

Handbook for Agrohydrology (NRI)

➔ Chapter 3: Erosion and sedimentation data

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 Appendix B: Erosion and sedimentation data

Handbook for Agrohydrology (NRI)

Chapter 3: Erosion and sedimentation data

This chapter covers five main topics:

- The estimation of soil loss by the application of empirically derived equations.
- Methods of measuring soil loss by the collection of the total amount of eroded material from runoff/soil erosion plots.
- Methods of sampling runoff that are carried in suspension to determine overall soil losses.

- Methods of laboratory analysis that determine the quantities of suspended soil material.**
- Methods of laboratory analysis that determine soil particle size.**

For practical purposes, soil erosion is regarded as the detachment of soil materials from their previous location. Sedimentation is the transport and deposition of eroded soil and although erosion due to wind occurs, the main transporting agent in almost all environments is water. Thus the theoretical calculation of soil losses are covered by estimates of erosion, whereas the actual measurement of material lost from a catchment is regarded as the measurement of sedimentation.

The erosion that leads to a wide range of environmental problems is usually the result of human activity; such as deforestation, cultivation and over-grazing, and is intimately linked with the runoff process. Agriculture is often a powerful agent in promoting soil erosion and water harvesting frequently exploits the conditions that promote runoff and which can intensify rates of soil removal. The processes of sedimentation are frequently attendant.

Soil loss data are expressed in terms of weight per unit area, for example tonnes per hectare, per season. The measurement of sedimentation, the quantification of eroded soil materials, is likely to be of importance to projects that engage in the activities of agrohydrology and methods of reducing soil erosion are an important aspect of this book. A background is given to the main methods of erosion control, below, but the mechanical and constructional aspects of these controls are discussed in detail in chapter 7, Water Harvesting.

3.1 Soil erosion

The climatic factors that influence erosion and sedimentation processes are chiefly rainfall amount and intensity, which largely determine rainfall energy and runoff quantity, though the relation between them and soil loss is very complex.. Vegetation cover reduces rainfall energy and retards surface water flow, encourages infiltration through the physical perforation of soils and the reduction of the soil moisture reserve. Topography determines land slope and the length of flow of surface runoff, while the character of soils themselves, in terms of texture, structure, density, etc., influences the rate at which soil loss and sedimentation will take place.

Erosion can be summarised as taking five characteristic forms:

Rain splash erosion is the local movement of soil particles under the influence of raindrop impact. Soil particles are detached from the soil surface, elevated by the action of splash and return to the surface somewhat lower down the land slope. Areas with high slopes suffer from such erosion to a much greater extent than flat land. Large quantities of soil are removed from their original ground location by the action of raindrops. The energy equation that relates rainfall energy to intensity, developed by Wischmeier and Smith is of the form:

(kinetic) Energy, $E = 12.1 + 8.9 i$ where (3.1)

E is in m-Mg/ha-mm (metre-metric tonnes per hectare-millimetre) and rainfall intensity "i" is in mm h⁻¹

Clearly such factors as wind direction, drop size, velocity and the nature of the soil will also affect the quantity of erosion that occurs and although small, clayey particles are more easily transported than larger sandy ones, they are not so easily detached from the soil surface.

Sheet erosion is a simplified term for the formation of extremely small channels or rills. These are created under the influence of rain splash and microscopic topographical variability and their wandering causes the eventual erosion of soil in the manner of a sheet. For a given soil and vegetation cover, the erosive power of the overland flow will be related to its velocity and depth.

When runoff concentrates into small streamlets Rill erosion takes place, forming small channels. These channels are clearly visible, but their size is such that they can be obliterated by ploughing. The concentration of flow into rills is particularly important because it leads to the concentration of runoff, increasing its velocity and erosive power. On high slopes and shallow soils such erosion can be destructive.

Gully erosion occurs when channel flow is sufficient to overcome tillage practices and is yet another stage of the increasing concentration of runoff, even though it may take place on an ephemeral basis. Gullies extend by the cutting back of their head and by progressive channel erosion, but stabilisation may occur naturally as the channel becomes harmonious with its slope and vegetation growth is established

Stream channel erosion is dissimilar to gully erosion in that it represents a process that occurs in channels with lower gradients, in which streams often flow

continuously. Suspended material is carried along without contact with the bed, while material moved by the process of saltation bounces or skips along. The bed load is rolled or pushed along the channel bottom.

Soil erosion is a natural recycling process that has continued throughout geological time and has been responsible for the formation of sedimentary deposits that are now the major components of the continental land masses and sea floor. Tolerable rates of erosion have been suggested as being between 5 and 10 tonnes per hectare per year, but extremely different local circumstances of soil depth, formation and productivity make such values rather meaningless.

Influences on Sedimentation

Catchment Size. Generally, an increase in catchment size reduces the proportion of material removed (the "sediment delivery ratio"). This is due the increased opportunities for the entrapment of sediment within the catchment area. The presence of flood plain areas adjacent to large streams and rivers provides an environment for deposition that does not exist with smaller, steeper channels.

Topography and Channel Density. Important topographic factors that influence sediment removal are channel slope and the channel density of a catchment. The sediment delivery ratio is highest for steep channels with well defined courses, rather than low slope streams with ill defined channels. The use of channel density factors (see chapter 6) is common in the regression analysis of sedimentation data.

Precipitation and Runoff Regimes are critical factors in determining the removal of

sedimentary material. Streams of a flashy nature are effective at its removal. High intensity storms not only give large runoff amounts, but also increase soil erosion by rain splash from high energy drops. Catchments that suffer from such storms display high delivery ratios.

Loss of Vegetation Cover and Agricultural Activity play a major part in creating soil erosion.

These influences are built in to the theoretical models of soil erosion that are discussed below.

3.1.1 Theoretical Estimates of Soil Erosion

a. Universal Soil Loss Equation (USLE)

The Universal Soil Loss Equation (USLE) is widely known and was developed in many locations of the US by Wischmeier and Smith. In the 1978 publication (USDA Handbook 537), site data from US locations are given. For example, the full set of data of which Table 3.3 is a sample, covers 160 crop to fallow conversion ratios of soil loss (see Appendix B). The USLE is used to determine the value of conservation measures in farm planning and predicts non-point sediment losses. As its name suggests, it is the most widely accepted method of estimating soil loss and has generated variations that are adapted to various local conditions. Special note is made of the difficulty of applying sod-based rotations in semi-arid areas as are soil and moisture conservation opportunities of residue/mulch management (Table 3.3). However, it is important to point out that despite the simplification of the variables involved in the erosion process by the USLE, its use

is often limited because the evaluation of these variables has not been achieved in many regions of the world. Those wishing to apply the USLE are recommended to obtain Handbook 537, but an outline of procedures is presented below. The average annual soil loss is given by:

$$\mathbf{A = 2.24 RKLSCP \text{ where (3.2)}}$$

A = average annual loss of soil in Mg ha⁻¹ (tonnes ha⁻¹)

R = rainfall and runoff erosivity index by geographical location

K = soil erodibility factor

LS = topographic factor

C = crop management factor

P = conservation practice factor

The factor R was found (under fallow conditions) to be related to the maximum 30 minute rainfall intensity and the kinetic energy of storms. The factor K in t ha⁻¹ was assessed by measurements of actual soil loss for a series of soils with a range of physical and chemical properties. The factor LS converts soil losses from the experimental plot length and slope (22m and 9%, respectively). The conversion formulae for different slopes and values of L are:

$$\mathbf{L = (1/22)^x \text{ and (3.3)}}$$

$$\mathbf{S = (0.43 + 0.30s + 0.043s^2) / 6.574 \text{ where (3.4)}}$$

x - a constant, 0.5 for slopes > 4%, 0.4 for slopes 4% and 0.3 for slopes < 3%

I = slope length in m

s = field slope in %.

C, the cropping management factor includes the effects of cover, crop sequence, length of season, tillage and storm time distribution. Conversions may be made from cropping to continuous fallow. P, the conservation practice value, discriminates between contouring, strips and terraces. Terraces alter the value of the slope length L, which becomes the terrace interval for losses from the terrace, whereas if losses from the terrace channel are required, the contour factor is applied.

Location *	Return period, years			
	2	5	10	20
Annual erosion index R				
Arkansas	308	422	510	569
Indiana	166	225	275	302
North Dakota	56	90	120	142
Single storm index R				
Arkansas	69	115	158	211
Indiana	41	60	75	90
North Dakota	27	39	49	59

* Values within states may vary greatly

Table 3.1: Values of R for Different Return Periods, Annual and Single Storms, USA

Estimates of soil losses can be made and if found to be unacceptable, the manipulation of farm management and conservation practices can be planned for its reduction: for example contour and terrace size and intervals. Although soil loss in terms of t ha⁻¹ (with account taken of economic productivity) is the usual criterion by which the acceptability of erosion is judged, the effects of sedimentation may demand that smaller soil losses should be aimed for.

Tables 3.1 to 3.3 and Figure 3.1 give typical values for the variables in equation 3.1.

Texture	Organic matter content in %		
	0.5	2	4
Fine sand	0.07	0.06	0.04
Very fine sand	0.19	0.16	0.13
Loamy sand	0.05	0.04	0.04
Loamy very fine sand	0.20	0.17	0.13
Sandy loam	0.12	0.11	0.08
Very fine sandy loam	0.21	0.18	0.15
Silty loam	0.21	0.19	0.15
Clay loam	0.13	0.11	0.09
Silty clay loam	0.17	0.14	0.12
Silty clay	0.11	0.10	0.08

* USDA 537 gives a nomograph for the calculation of K, see Appendix B

Table 3.2: Soil Erodibility Factor, K, by Soil Texture in Mg ha^{-1} (t ha^{-1}) *

Cover sequence and management	Crop yields			Crop stage period			
	Meadow (tons)	Corn (bu)	0 (%)	1 (%)	2 (%)	3 (%)	4(%)
1st yr corn after meadow, RdL	2	60	15	30	27	15	22
2nd yr corn after meadow, RdL	3	70	32	51	41	22	26
2nd yr corn after meadow, RdR	3	70	60	65	51	24	65
3rd yr or more corn RdL	-	70	34	63	50	26	30
Small grain with meadow seeding:							
1. Residues disked- in.							
After 1 st corn after meadow	2	60	-	30	18	3	2
After 2 nd corn after meadow	2	60	-	40	24	5	3
2. On disked-in corn stubble, RdR:							
After 1 st corn after meadow	2	-	-	50	40	5	1
After 2 nd corn after meadow	2	-	-	80	50	7	3
Established grass and legume meadow	3	-	-	-	0.4	-	-

Table 3.3: Ratio of Soil Losses from Crops to Corresponding Loss from Continuous Fallow

Crop stages:

0 Turnploughing to seabed preparation: 1 Seedbed to first month after seeding: 2 Establishment to second month: 3 Growing cover from 2 months: 4 stubble or residue to new seedbed.

RdL = Crop residues left and incorporated by ploughing: RdR = residues removed

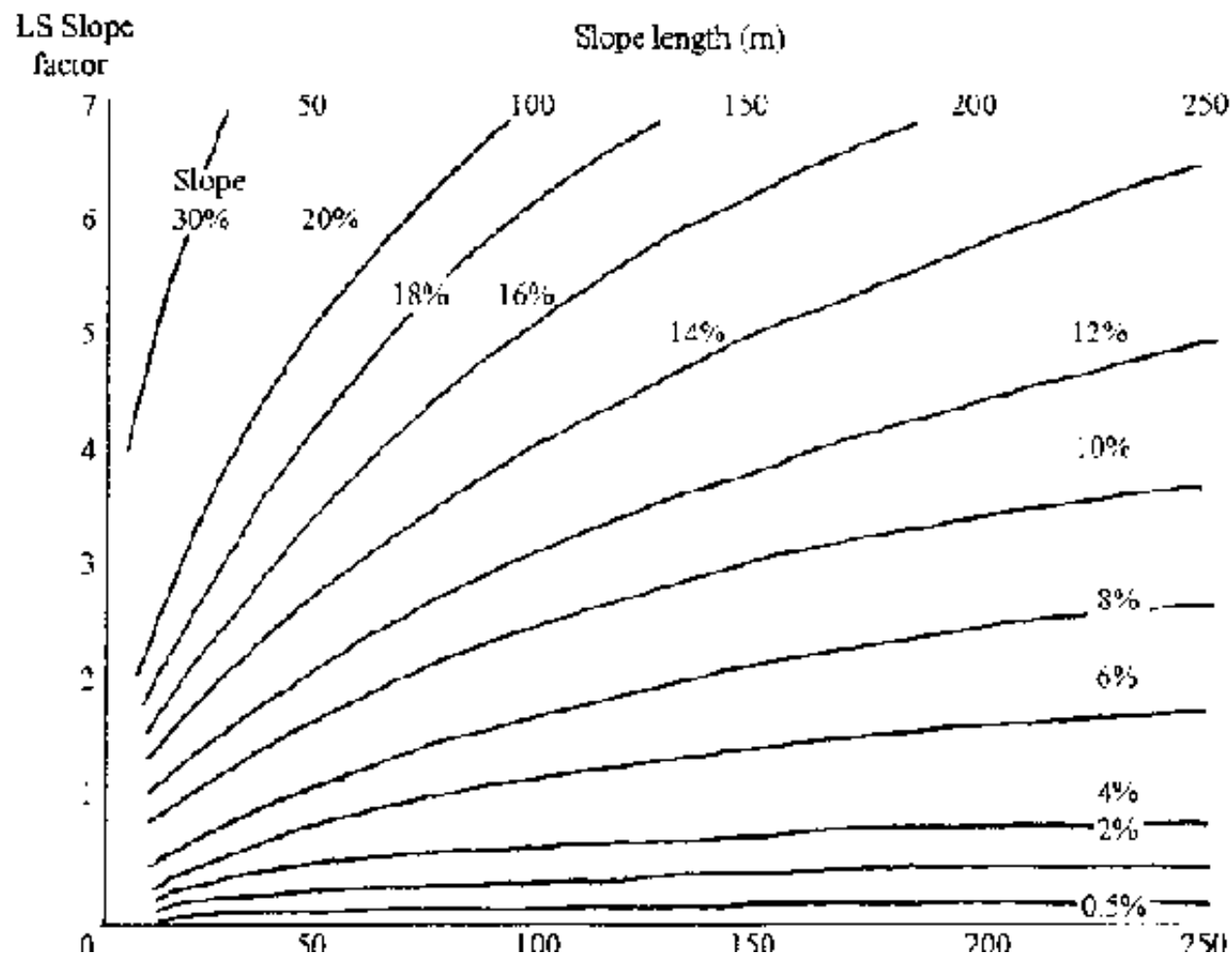


Figure 3.1: Factor LS soil loss for Length and Slope

Some success was obtained using the USLE at Patancheru, India by ICRISAT, though generally the slope factor was not seen to influence soil loss strongly. In South Africa, research in the Orange Veldt region, using rainfall simulator tests to validate USLE information derived largely from the USA, has shown that the use of these inputs was acceptable.

