)$ is piecewise const. wrt s

$$R^z = V_z \neq 0$$

Presence of stringers
 \Rightarrow non const shear flow

Princ. of Superposition
 due to linearity



$$q_{12} = q + \tilde{q}_{12}$$

$$q_{23} = q + \tilde{q}_{23}$$

$$\dots \quad q_{ij} = q + \tilde{q}_{ij}$$

Anal. Algo:

obs: One unknown q (\tilde{q}_{ij} are known after solving P_2) \Rightarrow need 1 eq for 1 unknown.

Method: Data: V_y, V_z

1) Solve P_2 for $\tilde{q}_{12}, \tilde{q}_{23}$ ($\tilde{q}_{31} = 0$)

2) Moment eq. : Take mom. about any pt in plane (y, z)

2.1) Superposition: $q_{ij} = q + \tilde{q}_{ij}$
known from 1)

2.2) Select pt \bar{a} in plane (y, z)

$$\sum_{\bar{a}} \text{Mom of } (V_y, V_z)$$

$$= \sum_{\bar{a}} \text{Mom of } q_{ij}$$

(1 eq. for 1 unknown q)