Table 3.4 below gives values for recommended conservation practices. These are greatly simplified from the complex indices given by Wischmeier and Smith (1978) which include values for rangeland, pasture, crops, woodland mulches, etc. Slope length limits are derived from work by the US SCS.

Percent slope S	Contouring (Max slope length in m) P _c	Strip Contouring P _{sc}	Terracing and Contouring P _{tc}
1.1 - 2	0.6 (150)	0.30	-
2.1 - 7	0.5 (100)	0.25	0.10
7.1 - 12	0.6 (60)	0.30	0.12
12.1 - 18	0.8 (20)	0.40	0.16
18.1 - 24	0.9 (18)	0.45	-

Table 3.4: Recommended Conservation Practices

Worked Example

Calculate the annual soil loss for a location with R= 310, loamy sand soil with K= 0.10 (Table 3.2), C = 0.32 (Table 3.3) length L= 60 m and slope S =12%.

From Figure 3.1, LS=2.5 and the field is to be contoured, therefore from Table 3.4, PC=0.5.

$$\text{Annual soil loss} = 2.24 \times 310 \times 0.1 \times 2.5 \times 0.32 \times 0.6 = 33.3 \text{ Mg ha}^{-1}$$

A reduction of these levels of soil loss would appear desirable. This must be undertaken by changing the cropping factor C and/or the conservation practice P and thereby reducing the value of these indices.

b. Modified USLE

The replacement in the USLE, of the rainfall energy factor with a runoff energy factor, has been undertaken in the USA. The equation of prediction is:

$$Y = 11.8 (Qq_p)^{0.56} KCPSL \quad (3.5)$$

All components of the equation are as for USLE, except the energy factor, 11.8 $(Qq_p)^{0.56}$ where Q = runoff volume in m^3 and q_p is the peak flow in $m^3 s^{-1}$.

A combination of measurements from a variety of catchments were tested against this equation and the results encouraging, however the limitations in this model must be regarded as being similar to the original USLE model.

c. Soil Loss Estimation for Model for Southern Africa (SLEMSA)

This model was developed in Zimbabwe, following disappointing results using the USLE. It is based on defined agroecological zones, their physical environments and soils. In particular, the concentration of the USLE on cropped areas and cropping systems was regarded as unsuitable in a region where rangeland conditions are very important. The structure of the model is shown in Figure 3.2 which also provides details of the model components. However, values of many of these components have not yet been determined outside Zimbabwe. Some extrapolation of the model to other countries in the region has been made, but this work has largely depended upon the direct translocation of experimentally-derived values from Zimbabwe, in particular the regression coefficients for the bare soil sub model K , and the soil erodibility factor F . Other components can be measured or

determined at a location. Compared to USLE, this model may become more widely used because of the relative simplicity in obtaining empirical values.

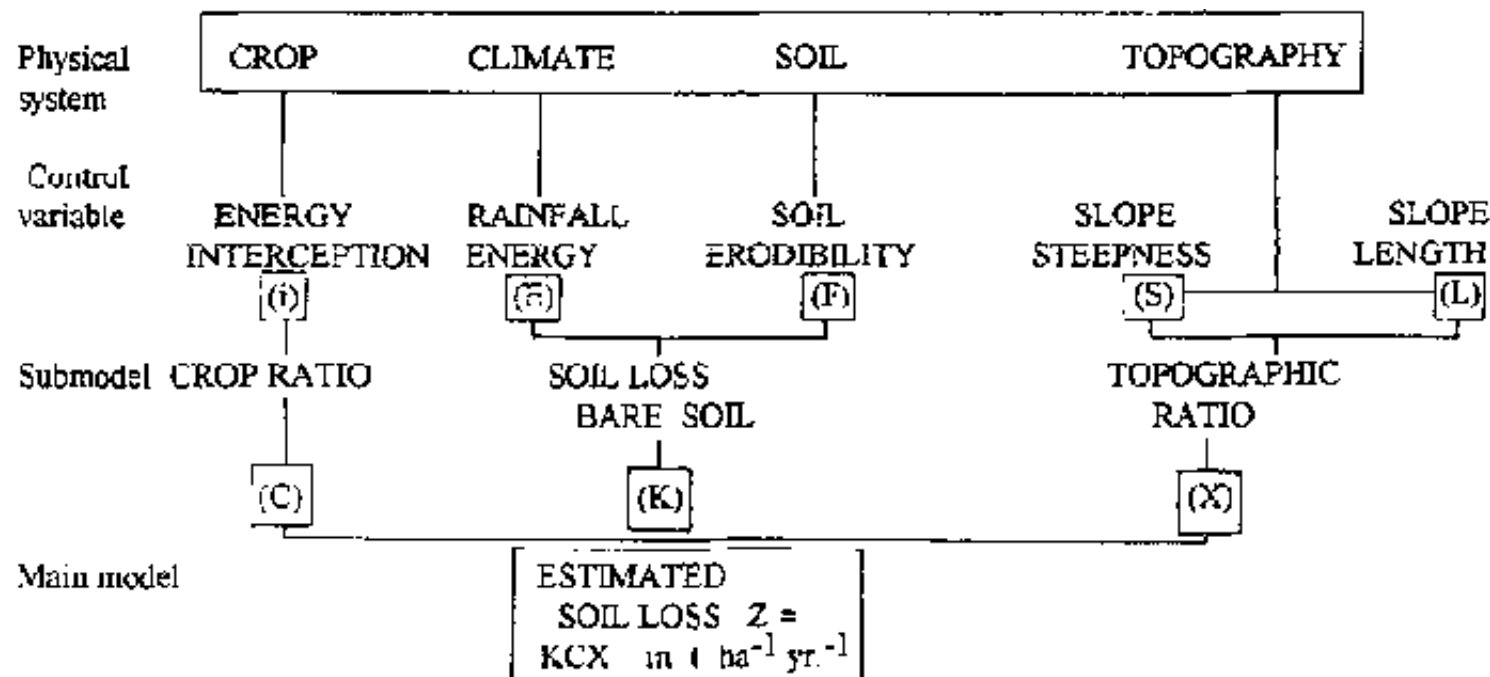


Figure 3.2: Structure and Components of the SLEMSA Model

Empirical studies have defined the values of the parameters K, C, and X in the following terms.

E = Seasonal rainfall energy	Joules m^2	Energy of mean annual rainfall
F = Soil erodibility	-	Index of soil characteristics related to known erodibilities
i - Rainfall energy intercepted	%	Vegetation cover

S = Slope steepness	%	From contours
L = Slope length	metres	
Sub-models		
K = Bare soil condition	tonnes ha ⁻¹ linking E and F	
C = Canopy cover	-	Soil loss ratio related to i
X = Topography		
Main Model		
Z = Predicted soil loss	tonnes ha ⁻¹ = K•C•X	

The value of index K, the bare soil submodel, is obtained by estimating rainfall energy. For example in Zimbabwe and Botswana, research has indicated that with an average annual rainfall of 550 mm, the energy received = 10, 000 joules m⁻² . Luvisol and Regosols were given an erosion index of 4 (the F value). The equation combining rainfall energy and erosivity was found to be:

$$K = \exp(0.461 + 0.7663 F) \ln E + 2.884 - (8.1209)F \quad (3.6)$$

The canopy cover submodel converts the bare soil submodel K soil loss prediction, to a prediction for an area with vegetation, in the form:

$$C = \exp(-0.06) i \text{ where } (3.7)$$

i is the intercepted energy and = mean cover.

The topographic submodel, X, takes account of slope with the equation:

$$X = L^{0.5} (0.76 + 0.53 S + 0.076 S^2) / 25.65 \text{ where (3.8)}$$

L = slope length in m

S = slope in %

Calculations of soil loss were undertaken in grid cells of 1 km².

d. Other Models

The inability of general models to give good results under different geographical conditions has led many researchers to develop their own for particular localities. The majority of these have been regression models, whereby the dependent variable, soil loss, is regressed against a combination of independent variables. In most cases, erosivity equations giving the highest statistical correlations have been developed from the use of a rainfall /energy factor.

Some examples of rainfall/energy relations that have been examined and found to be significantly related to soil loss are given below, but the problems of site specificity are equally as important with these examples as with more general types. Variations in soils, slope and vegetation cover, as well as the characteristics of rainfall, render such equations subject to misuse. However, they do illustrate the main direction of research into soil erosion studies and the main factors in soil loss processes.

Some examples are given below:

Index	Localities
1 KF > 25 index	Nigeria Zimbabwe

1. RL / ZJ INDEX	NIGERIA, ZIMBABWE
2. EI ₃₀	index USA, Kenya
3. EI ₅	index Zimbabwe, Kenya
4. E ₁₅	index Zimbabwe, Kenya
5. AI ₃₀	index Nigeria
6. p ² /P	index Kenya
7. SUM of pi ² /P index	West Africa (related to EI ₃₀)

In case I, the energy of all rainfall intensities greater than 25 mm hr⁻¹ are summed for example:

(a)	(b)	(c)	(b)
x (c)			
Intensity (mm hr ⁻¹)	Amount (mm)	Energy (J m ⁻² mm ⁻¹)	Total
0-25	24	-	-
25-50	25	25.2	630
50-75	20	27.8	417
> 75	10	28.3	290
			1337

Energy values for column (c) are obtained from $E = 30 - 125/I$

In the cases of 2, 3 and 4 (EI indices) the subscript refers to the intensity period in minutes. The energy values are obtained from $E = 11.9 + 8.8 \log I$, then total energy is multiplied by the intensity to give an erosivity value.

In example 5, the index is obtained from the total storm rainfall (A) and peak intensity, I_{30} . In 6 and 7, p is mean rainfall for the wettest month and P is mean annual rainfall.

Examples of EI regression equations from Kenya are:

Soil loss in tonnes per hectare	= 0.026 (EI ₁₅) - 1.18 with	R ² = 0.71 and standard
error = 4.19		
Soil loss	= 0.35 (EI ₃₀) - 1 11	R ² = 0.69 se = 4.26
Soil Loss	= 0.0054 (AI ₁₅) -1.35	R ² = 0.73 se = 4.07

3.1.2 Soil Erosion Control Practices

Soil erosion practices are widely known in agricultural practice and a brief description is given of those most widely implemented.

a. Rotations

Rotations assist in erosion control by ensuring that soils are not exposed to the same risks each season. Differing crop plant covers, growing period and rooting densities, especially when periods of grass cover are incorporated, help to reduce erosion and increase the binding organic matter content of soils.

b. Tillage

Tillage helps to increase infiltration and reduce runoff and soil loss, at least for a short time. However, excessive tillage destroys the natural structure of soils and exposes organic matter to oxidation. Thus a balance must be made between sufficient tillage to achieve a good growing environment for crops and too much tillage which can lead to soil crust development and enhanced runoff. Ploughing depths should be varied to reduce the risk of a hard plough pan forming beneath the soil surface.

c. Minimum and Mulch Tillage

Minimum tillage can give better erosion control and reduce costs compared to conventional methods. Mulch tillage involves covering the soil surface with suitable residues and can reduce runoff and soil losses considerably. It tends to even out temperature differences and helps to protect the soil when plants are small and cannot do so themselves.

d. Grazing Control

The control of stocking rates can be an extremely important factor in preventing soil erosion by maintaining sufficient vegetation cover, preventing soil compaction and thereby reducing runoff.

e. Water Conservation

Water control and conservation, which covers a wide range of practices: tillage, cropping systems, farm planning and physical conservation techniques, is the

most effective way to reduce soil erosion. It amounts to an integrated approach to land management.

Contouring: any farm operation carried out on the contour, ploughing, planting, weeding, etc. may be regarded as contouring. Surface runoff is reduced by the physical barriers thus formed which restrict the movement of water to small distances and low velocities. Ridging increases its effectiveness, but field with microtopography, drainage and gullies may be unsuitable, especially on high slopes. Breakage of the features formed by contouring concentrates runoff and increases soil erosion.

Strip cropping: this practice may be regarded as a type of contouring, but different crops, grass or fallow are placed alternately on the contour to increase infiltration and reduce runoff.

The design and construction of physical conservation practices (ridges, bunds, terraces and flow channels) is covered in detail in chapter 7.

3.2 Field measurement of sediments (eroded material)

3.2.1 Total Sediment Collection

Equipment and Collection of Data

3.2.2 Suspended Sediment Samplers

Equal Transit Rate Method Depth Integrated Sampling

Point Integrated Sampling

3.2.3 Pumping Samplers

Considering the site specific limitations of soil erosion models, it is perhaps not surprising that erosion losses are still widely quantified by direct measurement. The practical aspects of the measurement of eroded material are discussed below.

As with runoff studies, the selection of site locations will be largely determined by their suitability to project objectives. From the agricultural point of view, sedimentation and the loss of soil is the main focus of attention and in many ways it is sensible to measure runoff and sediment at the same site and, where possible, on the same experimental plots. Runoff plots are usually used to measure basic erosion rates, or total sediment load, under specified soil/cover/ slope conditions, and replication will probably be necessary. Total sediment loss measurements obtained from small plots may not reflect real catchment conditions, because the natural conditions of runoff loss and redistribution are imperfectly represented. Natural catchment stream-gauging location points are usually suitable for suspended sediment sampling, as are artificial controls emplaced on catchment outlets. Little extra investment is needed to obtain sediment samples manually, though laboratory facilities must be available for soil loss determination. Where the site is remote and the cost can be justified, pumping sediment samplers are sometimes used. A wide range of sampling devices are available and may be adapted to suit local conditions. A number are described below.

3.2.1 Total Sediment Collection

Total sediment collection is obtained from the kind of runoff plot described in the section on volumetric runoff data collection systems, in chapter 2. These systems are constructed and operated exactly as described in the Runoff chapter, and the design criteria of the tanks, peak flows and total volume are calculated in the same way. The limitations of very small catchment size and regular site visits are also the same. The data from these plots are important in agrohydrological research and such erosion plots are commonly found on agricultural research stations. Arrangements for ploughing, cropping will be made and bare or uncultivated plots may be necessary to provide a controlled comparison. Natural slopes are best suited to the collection of useful data, as reshaped areas will not contain normal soil nor slope profiles.

Unlike most plots used for runoff measurement, the surfaces of plots which suffer from high levels of erosion may become lower over several seasons and it is worth considering whether or not to build in facilities that allow the collector gutter at the downslope end of the plot to be lowered accordingly. The gradient of the pipes leading to the collector tanks must be retained to prevent sediment collecting in them (a velocity of 0.6 m s^{-1} is adequate). This may mean lowering the tanks themselves, and during installation they should not be set in concrete, but put on stands suitable to accommodate the lowering. Over-deep excavation of their position may be necessary and this is often a laborious process necessitating the use of heavy earth moving equipment. Chapter 2 gives details of construction, installation and maintenance.

Samples taken from multislots and rotary dividers are expected to be representative of the total runoff and the following equations may be used in the calculation of sediment discharge.

$$G_S = (Q \times C) / (A \times 103) \text{ where (3.9)}$$

G_S = Sediment discharge in kilograms ha⁻¹

Q = Discharge in m³

C = Storm weighted concentration in ppm

A = Area of plot in hectares

Instantaneous sediment discharge rates can only be found if records of runoff rates are available, that is if a control section and water level recorder are installed on the catchment. If these data are available, then the formula to calculate instantaneous sediment discharge 'g' is:

$$g = (q \times C) / 103 \text{ where (3.10)}$$

q = Instantaneous discharge rate in m³ s⁻¹

C = Concentration in ppm by weight for runoff rate q

Sediments collected in the conduit and tanks of the runoff measuring equipment are weighed in the field. Samples are taken as the material is weighed and the percent of dry material is determined in the laboratory. Dry weight of the deposits is given by:

$$W_{sd} = W_{sw} \times P_{dm} \text{ where (3.11)}$$

W_{sd} = weight of dry sediment

W_{sw} = weight of wet sediment

P_{dm} = percent dry material

Equipment and Collection of data

a. Multislot Dividers

For details of manufacture, installation and use of this equipment, see chapter 2. It is likely that some runoff events will provide no soil material, perhaps because the runoff plot is heavily vegetated or the rainfall is small. In these instances, sedimentation data will not be available.

In cases where runoff provides sediment that can be suspended by stirring the following procedure is followed:

- Agitation should be done energetically using flat paddles. It will probably take two people.**
- Fill three 1 litre sample containers, using a smaller container to fill them. It is best to use 2 or 3 samples to fill each litre container so that examples from the mix are selected.**
- The containers should be pre-labelled to avoid accidental confusion with date, time, plot, tank and sample no.**
- The total measurement of runoff can then be taken, adding the 3 × 1 litre samples to the total runoff amount.**
- It is important to ensure agitation of the mix is continuous, so in total at least three people will be necessary.**

- This should be done for all tanks.**
- As samples will have to be sent for analysis, it is important to check with the laboratory that the required number of samples can be dealt with conveniently. If not, it is probably best to reduce the overall number but increase sampling dips, so as not to become involved with problems of storage and the possibility that results will be late, samples lost or accidentally destroyed. To a large extent this will depend on individual circumstances, but it is an important point to note.**

In cases where sediment is too heavy to be stirred totally into suspension, the following procedure should be followed to collect samples:

- In the divider system described in chapter 2, the sludge will be trapped in the first container in line.**
- The supernatant (water and suspended) material should be removed to within about 2 cm of the top of the sludge.**
- This should be done carefully, with no disturbance of the deposited material. It is likely that a syphon will have to be used rather than a pump, which could suck up the deposited material.**
- Allow more time for this procedure than would be needed for runoff measurement alone.**
- The sample bottles can be filled as the supernatant is siphoned and measured for runoff volume**

- **Stir the sludge aggressively until it is liquid enough to find its own level.**
- **Measure its depth. It is necessary to ensure that the form of the tank/container is known. This can be achieved by calibrating the volume of the container for a given depth (using water), after installation. The precision of calibration depends on the size of the tank, the advantage being with smaller tanks, for which greater imprecision of depth measurement gives smaller inaccuracies.**
- **Mix the sludge once more and during the process take the samples.**
- **Tanks and site should be left clean and free of debris.**

b. Rotary Samplers

For details of manufacture, installation and use of this equipment, see chapter 2. - Heavy sediment should be collected and weighed from the collection conduit and transfer pipes. Tanks samples are taken as outlined above for multislots dividers.

3.2.2 Suspended Sediment Samplers

Suspended sediment samples are taken from water bodies such as streams and reservoirs and do not involve the wholesale collection of runoff. However, knowledge of the location of the source of material (other than it being somewhere within the catchment), is difficult to obtain.

Suspended sediment samplers are designed to sample the sediment/water mix and should ideally have the following features:

- They should sample with the intake at the same velocity as that of the stream.**
- They should sample away from the disturbance they cause.**
- They should be able to sample close to the stream bed.**
- They should be rugged and inexpensive.**

In general it is probably best to purchase specially manufactured equipment, though one set of such equipment could be used to calibrate the results of any locally made equipment capable of fulfilling the criteria listed above.

There are two main types of samplers: depth and point integrating samplers.

The former instrument takes a continuous sample as it is lowered and raised from and back to the water surface. Point samplers are equipped with an electrically-operated valve which can be opened at desired depths and samples taken. If the valve is left open, it will operate as a depth integrating device. These samplers are streamlined bomb-shaped instruments, housing a glass bottle type sampling container. The samplers can be used by wading in streams, but are often used on rivers that necessitate the suspension of the instrument from bridges or cable ways. Several sizes of sampler may be necessary where variations in flow volume are present. The USDA gives recommendations on sampler types according to speed of flow and depth (USDA handbook 224), but these relate only to equipment obtained from within the US. Suspended sediment samplers are especially important where runoff from river catchment areas is under study. Figure 3.2 shows a typical suspended sediment sampler

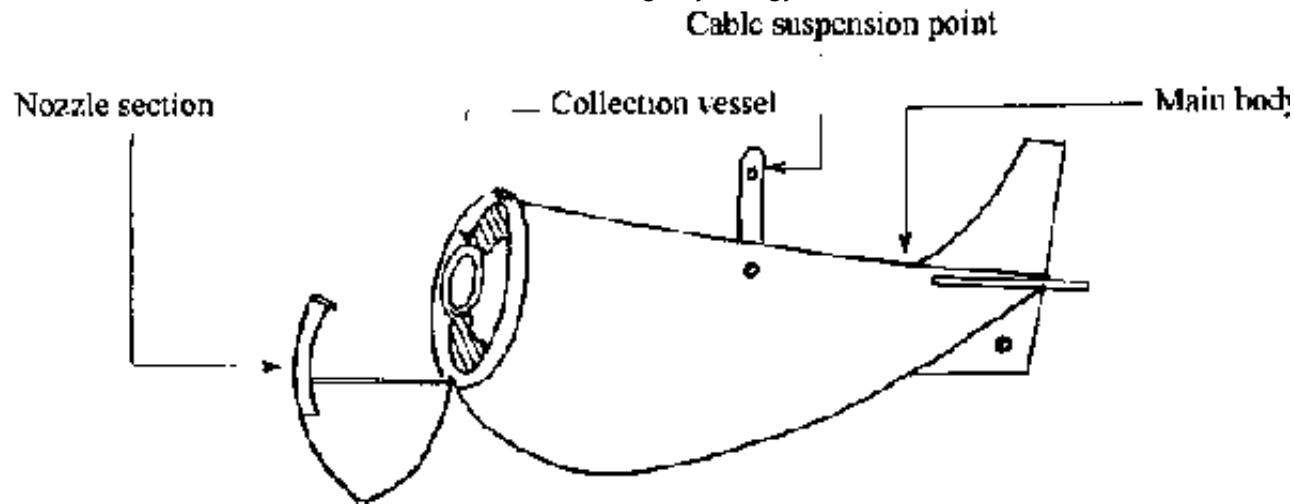


Figure 3.2: Suspended Sediment Sampler

The concentration of sediment within any flow is not only a function of the physical characteristics listed earlier in the chapter, but also of time of flow. In general, the rising limb of a hydrograph is prone to greater changes in sedimentation as the runoff process gets underway and therefore requires more frequent sampling. Similarly, small streams undergo a more rapid change in flow stage than large streams and these too require more frequent sampling.

Table 3.5 gives guidelines for sampling frequencies, but it is important to determine precise rates of sampling according to experience:

Catchment area	Rising limb of hydrograph		Falling limb
	Fast flow	Slow flow	
130 ha	Every 5 min	15 min	30 min for 1.5 hrs, every hour thereafter Every two hours thereafter.
130 ha - 30 km ²	15 min	30 min	Hourly for first four hours. Every three hours thereafter.
30 km ² and greater	30 min	1 hour	Every 3 hours for 8 hours. Every eight hours thereafter.

Table 3.5: Approximate Frequency of Sediment Sampling

Figure 3.3 shows the theoretical distribution of suspended sediment concentration in a stream section, compared to the velocity distribution. Coarse, sand-sized particles count for most of the variation in concentration, fine particles are usually fairly evenly distributed throughout the stream section. As in many cases actual concentrations vary with stage and turbulence, samples must be collected systematically throughout the stream section.

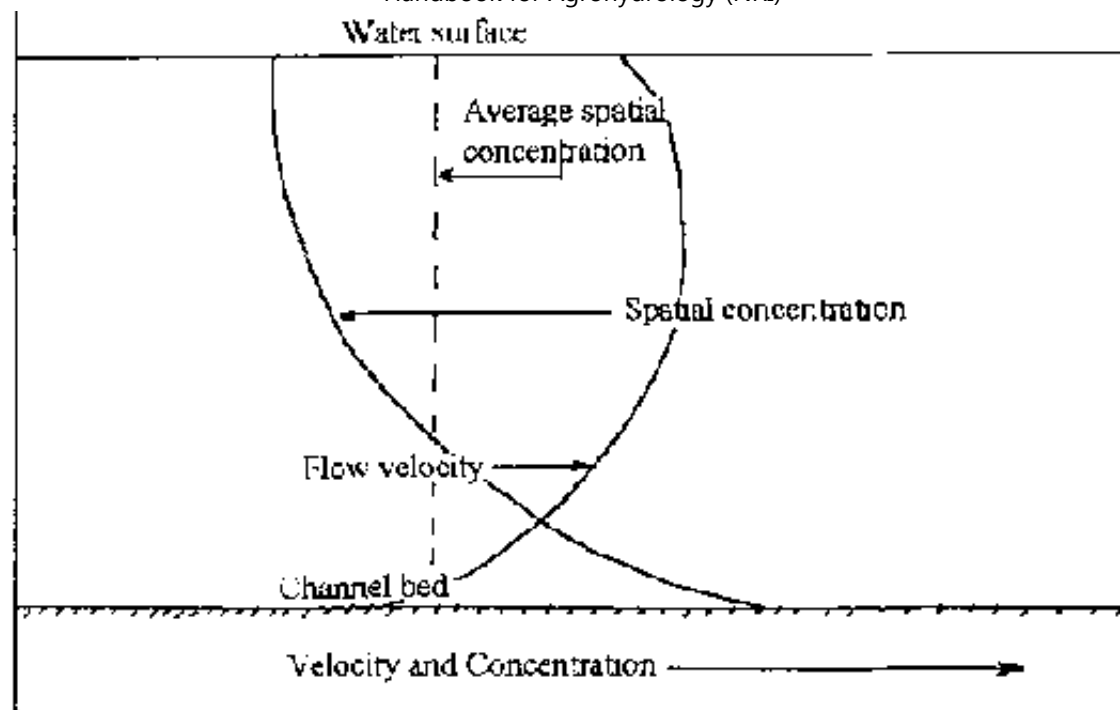


Figure 3.3: Sediment concentration and Stream Velocity

a. Equal Transit Rate (ETR) Method

This method is most commonly used on small streams on agricultural land and gives a discharge-weighted sample for the stream cross-section. Six to twelve sampling verticals spaced equally across the stream usually give sufficient accuracy. Stream discharge measurement at the time of sampling is not necessary, total flow may be estimated from gauge height and a rating curve or rating table. The collection and computation of data are relatively simple using this method, though an integrating sampler is needed and the transit (lowering/highering) rate of the instrument must be uniform. The composite sample that is collected represents the mean, discharge-weighted concentration. Suspended sediment discharge is calculated from the mean concentration and the total water

discharge. Because it is commonly used, this method is covered in detail below.

Figure 3.4 shows the path of a sampler during a typical sampling procedure:

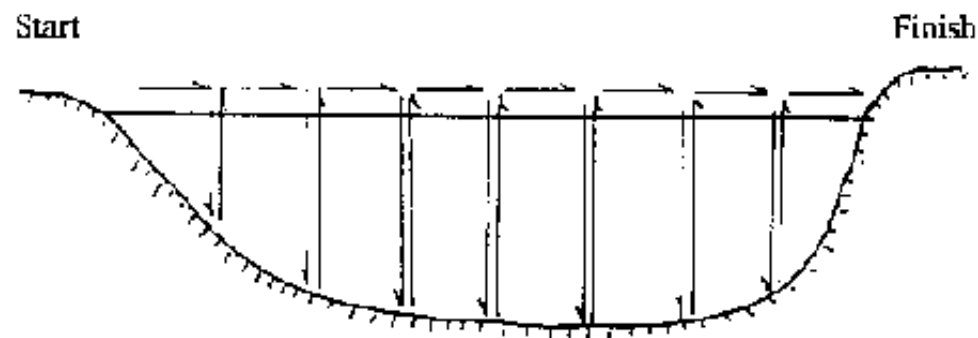


Figure 3.4

Path of Sampler During Equal Rate Sampling Procedure

- **Select a straight section of stream, with as uniform a cross section as possible, near the gauging station. Avoid shallow sections.**
- **Lay out a tape or line across the stream, standing downstream of the line if wading.**
- **Determine the position of about 6 verticals (sufficient for a wadeable stream of 10 m width)**
- **Record the stage of the stream.**
- **Rinse the sampler bottle, check for a clear orifice and stand about 1 m down stream of the line at the first position to take the sample.**

- **Hold the sampler rod and move the sampler down at a constant speed, touching the stream bed at each vertical.**
- **Sampling from surface to bed is recommended to avoid sampling disturbed bed material at higher sampling points.**
- **The sampling velocity should not exceed 0.4 times the stream velocity.**
- **When a bottle becomes almost full, replace it and mark the sequence of verticals and replacement .**
- **Usually between 1 and 6 bottles are needed.**
- **Record water temperature.**
- **Record stage if stream has fallen or risen significantly.**
- **It is convenient to put as much information on the bottle cap as possible for ease of future sorting, but in any case ETR; the stream/catchment name; date and time; stage; bottle sequence numbers; temperature and signature should be written on the bottle label.**

Where sandy bed streams are being sampled, a second sample run will reduce sampling errors. Deeper streams can be sampled in a similar way, but a sampler for use with line and winch equipment will be necessary. Different nozzles will probably be available, so that the rate of collection of samples can be adjusted to such prevailing stream conditions as depth and velocity. Charts are available that indicate the speed of sampler transit and whether a single or two way transit is

recommended for particular nozzles, but the details will depend on manufacturer. Errors that may be caused because of different flow velocities close to the stream banks are not serious.

Where panicle-size analysis is to be undertaken, a second sample must be taken. For all other methods, discharge must be measured at time of sampling.

b. Depth Integrated Sampling

A relatively large number of depth integrated samples must be taken on verticals at the midpoint of equal sections of the width of the stream. Usually 6 to 12 are sufficient, each located within cross-sections of equal discharge. Mean sediment concentration is found by weighting the mean concentration in each sampling vertical with respect to the discharge in the vertical.

Total suspended sediment is found by mean cross section concentration and total water discharge. Discharge must be measured at the time of sampling. Variations in sediment concentration across the stream may be obtained. Alternatively, the collection of depth integrated samples at verticals that represent the middle of sections of equal discharge may be undertaken.

c. Point Integrated Sampling

Samplers used in this method are equipped with an electrically operated valve which takes samples on command. With the valve continuously open, they perform in a manner similar to depth integrating samplers. Point samples are taken in stream verticals which represent equal or known discharge. Mean values are weighted accordingly, but the number of points depend on the physical character

of stream flow.

In general, the ETR method (a.) is most widely used.

3.2.3 Pumping Samplers

These samplers are complex pieces of equipment and relatively costly to obtain and install. It is probable that their cost can only be justified if soil erosion studies are a major activity of a project and if suspended sediment sampling from streams is an important component of this activity. Pumped samples do not represent discharge weighted samples as they are point measurements. Therefore, calibration curves that plot pumped samples against discharge weighted samples (taken simultaneously) must be compiled until a known relation is established. These must be revised should the relation change with time.

They are located in a shelter at the side of the stream and take samples of the flow, a portion of which is retained. They are useful for remote locations where site staff cannot be stationed. Samples are collected and stored in bottles. An intake is placed in the stream and typically a float activated, battery powered system comes into operation at a predetermined stream level. Samples are pumped into bottles at selected, predetermined time intervals until the stream level falls or the containers are full. Most of the pumped water goes to waste, to remove any debris drawn in by the previous sampling process, samples only being taken at the end of the pumping period. It is usual to site the equipment at a gauging station so that an indicating mark can be made on the water level recorder chart, when sampling takes place. Records of stage and sampling are thus linked. Some systems rely on a gravity feed sampling and are usually sited at

such locations as reservoirs and weir installations.

Bed samples may be collected from perennial streams using special sampling dredgers, sampling cores and spuds but in most cases the costs of such equipment will not be justified by the information returned. Deposits of bed sediment may be sampled by soil sampling cores for the determination of bulk density and chemical analysis, during the dry season.

The mapping and surveying of channels, changes in gullies and the upslope movement of gully scarps can be important aspects of erosion and sedimentation studies. Mapping is required in great detail

3.3 Laboratory analysis

3.3.1 Sediment Concentration

Evaporation Method

Filtration Method

Separating Fines and Sands

Particle Size Analysis

Pipette Method

Hydrometer (Bouyoucos) Method

Wet Sieving

Dry Sieving

The analysis of samples will probably be undertaken at a specialist laboratory, but this is an expensive procedure and may even involve the dispatch of samples to another country. Therefore descriptions of common analysis techniques are given

below. In some cases analyses can be undertaken with relatively simple equipment and is possible even where orthodox laboratory facilities are not available, if costs can be met. Evaporation and filtration are the two usual methods of determining sediment concentration. As it is more convenient to work with weights rather than volumes concentrations are usually determined as a ratio of dry sediment weight to sediment/water mixture. Conversion to units of milligrams per litre or parts per million is undertaken afterwards.

Packing and Transport of Soil and Water Samples

The most suitable container for a soil sample is a thick polythene bag which can be sealed with tape. This can then be put into a second paper or cloth bag for extra protection. For all analyses except bulk density, there is no problem if the sample is disturbed. Samples of about 1 kg are suitable, large stones having been removed. Where gravel content is of interest the sample may be 2-3 kg. Very wet samples may be dried, but any mixture of samples or contamination should be avoided. Samples should be placed in small wooden or cardboard boxes for transport as soon as possible to the laboratory; the addition of tuline to kill organisms may be necessary if samples are to be tested for nitrate and cannot be delivered promptly.

Water samples should be carried in screw-top polythene bottles and placed in boxes that are fitted with sections to separate each bottle from the next. Any empty space in packing cases or boxes should be packed with wadding to prevent movement of the samples during transport. Samples of 1 litre are adequate and soda glass containers should be used if an analysis for boron is to be undertaken.

Water and soil sample containers should be clearly marked, at least twice, with a water proof pen. So should any outer container. Details of samples should be recorded in a sample book and generally the less detail on the sample the better, to avoid confusion, however a detailed packing note should accompany any container of samples. It may be necessary to check with the laboratory in case it has a preferred system of labelling.

3.3.1 Sediment Concentration

a. Evaporation Method

Basic equipment is as follows:

- Graduated containers 0.5 -1.0 litre capacity.**
- Large container, 5 litres or more**
- Distilled water**
- Vacuum source**
- Evaporation dishes of several sizes**
- Convection drying oven**
- Pipette**
- Desiccator**
- Flocculating agent**
- Balances accurate to 0.1 gram and 0.1 milligram**

The procedure is as follows:

- Prepare a worksheet (see Figure 3.5)**

- Transfer all sample details from the bottles to the work sheet.**
- Weigh the total sample, less the container(s) weight(s) and record.**
- If the colloidal material is in suspension, flocculate adding 0.40 millilitre of 0.2 molar solution of alum (90.7 g l⁻¹).**
- Allow the sample to settle overnight or for at least 12 hours.**
- Note that flocculant is not usually used for concentrations < 1000 milligrams per litre. If this is done then the introduced error can be calculated for example: 50 milligram per litre solution added to a concentration of 1000 milligrams per litre will give an error of 5%, if all the flocculant is sorbed by the sediment.**
- If samples have relatively little colloidal clay, they can be allowed to settle for a few hours and no flocculant is needed.**
- Using the vacuum source and appropriate tubing, remove all water except 30 millilitres from the sample. This amount can be approximate, so long as it is the same amount for all the samples. All the effluent from the samples from one sampling site can be combined in one large container.**
- Wash the remaining sample into a numbered evaporation dish with distilled water.**
- Place in the oven and dry overnight at 105 - 110°C. It is best to avoid vigorous boiling and splashing of the sample. The sample container can be**

used where possible.

- Remove from the oven and place in the desiccator, weigh as soon as possible after removal.

- Enter gross and tare weights and obtain net weight of sample on the worksheet.

- Mix effluent thoroughly and withdraw 100 millilitres. Oven dry at 105 - 110°C, weigh sample enter on the worksheet.

- Compute the correction factor for the effluent by dividing the net amount of dissolved solids (the residue) by the aliquot volume (in this case 100 millilitres) and multiply the volume of water left in the large sample container × 1 million. $(0.3888 \text{ g} / 178.1 \text{ g}) \times 10^6 = 2183 \text{ ppm}$

To compute the concentration in milligrams per litre:

Concentration = B × (weight of sediment × 10⁶ / weight of water-sediment mixture) in mg l⁻¹.

The value of the factor 'B' can be obtained from Table 3.6 which is based on specific weights for water and sediment of 1.000 and 2.65 g cm⁻³.

SHEET NO. 1

DATE 25.05.90

COMPUTED BY

CHECKED BY

Location

Station 3, Kolobeng R, SE District

DISSOLVED SOLIDS

Date	22.05.90	Date	23.05.90
Time	15:06	Container	A-6
Gauge ht (m)	2.89	Gross (g)	49.1347
Sample Sta.	1/4 L/R *	Tare (g)	49.1226
Temp (°C)	17.8	Net (g)	0.0121
Remarks		Correction	0.0042
SAMPLE	Gross (g)	546	
	Tare (g)	360.2	
	Net wt (g)	178.1	Sand >
SEDIMENT	Container No	22	
	Gross (g)	54.4180	
	Tare (g)	54.0292	
	Net wt (g)	0.3930	
	Correction (g)	0.0042	
	Net wt (g)	0.3888	
CONCENTRATION (ppm)	2183		
TOTAL CONCENTRATION (ppm)			

* indicates that the sample was the first of four samples taken from left to right across the stream.

Figure 3.5: Example Worksheet for Evaporation Method of Sediment Concentration

Compute sediment concentration (in parts per million) as follows. Figure 3.5, the worksheet, provides the example:

Subtract correction factor from the net sediment weight. In Figure 3.5 for example.

$$0.3930 \text{ g} - 0.0042 \text{ g} = 0.3888 \text{ g}$$

Divide oven dry weight of the sample by the net sample weight of the sediment plus water and multiply by one million.

Ratio	B	Ratio	B
0 - 15,900	1.00	322,000 - 341,000	1.26
16,000 - 46,900	1.02	342,000 - 361,000	1.28
47,000 - 76,900	1.04	362,000 - 380,000	1.30
77,000 - 105,000	1.06	381,000 - 398,000	1.32
106,000 - 132,000	1.08	399,000 - 416,000	1.34
133,000 - 159,000	1.10	417,000 - 434,000	1.36
160,000 - 184,000	1.12	435,000 - 451,000	1.38
185,000 - 209,000	1.14	452,000 - 467,000	1.40
210,000 - 233,000	1.16	468,000 - 483,000	1.42
234,000 - 256,000	1.18	484,000 - 498,000	1.44
257,000 - 279,000	1.20	499,000 - 513,000	1.46
280,000 - 300,000	1.22	514,000 - 528,000	1.48
301,000 - 321,000	1.24	529,000 - 542,000	1.50

Table 3.6: Values of Factor 'B' for the Computation of Sedimentation Concentration in mg l^{-1} When Used with Ratio ($\times 10^6$) of Weight of Sediment to Weight of Water/Sediment Mixture (0-29°C)

b. Filtration Method

The filtration method works well with low sediment concentrations and obviates the need for the dissolved solids correction. Compile a work sheet as shown in Figure 3.6

Equipment is as follows:

- Got crucibles, at least 25 millilitre capacity with perforated bottom, suitable to be fitted to a vacuum system.**
- Filters. Commercial glass fibre or cellulose are satisfactory for most sediments.**
- Distilled water**
- Vacuum system**
- Evaporation dishes of several sizes**
- Convection drying oven**
- Desiccator**
- Flocculating agent**
- Balances accurate to 0.1 gram and 0.1 milligram**

Set up as follows:

- **Determine the weight of the sediment/water mixture (the sample).**
- **Allow to settle until clear then decant the excess liquid into another beaker.**
- **Install suitable filter into crucible and determine tare weight.**
- **Connect crucible to vacuum system and transfer sample.**
- **When filtration is complete place crucible into oven and dry at 105 -110 °C.**
- **Remove and place in desiccator. Remove and weigh and compute concentration. Other methods of filtration can be used.**
- **A simple glass funnel fitted with a filter paper, the sample draining under gravity can be used when samples have a relatively high colloidal content and if care is taken.**

Procedure:

- **Follow the first three steps of the evaporation method.**
- **Wet sieve the material using sample water.**
- **Remove the material < 0.062 mm.**
- **Dry and weigh this material.**

- Remove the material > 0.062 mm (sands) and put in a tared evaporation dish.**
- Oven dry and weigh.**
- Adjust to pH 3 - 5 with HCl, using pH paper.**
- Add about 1 millilitre of 30 percent H₂O₂ (hydrogen peroxide) per gram of dry sample, in 40 millilitres of water.**
- Allow to stand to oxidise the organic material (this will take a few hours) and remove any floating organic material.**
- Destroy any remaining organic material and hydrogen peroxide by bringing the sample to a boil.**
- Oven dry sand sample and weigh.**
- Determine the organic content by subtracting the gross weight of sands from gross weight of sands before peroxide treatment and record.**
- Record gross and tare sand weights on worksheet, compute net sand weight and record.**
- Subtract weight of organic matter from weight of sand and record. No correction for dissolved solids is necessary as the effluent was washed into the fines portion during sieving.**

- Compute sands and fine concentrations as described in the Evaporation method. Total concentration is equal to sum of fines and sands.

SHEET NO.	1	DATE	25.05.90	COMPUTED BY	CHECKED BY
Location	Station 3, Kolobeng R, SE District				
Date	22.05.90		DISSOLVED SOLIDS		
Time	15:06		Date	23.05.90	
Gauge ht (m)	2.89		Container	A-6	
Sample Sta.	1/4 L/R		Gross (g)	49.1347	
Temp (°C)	17.8		Tare (g)	49.1226	
SAMPLE	Gross (g)	546.7	Net (g)	0.0121	
	Tare (g)	360.2	Correction	0.0042	
	Net wt (g)	178.1	Sand > 0.062		
SEDIMENT	Container No	22	37		
	Gross (g)	54.4180	33.3463		
	Tare (g)	54.0292	33.2144		
	Net wt (g)	0.3930	0.1349		
	Correction (g)	-0.0042	n.m.	0.0041*	
	Net wt (g)	0.3888	0.1308		
CONCENTRATION (ppm)	2183	+	678	* organic matter	
TOTAL CONCENTRATION (PPM)	2861				

Figure 3.6 Example Worksheet Filtration Method

c. Separating Fines and Sands

The separation of fines and sands is frequently used in analysis and is normally made at 0.062 mm, though different preferences can be catered for. The equipment is the same as the Evaporation method, with the addition of a 0.062 mm (or 0.053 mm) meshed sieve. Figure 3.7 gives an example worksheet.

Procedures:

For the sands:

- Follow the first three steps of the Evaporation method
- Wet sieve on the 0.062 mm sieve using sample water.
- Remove material < 0.062 mm. Dry and weigh to the nearest 0.1 ma.
- Adjust to pH 3 -5 with HCl using pH paper.
- Add about 1 millilitre of 30 percent H₂O₂ per gram of dry sample in about 40 ml of water.

Allow to stand to oxidise the organic matter.

- Destroy hydrogen peroxide by boiling
- Oven dry sample and weigh to nearest 0.1 ma.
- Determine organic content by deducting weight of sample after from before hydrogen peroxide treatment.
- Record gross and tare sand weights, compute net sand weight.
- Subtract organic matter weight from sand weight and record.

- **Compute sand and fines concentrations as for Evaporation method.**
- **Total concentration is found by totalling concentrations of sands and fines.**

SHEET NO. 1	DATE	25.05.90	COMPUTED BY	CHECKED BY
Location	Station 3, Koloheg R, SE District			
Date	22.05.90	DISSOLVED SOLIDS		
Time	15:06	Date	23.05.90	
Gauge ht (m)	2.89	Container	A-6	
Sample Sta.	1/4 L/R	Gross (g)	49.1236	
Temp (°C)	17.8	Tare (g)	49.1120	
SAMPLE	Gross (g)	536.8	Net (g)	0.0116
	Tare (g)	358.8	Correction	0.0035
	Net wt (g)	178.1	Sand > 0.062	
SEDIMENT	Container	497.0	38.0	
	Gross (g)	60.4484	44.6157	
	Tare (g)	60.2060	44.4876	
	Net wt (g)	0.2424	0.1281	
	Correction (g)	-0.0035	o.m. 0.0041*	
	Net wt (g)	0.2389	0.1231	
CONCENTRATION (ppm)	1340	+	690	* organic matter
TOTAL CONCENTRATION (PPM)	2,030			

Figure 3.7: Example Worksheet Sands and Fines

For the fines:

- Continue with the steps of the Evaporation method

3.3.2 Particle Size Analysis

Particle size analysis is undertaken not only for sedimentation work, but also to determine soil textural type. Several methods may be employed because of the wide range of particle sizes frequently present in samples. Table 3.7 gives size range, analysis concentration quantity of sediment and methods of analysis, recommended by the USDA SCS. Table 3.8 gives a grade scale of sediment particle sizes.

Method of particle size analysis	Size range	Desirable range in concentration	Range in optimum quantity of sediment
	mm	Mg l ⁻¹	G
Sieves	0.062 - 32	-	0.05
VA tube	0.062 - 2.0	-	0.05 - 15.0
Pipette	0.002 - 0.062	2,000 - 5,000	1.0 - 5.0
B/W tube	0.002 - 0.062	1,000 - 3,500	0.5 - 1.8
Hydrometer	0.002 - 0.062	25,000 - 50,000	20 - 200

Table 3.7 Recommended Particle Size Analyses

USDA System

Textural class	Size range (mm)	Textural Class	Size range (mm)
Very coarse gravel	64 - 32	Very fine sand	0.125 - 0.062
Coarse sand	32 - 16	Coarse silt	0.062 - 0.031
Medium gravel	16 - 8	Medium silt	0.031 - 0.016
Fine gravel	8 - 4	Fine silt	0.016 - 0.008
Very fine gravel	4 - 2	Very fine silt	0.008 - 0.004
Very coarse sand	2 - 1	Coarse clay size	0.004 - 0.002
Coarse sand	1 - 0.5	Medium clay size	0.002 - 0.001
Medium sand	0.5 - 0.25	Fine clay size	0.001 - 0.0005
Fine sand	0.25 - 0.125	Very fine clay size	0.0005 - 0.00024

Note: Sand through clay sizes are sometimes quoted in microns. 1 mm = 1000 microns

International System

Coarse sand	2.0 - 0.2	Silt	0.02 - 0.002
Fine sand	0.2 - 0.075	Clay	less than 0.002

Table 3.8 Soil Textural Classes and Particle Sizes

The most commonly used methods of analysis are:

Fine sediments: Pipette, Hydrometer and Bottom Withdrawal (B W) Tube Methods. The former two methods are most commonly used and are detailed below.

Coarse sediments: Sieving and the Visual Accumulation Tube Methods. Details of these methods are given below.

In many cases, suspended sediment samples do not contain sufficient material for accurate analysis and thus procedures must necessarily be limited to the separation of fines and sands, though the possibility of combining samples from the same runoff event could be considered where practicable. Soil samples, by comparison, usually require splitting to the required sample amount before analysis. Dry samples can be split by a commercial sample splitter; moist samples by quartering or by extracting two or three samples from a thin tube inserted to the bottom of the material.

Fine Samples

a Pipette Method

This method is based on the principal that thoroughly dispersed particles of a given size will settle below a withdrawal point in a given time, according to Stokes law which is defined by:

$$2(D_1 - D_2)GR^2 / 9p \text{ where (3.12)}$$

D_1 = density of particle in g cm^{-3}

D_2 = density of the liquid in g cm^{-3}

G = acceleration due to gravity in cm s^{-2}

R = radius of the particle in cm

P = viscosity of the liquid in $\text{g cm}^{-1} \text{s}^{-1}$

Equipment is as follows:

- **25 millilitre pipette apparatus (see Figure 3. 8 below)**
- **A vacuum source.**
- **Sedimentation cylinders of 1,000 millilitre capacity with rubber bungs.**
- **Stirring rods, brass of 6.4 mm diameter by 61 cm with a perforated plastic disc 5 cm in diameter, attached to end.**
- **Thermometers, Evaporating dishes.**
- **Desiccators, Stopwatch.**
- **Worksheets, 0.062 mm sieve.**

Predetermined depths and times of withdrawal are given below in Table 3.9, based on the assumptions that particles have a spherical shape and a specific gravity of 2.65. The viscosity of the fluid is assumed to vary from $0.010087 \text{ cm}^2\text{s}^{-1}$ at 20°C and $0.008004 \text{ cm}^2\text{s}^{-1}$ at 30°C . The gravitational constant is 9.80 m s^{-2} .

Procedure:

If an organic matter and soluble salts content is required, the supernate is removed, the sample dried and weighed before these items are removed.

Otherwise the first step is to remove any organic material by oxidation.

- **Use HCl to adjust the sample pH to 3 - 5 for oxidation.**

- **Add about 1 millilitre of 30% H₂O₂ for each gram of dry sample. Stir and allow to stand for several hours.**
- **Any floating material can be removed.**
- **Usually samples need to be heated (less than 70°C) and more H₂O₂ should be added.**
- **When the reaction has stopped, the sample is boiled or washed with distilled water to remove the H₂O₂.**

The second step is to remove soluble salt material.

- **Effervescence, evident when a little dilute HCl is added to the sample indicates the presence of carbonates.**
- **To remove them, add 50 millilitres of a slightly acid sodium acetate (Na O Ac) buffer solution (Na C₂ H₃ O₂ 3H₂O, 1N, 136 g l⁻¹ adjusted to pH 5 with acetic acid) to each 5 grams of sample.**
- **Bring to suspension by stirring with a rubber tipped glass rod. Digestion is helped by heating the beaker in a water or sand bath at near boiling temperature.**
- **After 30 minutes the suspension is washed by filtering with a filter candle (= to Pasteur-Chamberian or Selas type FP porcelain candle, 02 or 03 porosity). Some samples may need two or more treatments.**

- After salts and organic matter have been removed, use the filter candle to remove excess liquid.**
- When the candle is coated with soil, reverse the stopcock and add pressure with the rubber bulb.**
- Touch the candle with inner surface of the beaker to remove any soil. Repeat the filtration and soil removal process.**
- When free water has been removed, mix the sample with a jet of distilled water.**
- Repeat the filtering and mixing process several times and when complete, add pressure as before to dislodge as much soil as possible and wash the soil back into the beaker. The rubber tipped glass rod can be used as an aid to this.**

Temperature	° C	Diameter of particle (mm)					0.002
		0.062	0.031	0.016	0.008	0.004	
		Depth of withdrawal (cm)					
		15	15	10	10	5	5
		Time of withdrawal					
		Seconds	Minutes			Hours	
20.....		44	2:52	7:40	30:40	61:19	4:05
21.....		42	2:48	7:29	29:58	59:50	4:00
22.....		41	2:45	7:18	29:13	58:22	3:54
23.....		40	2:41	7:08	28:24	57:05	3:48
24.....		39	2:38	6:58	27:52	55:41	3:43
25.....		39	2:34	6:48	27:14	54:25	3:38
26.....		37	2:30	6:39	26:38	53:12	3:33
27.....		36	2:27	6:31	26:02	52:02	3:28
28.....		36	2:22	6:22	25:28	50:52	3:24
29.....		35	2:19	6:13	24:53	49:42	3:19
30.....		34	2:16	6:06	24:22	48:42	3:15

Source: USDA Handbook 224

Table 3.9: Time of Pipette Withdrawal for Given Temperature, Depth of Withdrawal and Diameter of Particle

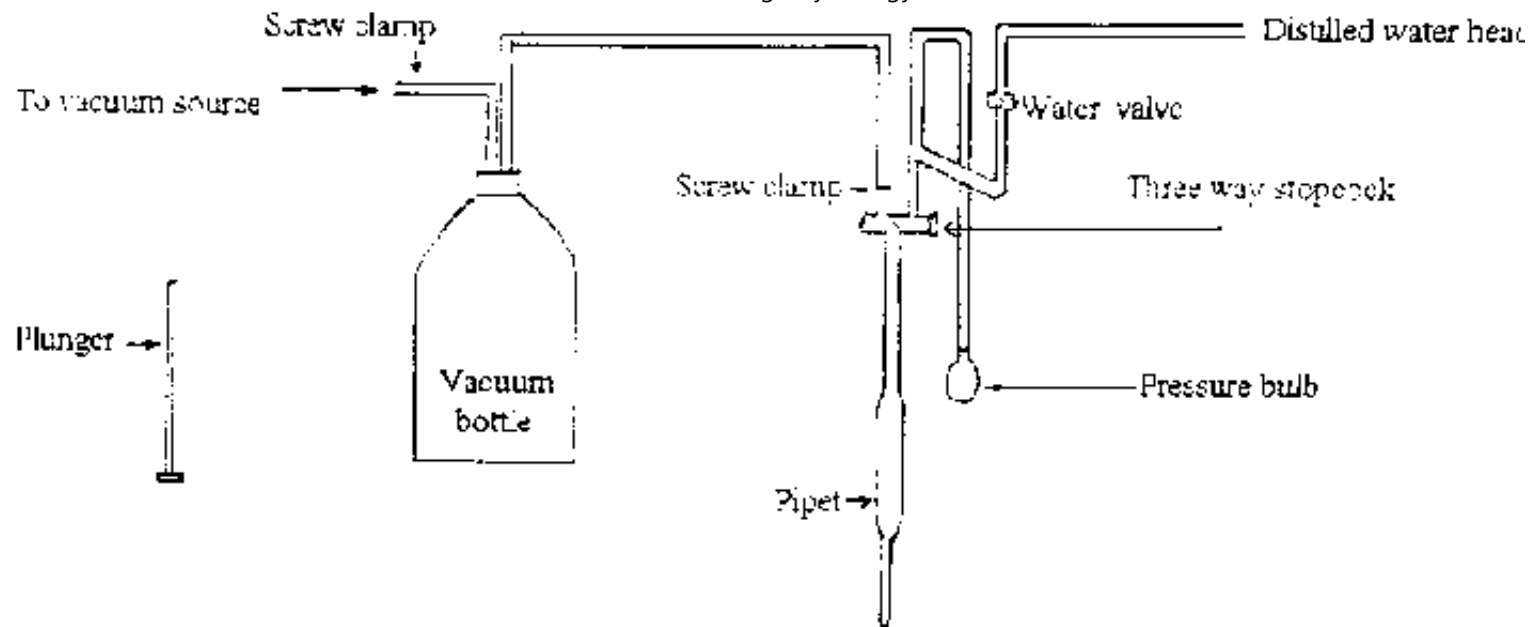


Figure 3.8: Schematic Diagram of Pipette Method Equipment

Thereafter analytical procedures are as follows:

- **If both concentration and particle size are needed, weigh the water/sediment to the nearest 0.1 gram before proceeding.**
- **Remove excess water with a filter candle and place sample in a tared evaporation dish, dry at 100 -110° C and weigh to the nearest milligram.**
- **Deflocculate by adding a dispersing agent (40g sodium hexametaphosphate (Na P03)6 and 8 grams sodium carbonate in distilled water to 1 litre) for each 5 to 10 grams of sample.**
- **Transfer to a 250 millilitre shaker beaker adding distilled water to bring the volume to 180 millilitres, shake overnight. More convenient is the use**

of a mechanical analysis stirrer which will complete the mixing in 2 -5 minutes.

SAMPLE DATA		Dispersing Agent Correction					
Station/ River	Madiba/ Kolobeng	Vol.:	25.0 cc				
Gauge height	2.23 m	Dish no.:	234				
Time	14.06	Gross:	45.6389				
No. bottles	1	Tare:	45.6293				
Sample weight	Gross: 463.6 g	Net:	0.0096				
	Tare: 358.6 g						
	Net: 105.0 g						
Fines < 0.062 mm *		Sand > 0.062 mm		Total sediment			
Gross:	0.0 g	Dish no.:	34	Beaker no.:	23		
Tare:	0.0 g	Gross:	41.4886 g	Gross:	103.1467 g		
Net:	0.9823 g	Tare:	41.3378 g	Tare:	102.0000 g		
Conc.	9,355 ppm	Net:	0.1508 g	O.M.:	0.0136 g		
		Conc.	1,436 ppm	Net:	1.1332 g		
				Conc.:	10,971 ppm		
Pipette No.	228	PIPETTE SIZES	Volume ratio: 40:51				
		Volume:	25 ml				
Size mm	Conc.	< 0.062	< 0.031	< 0.016	< 0.008	< 0.004	< 0.002
Clock Time	8:30	8:33	8:37	8:57	9:24	12:08	
Temperature	25	25	25	25	25	25	
Fall distance	10	15	10	10	5	5	
Settling time	0	2-34	6-48	27-14	54-25	3hr -38	
Container no.	A-1	A-5	A-6	A-10	B-10	D-12	
Wt. Gross	46.6719	47.8425	47.8545	47.8644	45.6213	58.0282	
in Tare	45.6381	47.8126	47.8269	47.8432	45.6008	58.0088	
g. Net (pipette)	0.338	0.299	0.276	0.0212	0.205	0.0194	
D.S. correction	0.0096	0.0096	0.0096	0.0096	0.0096	0.0096	

Net sediment	0.0242	0.0203	0.0180	0.0116	0.0109	0.0098
Finer than	0.9803 **	0.8224	0.7292	0.4699	0.4416	0.3970
% Finer than	86.5	72.6	64.4	41.5	39.0	35.0

* Fines by difference, i.e. total s.m. free sediment minus sand. ** Fines by immediate withdrawal (zero time)
(These values should not differ by more than 5- 10 %)

Figure 3.9: Worksheet for Pipette Method

In some cases, for example where the concentration of suspended sediments is very low, dispersion may not be necessary. Where it is, the dissolved solids correction must be determined for each new solution of the dispersing agent as follows:

- **Add 10 millilitres of dispersing agent to a calibrated sedimentation cylinder, dilute to volume with distilled water, mix thoroughly and remove 25 millilitres, transfer to an evaporation dish and dry overnight at 105 - 110 ° C, weigh the dish and contents to the nearest 0.1 milligram.**
- **Perform in triplicate and use the average. The net weight of the dissolved solids is subtracted from the net weight of each pipette withdrawal.**

Continue as follows:

- **Weigh each sedimentation cylinder while empty then fill to between 500 and 1000 millilitres with distilled water. Weigh again several times and use the average. Do the same for the 25 millilitre pipettes. The ratio of mean weight of water in the cylinder: mean weight of water in the pipette is used as a volume ratio in comparing the results of the withdrawals.**

- Select particle size determinations and using Table 3.9 set up a schedule for time and depth of withdrawals.**
- Use distilled water to wet sieve (0.062mm) the dispersed sample, passing material into a sedimentation cylinder. Place the sands into a tared evaporation dish and dry, weigh to the nearest 0.1 milligram for the net weight.**
- When ready to pipette, bung the cylinder and shake vigorously while turning end over end then plunge with the brass stirring rod.**
- Immediately lower the pipette 10 cm into the sample and take a "zero time" withdrawal. Take the temperature and plunge again for 1 minute. After this make withdrawals according to the depth/time schedule, always measuring the sampling depth from the existing surface of the suspension.**
- Each time, the pipette is flushed with distilled water and with the withdrawn sample, this is put into numbered and tared evaporating dishes. A rubber bulb may be used to blow out remaining droplets.**
- Oven dry the withdrawals overnight (100 - 110 ° C) cool in a desiccator and weigh to the nearest milligram to determine net weight. Results may be tabulated as in Figure 3.9.**

Calculations are carried out as follows:

- From the "zero time" withdrawal determine the net weight of fines in the sample and record. Make a dissolved solids correction if a dispersing agent**

was used. Compute the total weight of fines by multiplying the weight of fines in the suspension by the volume ratio.

- Determine the net dry weight of the sediment in subsequent withdrawals and multiply by the volume ratio. Note: this gives the weight of sediment in suspension finer than the size corresponding to the time and depth of withdrawal.**
- To obtain the fraction of total sediment finer than the indicated size, divide the weight of sediment in the sample finer than the size corresponding to the time and depth of withdrawal by the dry weight of the total sediment in the sample.**
- To obtain the concentration of the fines, sands and total sample in parts per million, divide the total net weight of each by the weight of the total sample (water/sediment mixture) by one million. Record on the form.**

b. Hydrometer (Bouyoucos) Method

The density of the soil suspension is measured with a special hydrometer which is marked with percentage calibrations, calibrated at 20°C. The technique as proposed by Bouyoucos does not remove organic or calcium carbonate material and therefore gives approximate results where these are present in large quantities.

A second hydrometer (ASTM 152 H) was developed by Day with a more rigid adherence to Stokes Law.

The original hydrometer method was devised to provide a quick and easy method, the accuracy of which could be established by comparison with pipette analyses and thereafter be used with confidence. The modifications introduced by Day increase accuracy, but the analysis is no longer rapid (it takes about as long as the pipette method) and it is essential that organic matter and calcium be removed by pretreatment.

Bouyoucos Method

Equipment:

Apparatus and reagents as for pre-treatments and separation of sand

Mixing plunger

Thermometer including the 15 - 25 °C range

Accurate clock or stop watch

Bouyoucos or ASTM hydrometer

Hydrometer jars marked at 1 litre

Procedure:

- Estimate whether sample is sandy (silt and clay < 15%) or not sandy. In first case transfer 100 g oven dry sample to 250 ml beaker and add 100 ml 5 percent solution of hexametaphosphate-sodium carbonate. In second use 50 g.

- Rest overnight, transfer to mechanical stirrer, washing out the beaker and making up the volume to 500 ml with water.

- Stir for 2-3 minutes

(- If sample contains much carbonate or organic material, remove by treatments described previously. Dry and weigh.)

- Transfer suspension to hydrometer jar, wash out stirrer cup and adjust volume to 1 litre. Mix with glass rod.

- Take temperature, which should be 15 - 25 °C. Mix with plunger.

- At selected times lower Bouyoucos hydrometer carefully into centre of solution. Read the scale to nearest 0.5 unit.

The times advocated by Bouyoucos are:

Particles less than	50 microns	40 seconds
	20 microns	4 minutes
	5 microns	1 hour
	2 microns	2 hours

In the case of particles < 2 microns, 6.5 hours may be more accurate for samples with organic matter removed. - Prepare a 0.5 percent solution of sodium hexaphosphate-sodium carbonate, transfer to a hydrometer jar and adjust to 20°C. Insert hydrometer and take reading as a blank. Discard solution. - If the temperature of hydrometer readings was not at 20 °C, the following corrections apply:

Temperature (°C)	Correction (g per litre)	Temperature (°C)	Correction (g per litre)
15	-2.0	21	+0.5
16	- 1.5	22, 23	+ 1.0
17, 18	- 1.0	24	+ 1.5
19	- 0.5	25	+ 2.0

- Subtract the blank reading obtained with the dispersing solution at 20 °C.

The values obtained are direct percentages of clay (< 5 or 2 micron according to time) or silt + clay (< 50 micron or 20 micron according to time) if 100 g were used. Values to be × 2 if 50 g were used.

Coarse Sediments

Both wet and dry sieving procedures may be followed for material greater than 0.062 mm. In each case it is essential not to overload the sieve, the USDA recommends 25 - 50 g for 8 inch diameter sieves for medium - fine sands (3 -7 g for 3 inch sieves) and 10 -20 g for fine sands (1.5 - 3.0 g for 3 inch).

Wet Sieving

- Immerse the sieve having the coarsest screen in a ceramic dish containing distilled water until the water surface is about 5mm above the sieve.

- Wash the sample on to the sieve and shake until all small particles have

passed through.

- Pass the material and washing water onto the next smallest sieve and continue to repeat the process until the smallest sieve is reached.**
- Transfer the material on to a tared container, dry and weigh each fraction. Any material passing through the 0.062 sieve is to be analysed by other methods**

Dry sieving

- Set a nest of sieves on a mechanical shaker, coarsest on top, proceeding to the finest.**
- Weights for each fraction are determined after about 10 minutes of shaking.**

Equipment costs

All costs of locally made equipment are approximate. The costs of raw materials and especially labour are highly variable from country to country, but a good idea of cost magnitude can be gained from the figures quoted below. The costs of manufactured equipment are based on 1993 prices. Shipping costs, agents fees and fluctuations in exchange rate cannot be taken into account.

Item	Quantity	Typical Approximate cost in \$ US
Locally made Equipment		
Total sediment sampler:		
Multislot tank set	1	800 - 1500
with housing	1	500 - 1000
Rotary divider	1	300 - 500
Sheet metal bunds cut into 30 cm deep strips	10 m length	30 - 50
Manufactured Equipment		
Suspended sediment sampler	1	3000 - 4500
Pumping sediment sampler with housing	1	highly variable
Laboratory analysis:		
Balance accurate to 0.1 gram	1	500 - 1000
Balance accurate to 0.1 milligram	1	3000 - 5000
Vacuum source	1	not available
Compressor	1	not available
Drying oven	1	600 - 2000

Figure

Appendix B: Erosion and sedimentation data

Appendix B1: Ratio of soil loss from crop land to corresponding loss from continuous fallow

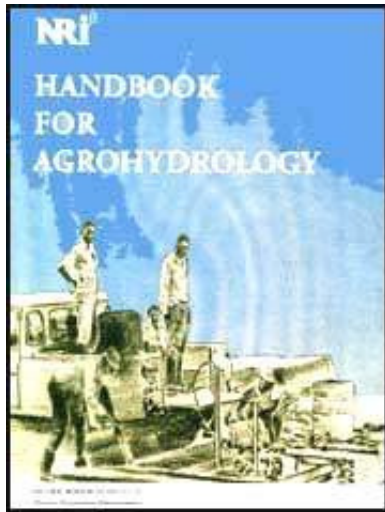
Line No.	Cover, tillage sequence, and management*	Spring residue†	Cover after plow†	Soil loss rate* for cropstage period and canopy cover†								L-FA No.	Cover, tillage sequence, and management*	Spring residue†	Cover after plow†	Soil loss rate* for cropstage period and canopy cover†								
				T	S8	1	2	3:20	90	96	100					F	S8	1	2	3:20	90	96	100	
CORN AFTER C, GS, G OR COF OR MEADOWS/LEG SYSTEMS											CORN AFTER WC OF BYSTRESS OR WHEAT BEEDS IN C STRIP													
Meadow plow, cover all, fall, spring TP											WC residues standing after March pl in killed WC													
1		4,500	31	35	48	38				70	23	79			4,000			2	2	7		7	6	(1)
2		3,400		24	29	32	41			14	20	20			3,000			13	11	11	11	9	7	
3		2,500		41	44	56	43	32	13	21	17				2,000			13	13	14	14	11	7	
4		2,000		53	68	80	45	13	18	22	47				1,500			20	17	18	18	14	11	
Fall, fall TP											Strip till one-fourth row space													
5		4M		44	45	53	38				20				4,000			13	13	14		11	8	(1)
6		CP		49	70	57	41			34	28				2,000			18	17	18	18	13	10	
7		TP		57	74	61	43	12	15	21					2,000			27	23	20	17	13	12	
8		TP		63	78	63	43	12	14	22					1,500			28	24	24	22	17	14	
Fall, spring TP											Rows on residue†													
9		MP		64	74	65	47			12	16				4,000			10	10	10		10	8	(1)
10		CP		67	73	66	47			12	13				3,000			13	13	13	13	12	8	
11		TP		68	76	67	48	13	17		16				2,000			10	10	10	10	10	12	
12		TP		68	77	68	48	13			17				1,500			13	14	13	13	12	8	
Fall, fall TP											TP, cover seedbed													
13		MP		76	82	70	49			23					4,000			18	18	17	17	17	14	(1)
14		CP		77	83	71	50			23					3,000			20	19	19	19	17	12	
15		TP		78	85	72	51	13	17						2,000			15	14	13	13	12	10	
16		TP		79	86	73	52	13							1,500			18	18	17	17	17	14	(1)
Whowback pl, fall, TP											Rows on residue†													
17		4,500				37	27	23			19	20			4,000			13	13	14		11	8	(1)
18		3,400				36	32	30			18	20			3,000			15	15	15	15	14	10	
19		2,500				43	38	33	29	23	19	27			2,000			20	20	21	20	18	14	
20		2,000				51	43	38	31	24	20	27			1,500			28	26	27	27	22	19	
Rows without disk or disk plow											Strip till one-fourth row space													
21		4,500	10			43	38	34			20	23			4,000			18	18	18	18	17	14	(1)
22		3,400	10			53	43	37			24	29			3,000			23	23	23	23	21	18	
23		2,500	5			57	48	40	32	23	21	27			2,000			28	28	27	27	25	22	
24		2,000				61	51	42	33	24	23	27			1,500			31	31	30	30	28	25	
March plow in crop residue											March pl in killed WC													
25		8,000	23			2	2	2			2	14			4,000			13	13	13	13	12	10	
26		6,000	20			3	3	3			3	14			3,000			15	15	15	15	14	11	
27		4,500	18			5	5	5			5	15			2,000			20	20	20	20	19	16	
28		3,400	20			8	8	8			8	19			1,500			28	28	28	28	27	23	
29		3,200	40			12	13	12	12	9	8	23			1,000			38	38	38	38	37	33	
30		2,400	50			15	15	14	14	11	9	27			500			50	50	50	50	49	45	
31		2,000	40			21	20	18	17	13	11	30			200			70	70	70	70	69	65	
32		2,000	30			26	24	22	21	17	14	38			100			100	100	100	100	99	95	
Flared, shallow disk, or MP soil, no disk tillage, or moderate strip											CORN IN SOO-BASED SYSTEMS													
33		8,000	70			8	8	7			7	17			103			1	1	1		1	1	1
34			80			10	9	8			8	17			104			1	1	1		1	1	1
35			20			13	11	10			9	18						2	2	2		2	2	2
36			46			15	13	11			10	19						3	3	3		3	3	3
37			34			18	15	13			12	20						4	4	4		4	4	4
38			26			23	20	18			18	21						5	5	5		5	5	5
											Either tillage after seed: (1) (1) (1) (1) (1) (1) (1) (1)													
											CORN AFTER SOYBEANS													
39		Da,	4,500	70		8	8	7			7	18			109			40	37	30	43		23	27
40			80			12	10	9			8	18			110			49	43	35	51		30	37
41			30			14	13	11			9	19			111			58	52	40	58		31	44
42			48			17	15	13			10	20			112			67	62	48	68		33	44
43			50			21	18	15			13	21			113			73	67	51	73		35	48
44			20			23	22	19			14	22			114			82	76	58	82		37	51

Appendix B1 part I

45	Do.	3,400	80	—	13	11	10	—	15	8	20	113	Fall & long chisel or roll	MP	450	—	48	33	29	—	—	23	29	
46			26	—	18	13	12	—	12	9	24	116		CP	75	—	45	19	30	—	—	27	37	
47			46	—	19	17	16	—	14	17	33	117		CP	29	—	21	44	39	34	27	23	37	
48			55	—	23	21	18	—	17	14	26	118		FP	13	—	28	31	44	36	28	23	41	
49			20	—	29	25	23	—	27	14	27	119		FP	10	—	47	29	48	36	28	23	41	
50			10	—	36	31	29	—	24	20	30	120	Verti-tilt pl in row field	MP	440	—	70	29	19	—	—	14	13	26
51	Do.	3,600	50	—	17	16	15	—	15	10	29	121		CP	20	—	30	29	33	22	18	14	33	
52			48	—	21	20	17	—	19	12	30	122		FP	20	—	44	38	32	27	23	18	48	
53			30	—	25	23	22	—	28	14	32		BEANS AFTER CORN											
54			20	—	22	20	20	—	27	17	34	123	Surg 7F, Roll, row field	MP	—	33	40	30	22	—	—	20	17	(?)
55			10	—	41	36	34	—	33	21	37	124		CP	—	19	41	36	41	—	—	21	18	
56	Do.	2,000	40	—	23	21	20	—	18	12	37	125		FP	—	42	48	30	45	29	22	—	—	
57			38	—	27	25	24	—	23	18	39	126	Fall 1F, Roll, row field	MP	—	46	49	37	38	—	—	23	17	(?)
58			20	—	33	32	30	—	32	18	42	127		CP	—	32	23	41	41	—	—	21	18	
59			10	—	46	42	38	—	33	26	47	128		FP	—	39	27	55	43	27	22	—	—	
60	On slopes > 12 percent. Lines 33-39 times factor of 1	—	—	—	1.3	1.3	1.1	—	1.0	1.0	1.0		Chisel or Roll only:			(?)	(?)	(?)	(?)	(?)	(?)	(?)	(?)	
	Disk or harrow after spring chisel or Roll only: Lines 33-39 times factor of 1	—	—	—	1.1	1.1	1.1	—	1.0	1.0	1.0	129	BEANS AFTER BEANS			(?)	(?)	(?)	(?)	(?)	(?)	(?)	(?)	
	On moderate slopes	—	—	—	1.1	1.1	1.1	—	1.0	1.0	1.0	130	GRAIN AFTER C, G, GS, COM											
61	On slopes > 12 percent	—	—	—	1.4	1.4	1.2	—	1.0	1.0	1.0	131	In disked rows:	4,500	70	—	12	12	11	7	4	2	(?)	
62	Ridge plants ¹⁰ Lines 33-39 times factor of 1	—	—	—	1.1	1.1	1.1	—	1.0	1.0	1.0	132		2,000	40	—	16	14	12	7	4	2		
	Rows on contour ¹¹	—	—	—	1.1	1.1	1.1	—	1.0	1.0	1.0	133			50	—	22	19	14	8	5	3		
63	Rows U/D slope < 12 percent	—	—	—	1.1	1.1	1.1	—	1.0	1.0	1.0	134			40	—	27	21	14	9	5	3		
64	Rows U/D slope > 12 percent	—	—	—	1.3	1.3	1.0	—	1.0	1.0	1.0	135			10	—	23	25	18	9	4	3		
	Full plants ¹² Lines 33-39 times factor of 1	—	—	—	1.1	1.1	1.1	—	1.0	1.0	1.0	136	Do.	3,000	40	—	27	24	19	9	4	3	(?)	
	Rows on contour ¹¹	—	—	—	1.1	1.1	1.1	—	1.0	1.0	1.0	137			10	—	42	34	24	11	7	4		
65	Rows U/D slope < 7 percent	—	—	—	1.6	1.6	1.0	—	1.0	1.0	1.0	138			10	—	37	39	27	12	7	4		
66	Rows U/D slope > 7 percent	—	—	—	1.6	1.6	1.0	—	1.0	1.0	1.0	139			10	—	44	36	26	12	7	4		
67	Strip till (one-fourth of row spacing): Rows on contour ¹¹	4,500	450	—	17	10	8	—	—	8	23	140	In disked stubble, Roll	—	—	—	77	62	47	17	11	6	(?)	
68		3,400	30	—	16	14	12	—	11	10	27	141	Winter G after fall 1F, Roll	MP	—	31	33	48	31	12	7	5	(?)	
69		3,600	40	—	22	19	17	—	14	12	30	142		CP	—	36	46	32	32	12	8	3		
70		2,000	30	—	27	23	21	—	20	15	34	143		FP	—	43	44	56	34	14	9	3		
71		2,000	30	—	31	28	25	—	24	15	34	144		FP	—	51	66	60	38	13	10	6		
72	Rows U/D slope	4,500	450	—	16	13	11	—	—	9	23	145	GRAIN AFTER SUMMER FALLOW											
73		3,400	50	—	20	17	14	—	12	11	27	146	With grain residue:	200	10	—	70	54	43	18	13	11	(?)	
74		2,600	40	—	24	22	19	—	17	14	30	147		300	30	—	41	34	23	12	10	8		
75		2,000	30	—	31	28	25	—	24	15	34	148		750	40	—	34	27	18	10	7	7		
	Verti-tilt: Rows on contour ¹¹	3,400	40	—	13	13	11	—	—	11	23	149		1,000	30	—	26	21	13	8	7	6		
76		3,400	30	—	16	13	14	—	12	12	26	150		2,000	40	—	28	16	12	7	5	5		
77		2,600	20	—	21	19	19	—	16	14	31	151		3,000	70	—	14	11	9	7	4	3		
78												152	With row crop residue:	100	5	—	87	63	44	19	14	13	(?)	
												153		300	15	—	62	47	35	17	13	11		
												154		750	20	—	50	40	29	14	11	9		
												155		1,000	30	—	46	31	24	13	10	8		
												156		2,500	40	—	31	24	18	10	8	7		
												157		3,000	54	—	33	19	14	8	7	5		
												158		2,600	45	—	17	14	11	7	5	4		
													POTATOES											
												159	Rows with slope	—	—	43	44	56	34	26	19	14		
												160	Contoured rows, ridged when row crop is absent 50 percent ¹¹	—	—	43	44	56	34	26	19	14	2	

Appendix B1 part II

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Handbook for Agrohydrology (NRI)

Chapter 4: Rainfall and other meteorological data

4.1 Rainfall

4.2 Other meteorological data

Equipment costs

Handbook for Agrohydrology (NRI)

Chapter 4: Rainfall and other meteorological data

4.1 Rainfall

The collection of snowfall data is not covered here. If detailed information is needed, consult a handbook such as USDA No. 224 or approach the local meteorological service. Ordinary, standard gauges with the collection/funnel component removed and a measured amount of antifreeze added can be used to measure snowfall, but errors due to wind effects can be very large.

Rainfall is the most important single factor in determining whether runoff will or

will not occur for a given set of environmental conditions. It determines runoff amount and frequency. There are two measurements of rainfall amount that are commonly collected for hydrological purposes: Daily and (runoff) Event rainfall.

Daily Rainfall is probably the most ubiquitously measured meteorological variable. It is the rain that falls awing a 24 how period starting in the morning of one day (commonly 06:00, 07:00 or 08:00 furs) until measurement is made at the same time the following day. Event Rainfall by contrast is the rainfall occurring awing an unspecified time period usually, but not always less than 24 hours, that can be seen to be responsible for subsequent runoff. The collection and use of each has advantages and disadvantages.

In the case of daily rainfall, data are usually available from many stations, even in countries with only the most basic meteorological network. The equipment to measure daily rainfall is relatively cheap, simple to install, read and maintain. All projects should easily achieve adequate instrumentation. In most cases, many years of historical data will be available for analysis from a variety of sources, in addition to that obtained from meteorological offices: these sources include various government departments, water resource and construction projects, state and private farms, schools and interested individuals. Often basic analyses will have been performed on the data (average monthly and annual totals, spatial distribution, etc.). For the analysis of runoff relations, however, daily rainfall can have one serious drawback. It is the lump sum rainfall awing a 24 how period and in some climatic environments may greatly exaggerate the amount of rainfall thought to be responsible for runoff, but despite this drawback, it is the most commonly used climatic variable in runoff studies.

Event rainfall, obtained from the careful examination of the records of an automatically recording rain gauge, can provide a precise and accurate evaluation of the rainfall responsible for runoff and it is often to be preferred for rainfall/runoff analyses. However, recording rain gauges are not usually in widespread use except at important synoptic stations (especially in developing countries). They are expensive to buy, can be difficult to maintain and staff must have a higher level of expertise to operate them. The analysis of data is more complex and time-consuming.

It is possible to determine whether or not daily rainfall and runoff event rainfall are for all practical purposes, the same, though a number of historical data are necessary to do this. Values of daily rainfall are plotted against values of runoff event rainfall, as illustrated in Figure 4.1. A visible correlation exists for these data from sites around Gaborone in SE Botswana, confirmed by the statistical significance of the coefficient of determination of the relation ($R^2 = 0.99$) The gradient of the line which is for all practical purposes 1:1, the regression equation being $y = 1.025x + 0.068$. In this case, daily rainfall values could be used instead of event rainfall, without fear of inaccuracy. The ratio of Daily to Event rainfall may not be 1:1, but any relation that is highly significant can be used. Where the relation is not significant, daily rainfall cannot be used to estimate the actual rainfall that caused runoff. The similarity between daily and event rainfall depends on climatic regime but it will be more evident in areas that have convective rainfall, giving heavy, short periods of rain and many thunderstorms. The analysis of rainfall data is discussed in greater detail in Chapter 8.

Rainfall Intensity defines the amount of rain falling during a specified time within the most intense period of the rain. This value is then converted into the amount

of rain that would fall in one hour at this intensity. For example if 5 mm of rain falls in a 2 minute period, the 2 minute duration rainfall intensity is 150 mm hr⁻¹ and if 27 mm falls in 30 minutes, the 30 minute duration rainfall intensity is 54 mm hr⁻¹. Rainfall intensity can have an important effect on runoff proportion, as it determines the rate at which rain arrives at the soil surface and, consequently, whether the infiltration rate of the soil is sufficient to allow absorption. Automatic recording gauges are needed to measure rainfall intensity. The duration under study will be determined to a large extent by catchment size; small plot runoff is often closely related to the 2 or 5 minute intensity durations while large catchment relations are more evident with long durations. The use of daily rainfall becomes a more attractive proposition under these circumstances, especially as the use of long duration intensities can lead to a serious reduction in data, when rainstorms are short-lived.

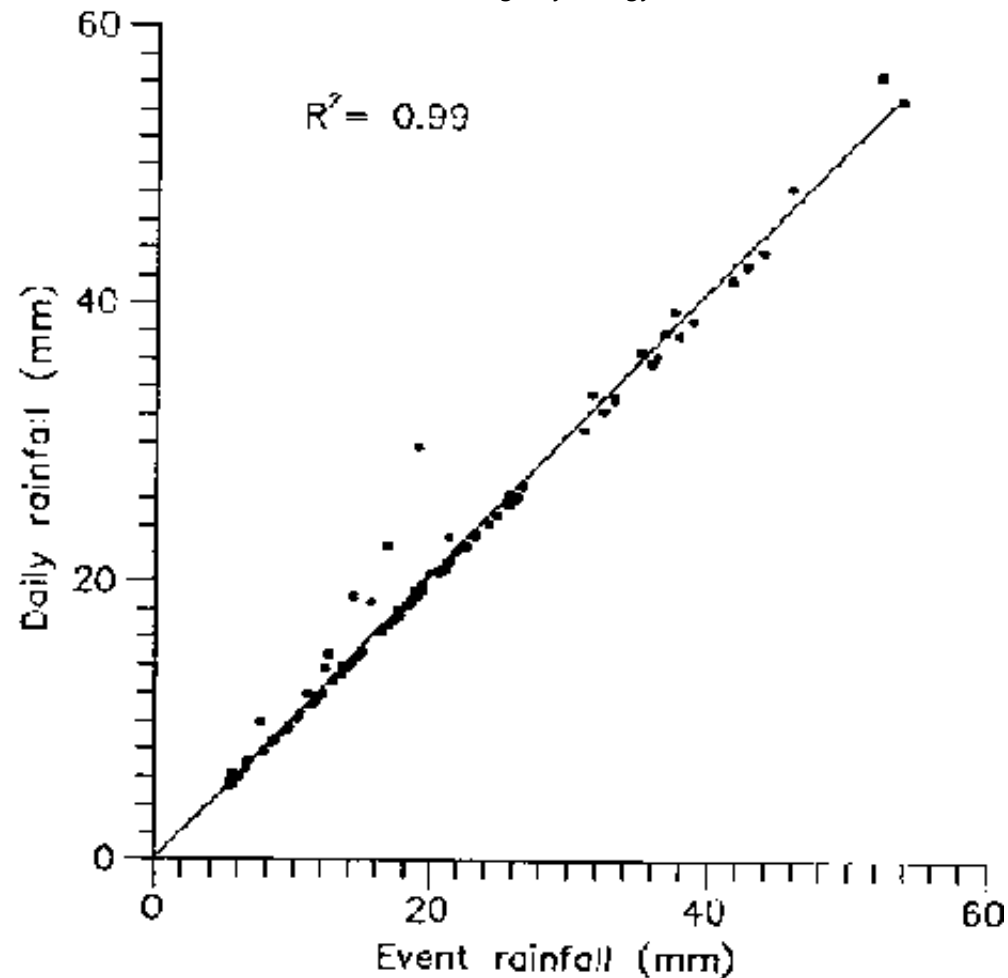


Figure 4.1: The Relation Between Daily Rainfall and Event Rainfall

4.1.1 Non-Recording (Daily) Rain Gauges

All agrohydrological projects will collect daily rainfall from manually read rain gauges. Even when automatic recording gauges are installed it is important to cross-check the amounts that they measure and ensure that at least daily values are collected if they develop faults. Some automatic gauges have integral manual gauges sited under the recorder to facilitate this. Different models of daily rain

gauges are used in different countries. In the case of some developing countries, which can ill-afford even basic meteorological equipment, national networks may use more than one type, having been supplied by different aid agencies or having inherited them from various projects. Usually this does not cause serious problems. It is essential however, to ensure that the correct measuring vessel into which the rain is poured from the gauge (and calibrated only for one particular model) is used to measure the rainfall amount. As a general rule, it is sensible to purchase equipment compatible to that already used by the national Meteorological Service. This has several advantages:

- Data will be strictly compatible.**
- In the case of loss or breakage of measuring vessels (glass), a temporary loan may be possible.**
- The Service will be familiar with procurement/replacement procedures.**
- The Service will be familiar with the most suitable methods of installation.**
- Service staff and other gauge readers who may be called upon to read the gauges, will be familiar with the equipment. In some cases it may even be possible to arrange the loan of gauges, if the Service has sufficient reserve.**

Types of Gauge

Any open ended vessel can be used as a rain gauge, but uniformity of design to provide consistent splash characteristics necessitates the use of purchased

equipment. Most gauges are made of corrosion-resistant metal such as brass.

Gauges with orifices of different sizes measure rainfall with about the same degree of accuracy. Results of tests show that readings are within 1% from gauge openings of between 5 to 50 cm. Differences in measurement usually result from installation and reader error rather than gauge design.

The standard US Weather Bureau non-recording gauge has an 8 inch (20.3 cm) orifice, the UK standard gauge (commonly adopted by former colonies) has a 5 inch (12.7 cm) opening. There is little to choose in design accuracy, but in general, smaller gauges tend to be less expensive. Figure 4.2 shows examples.

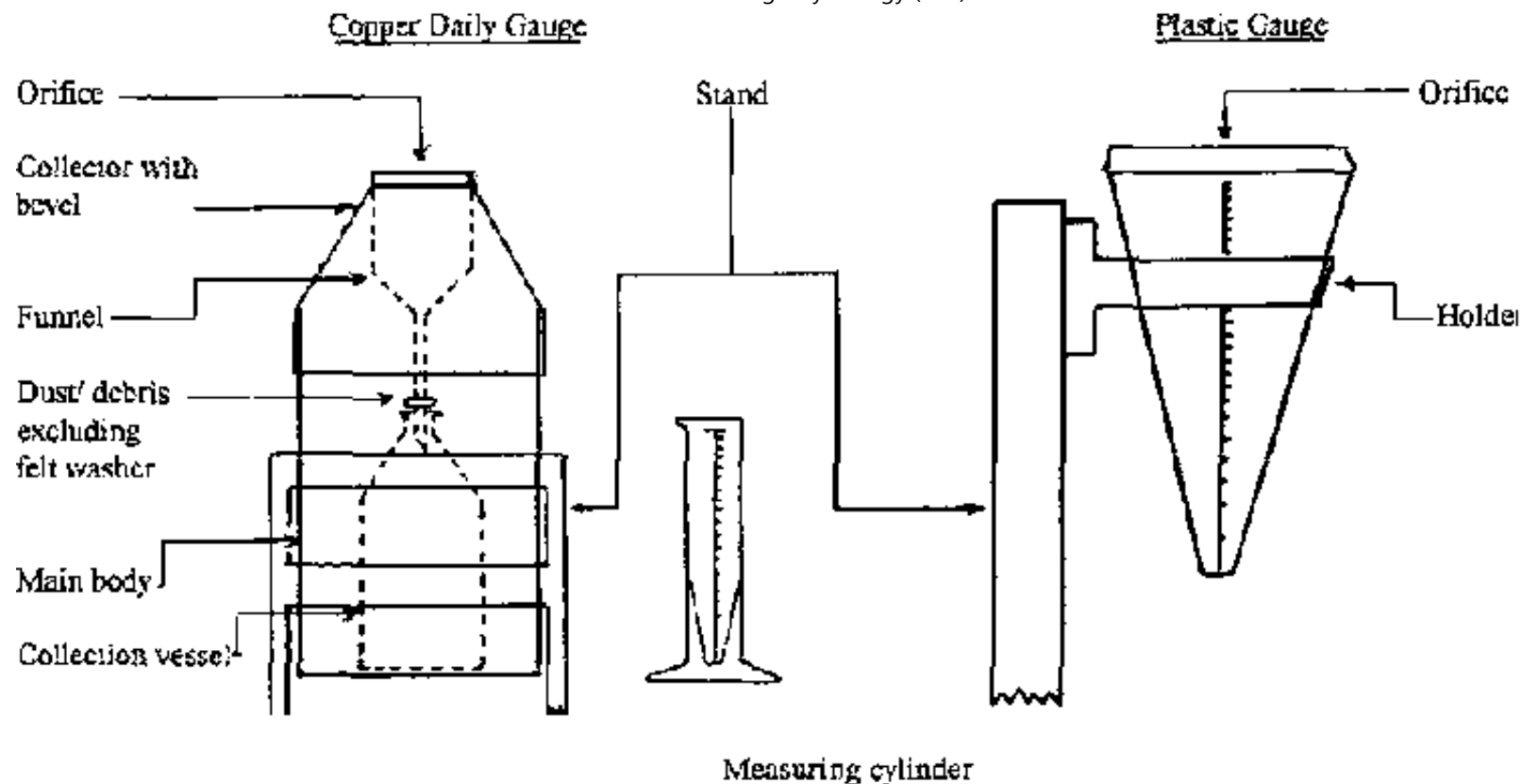


Figure 4.2 Non-recording Rain Gauges.

Non-recording gauges usually consist of a collector above a funnel which passes collected rain into a receiving vessel. Important requirements are that the collector walls should be vertical inside and steeply bevelled outside. It should prevent rain splashing in or out by having a sufficiently deep wall and a funnel with steep sides (at least 45 degrees). The area of orifices should be consistent. The receiving vessel should have a narrow neck (to prevent evaporation losses). It is usual to use a larger vessel in the same gauge, where it is impractical to visit the gauge on a daily basis.

The measuring (calibration) vessel should be of clear glass with engraved graduations (usually at 0.1 mm intervals). The type of gauge that it is to be used with, should be clearly marked. To achieve accurate readings for small rainfall amounts, the base will be tapered. Dip rods are sometimes used instead of measuring vessels, but this is unusual.

Plastic rain gauges, often inverted cones in shape and marked with mm gradations, are available in some countries and can provide a good, cheap alternative to expensive standard rain gauges. However, three important facts should be recognized.

- They eventually degrade due exposure to UV light, after one or two seasons.**
- They cannot be recommended in areas of frequent hard frost.**
- Not all such gauges are produced to accurate specifications and the accuracy of the gradations should be checked and if necessary, calibrated before use.**
- They are difficult to install with the orifice exactly horizontal, using a single post.**

Installation of Non-recording Gauges

The location of the gauge is the primary consideration in obtaining accurate rainfall measurements and the most serious problem is wind turbulence.

Buildings, trees, fences produce eddies and reduce accuracy. Isolated obstructions

should not be closer than twice their height to the gauge (further away if possible). However, openings in woods and orchards are suitable places (so long as the trees are no closer than specified); they act as windbreaks and reduce violent air currents. Sloping ground should be avoided and surrounding vegetation should be cut low. In general, smooth artificial surfaces are not suitable as they tend cause splashing and may attain high surface temperatures.

It is very important that the height above ground level of gauge orifices should be the same at all sites. It is preferable that the orifice be as close to the ground as possible. Wind velocity increases with height and the catch of the gauge is reduced thereby underestimating rainfall, however gauges positioned at ground level are prone to in-splashing, which is also undesirable. The UK Meteorological Office recommends that gauges' orifices be located at 305 mm above ground level as a compromise to minimise both of these effects. The influence of wind on rain catch can be expected to be greater in areas of convective rainfall where air turbulence is inherent.

In difficult field conditions however, the practical limitations of accidental disturbance and vegetation growth can result in problems with gauges set close to ground level. From experience, a suitable height is between 0.8-1.2 m. In this range of heights the gauge should be clearly visible and vegetation growth between visits from project staff should not affect readings. A consistent height for all gauges based on local field conditions is probably more important than following hard and fast recommendations. Ground-level gauges can be seen at meteorological stations, but here constant attention can be given to vegetation clearance, animals should not be a problem and checks can easily be made on the equipment. This may not be so at field sites.

The stand supports that hold the gauge should be inexpensive, strong, durable, rigid and easy to build from local materials. Wood is best avoided due to susceptibility to termites and rot. Materials should be easy to transport into the field. Although a concrete base in which stand legs can be set or bolted seems sensible and is often advocated, in areas where animal damage is possible, it is better to install the stands with metal pegs or by digging the legs into the ground to an depth of 50 cm, which is usually adequate. This may lead to the toppling of a gauge if pushed by cattle, but it can be reset immediately and will not be out of operation until the stand is replaced and re-cemented. In all cases, the gauge should be set horizontally by testing with a spirit level.

Gauges may often be placed on private land and it is essential to obtain permission from the owner before doing so. Usually a discussion as to the aims of the project, the (realistic) long-term benefits that are hoped for, and the purpose of the gauge are sufficient to obtain permission. Sometimes and if suitable, land owners can be recruited as gauge readers. It is important to provide an enclosure for the rain gauge to reduce the risk of damage from animals and to prevent interference with normal land use. Barbed wire is most suitable, with strands placed closer together (10-15 cm apart) at the bottom. Not only does this prevent access to animals and deter vandals, but also has no discernible effect on rainfall catch. Cut thorn tree branches (e.g. Acacia species) woven into the lowest metre of the fence are a good deterrent against goats and other agile animals, although in areas of termite activity this will need to be replaced each season. For the same reason, enclosure posts are best made of metal, though they are more expensive than wood. If they are too costly or unavailable, wooden posts will last several seasons if soaked in creosote or other preservative. In some areas, termite-resistant tree species can be found for use.

Even though wind turbulence is important in reducing rain gauge accuracy, the use of windshields can generally be discounted. Their main aim is to reduce wind speed, turbulence and splash over the gauge and allow siting close to the ground, but in practice they are not easy to make locally and increase problems of interference by people and animals. Research has shown that differences in shielded and unshielded gauges are relatively small (2-8%).

When locating rain gauges in the field it is important to keep in mind the problems of theft and vandalism. Prevention can be aided by the provision of fencing and enclosures, but the safest action is to enlist the help of local people. Sites placed by the homes of gauge readers are usually convenient and secure. When positioned in more public places, such as common village land, it is advisable to discuss the purpose of the equipment at an open meeting, where the use of rainfall data can be explained. An agreement for safe custody of the equipment can (usually) be made without difficulty.

The UK Met Office and US Weather Bureau recommend that rain gauges in exposed positions be located within a circular turf wall to negate the effects of wind turbulence. Figure 4.3 illustrates the construction of the protective wall which should be kept in a good, clean condition. Note that the rain gauge orifice is level with the top of the wall. It should be stated that in many circumstances, recourse to such a structure will be very difficult.

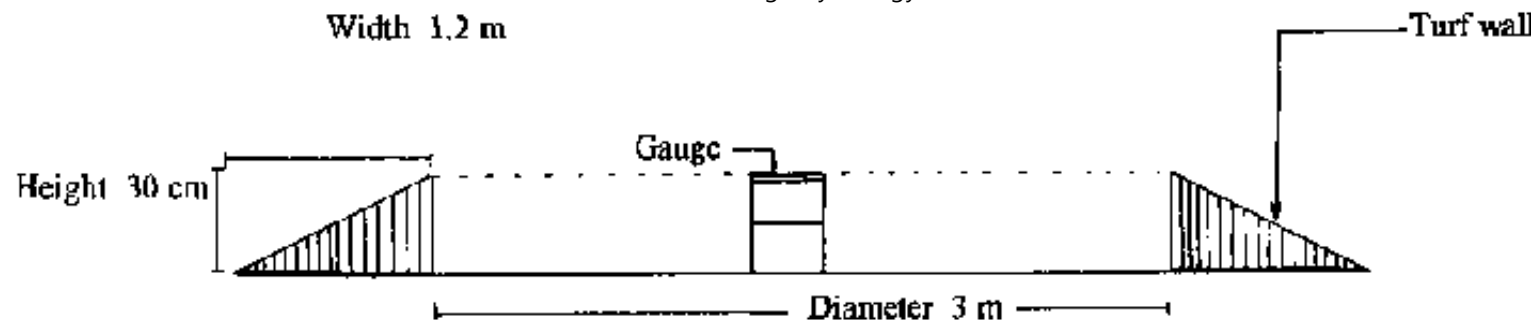


Figure 4.3 Construction of Turf Wall to Remove the Effect of Wind

Non-Recording Rain Gauges: Routine Data Collection and Site Staff

At field sites that do not have automatic recording gauges, locally-hired gauge readers will almost certainly be required and it is essential that a good working relationship be established and maintained with such gauge readers, observers or site guards. More difficulties in the collection of field data arise from people than from instruments and reliable field staff are an invaluable asset to any project. The selection of suitable field staff can be difficult and common sense and judgement of personality are important criteria in arriving at a suitable choice. The opportunities to recruit will vary from country to country and will depend on social as well as educational factors. The following points can help.

- The person should be reliable and suited for the job. They should be literate and numerate at least to a basic level. It is unlikely that they will need skills sufficient to understand instrument manuals (this can actually be a disadvantage if they are "zealous" in the performance of their duties), but they will be using printed record sheets and measuring and recording numerical values.**

- Village school teachers are often used as recorders of rainfall, by national meteorological services. However, they have other responsibilities and often take holidays away from their village. Rainfall records often suffer as a result.**
- If possible, it is useful to have someone who can speak your own language, or an interpreter will always be necessary.**
- A permanent resident is preferable, even if less educated. Their new responsibilities should not be in conflict with their normal day-to-day business.**
- Be aware that the recruitment can give rise to discontent within the family/community by imparting status and/or financial gain to the individual.**
- It is important to resist the recruitment of individuals who may be using their status within the community to acquire more and/or who expect financial gain, without being prepared to carry out the work properly.**
- The gauge reader should be able to reach the gauge easily, therefore they should live close by and have permission for access if it is sited on someone else's land.**
- After selection, the reader should be given a thorough explanation of why the data are being collected and the importance to the village and project.**
- They should be tested for competence in all their responsibilities.**

A contract should be drawn up covering all duties and rewards in a simple but comprehensive list. For example:

- Reading the gauge at specified times (check that this can be done, or at least that the time of reading can be noted).**
- Keeping the record book and measuring vessel at a specified place so that they can be checked on field visits.**
- Cleaning the gauge when necessary.**
- Noting damage to the gauge, enclosure and keeping down vegetation etc.**
- Repairing any such damage when possible.**
- Keeping the site clear of vegetation on a routine basis.**
- Informing in advance the use of the deputy and expected departures from site.**
- Basic pay, holidays or pay in lieu.**
- Deductions or action in the event of neglect of duties.**
- Best method of contact in the case of unforeseen events.**

Any seasonal changes in these duties should be stated, as should a clause covering unsatisfactory performance. A suitable deputy should be appointed as it is inevitable that at some time the gauge reader will have to leave the site. The

contract should be translated into the local language if necessary. It is best if payment is made for the services of the gauge reader. This puts the arrangement on a business footing and no favours are asked by either side.

Payment should reflect local employment conditions, bearing in mind that while the work is not arduous, it does restrict the mobility of the gauge reader. All payments should be recorded, copied and signed for without failure. Most of these points will be influenced by local conditions, but a friendly involvement by the project field staff in the reader's work coupled with a direct and business-like approach seem to get best results.

The maintenance of non-recording gauges is straight forward and should be carried out by the gauge reader. The gauge should be checked for any blockage (cobwebs, insects, leaves etc.) in the funnel at each reading by holding the orifice component to the light. Even partial blockages should be cleared. The collection vessel should be seen to be watertight and clean and the felt washer on the funnel of the gauge which fits over the vessel mouth (and helps to prevent evaporation) intact. The measuring (calibration) vessel should be clean and in good repair. It is useful to have a spare at site as these are made of glass. It should be ensured that the gauge is correctly seated on or in its stand, often gauges are disturbed when the tightly-fitting collection component is removed to measure the rainfall

Station/site **Belmopan Met Station.** Gauge Reader..... **R. Mpie**
 Month..... **July**
 Year..... **1988**

(1)	(2)	(3)	(4)
Date	Time	Rain mm	Remarks
01	08:04	0.0	Routine grass cut.....
02.	08:07	14.2
.
.
31.	07:58	8.9	Leave 1 day. Read by A. Sele

Monthly Total..... **34.8**..... mm

Read by..... **R. Mpie**

Collected and Checked by **S. Mpalomo**..... Record to be thrown back 1 day YES NO

Figure 4.4: Routine Data Collection Sheet, Non-Recording Rain Gauges

The stand should be checked for stability and deterioration and repairs effected if necessary. The area around the gauge should be kept clear of vegetation and the enclosure fence kept in good condition. Any repairs to or re-instatement of the gauge, including the date of original installation should be noted in the record book. The field team should carry a book identical to the site record book, with the same headings and columns, so that information can be copied from one to the other with minimum error. A hardback notebook, protected from the weather with stitched pages (rather than a file with loose, detachable pages) is best. Columns to be set out in a form similar to those in Figure 4.4, in addition to the name and address (and telephone number if appropriate) of the project, contact name etc. being clearly printed, in more than one language if necessary, on the book.

Field teams should routinely carry certain items such as a tool kit for impromptu repairs to gauges and stands (depending on the types used), spare record book, spare measuring glass, pay sheets, etc.

It is important to remember that meteorological offices "throw back" daily rainfall records, i.e. to attribute rainfall recorded on a morning to the previous day. This may not be appropriate for some studies.

4.1.2 Recording (Intensity) Rain Gauges

There are three main types of recording rain gauge systems: Tipping bucket, Syphon and Weighing. Recent advances have led to the use of electronic loggers which now frequently replace the usual chart and pen clockwork systems.

Alternatively, very compact intensity gauges measuring to a precision of 1 mm of rain and reputedly accurate to 2% over 2 years are available, though the cost of this equipment is not low. A liquid crystal display readout is given via a connecting cable and daily, weekly and monthly accumulated totals may be collected.

a. Tipping Bucket Gauge

These are commonly seen as in Figure 4.5, below. There are many different types and the manufacturers manual must be followed carefully, as is the case for all recording rain gauges. A dual tipping bucket pivots on a horizontal axis which lies beneath the funnel of the orifice, such that only one bucket receives rainfall at a time. When filled to a preset, calibrated amount (for example 0.2 mm) the bucket tips and is emptied, leaving the second bucket to receive rain. Tips are recorded

electronically and individually.

Data are downloaded and analysed by computer software, though hard copies used for manual analysis are sometimes available via a portable printer carried to the field. Alternatively, the data may be recorded on a mechanically driven chart. In general the tipping mechanism works well, but sometimes does not register very light rain in hot climates. It may also under-register during very intense storms, because of the finite time taken for the buckets to exchange positions.

Electronic logger type

Advantages:

- Simple download directly to a computer.**
- With computer programs, the analysis of intensity data is quick and easy.**
- No problems with mechanical clock, ink, pen etc.**
- Very large amounts of data can be stored (32 - 120 kb)**
- Options for storing data, time, etc. only when rain occurs, thereby saving memory.**
- Can operate for very long periods.**

Disadvantages:

- Some designs are new and may not be well field tested.**

- If the logger or battery fails, then all the data can be lost.**
- Needs computer facilities at base (good electricity supply, etc.).**
- Needs spare loggers or portable computer, both of which are expensive.**

Maintenance

Buckets must move freely and be oiled on a regular basis in some cases.

Mechanism must be checked frequently to ensure that it is horizontal.

Battery and connections must be tested at each visit and replacements made when necessary. Gauges come precalibrated, but they must be re-calibrated at the end of each season in the laboratory. This is a simple process whereby known volumes are emptied into the gauge from a pipette and checked against the record. Check with the manufacturer's instructions.

The tipping motion closes an electrical contact (usually current is provided by a 6 volt dry battery) which registers a pulse on an electrical counter or logger, each pulse representing the bucket contents (the example 0.2 mm of rain). The data are usually in the form of each tip represented by a time (Month: Day: Minute: Second). The opportunity to record the gauge name/number may or may not be offered, so it is wise to keep a careful note of from where the logger came. A 'record only with rain' facility is usual.

Various methods of down-loading can be used. In some instances the logger must be removed and taken to base to be down-loaded via an interface and computer. A replacement must be provided. In some cases the data can be downloaded in to a portable computer and the logger can remain at site. In all cases some sort of set-

up procedure is necessary to re-activate the logger once the data are extracted.

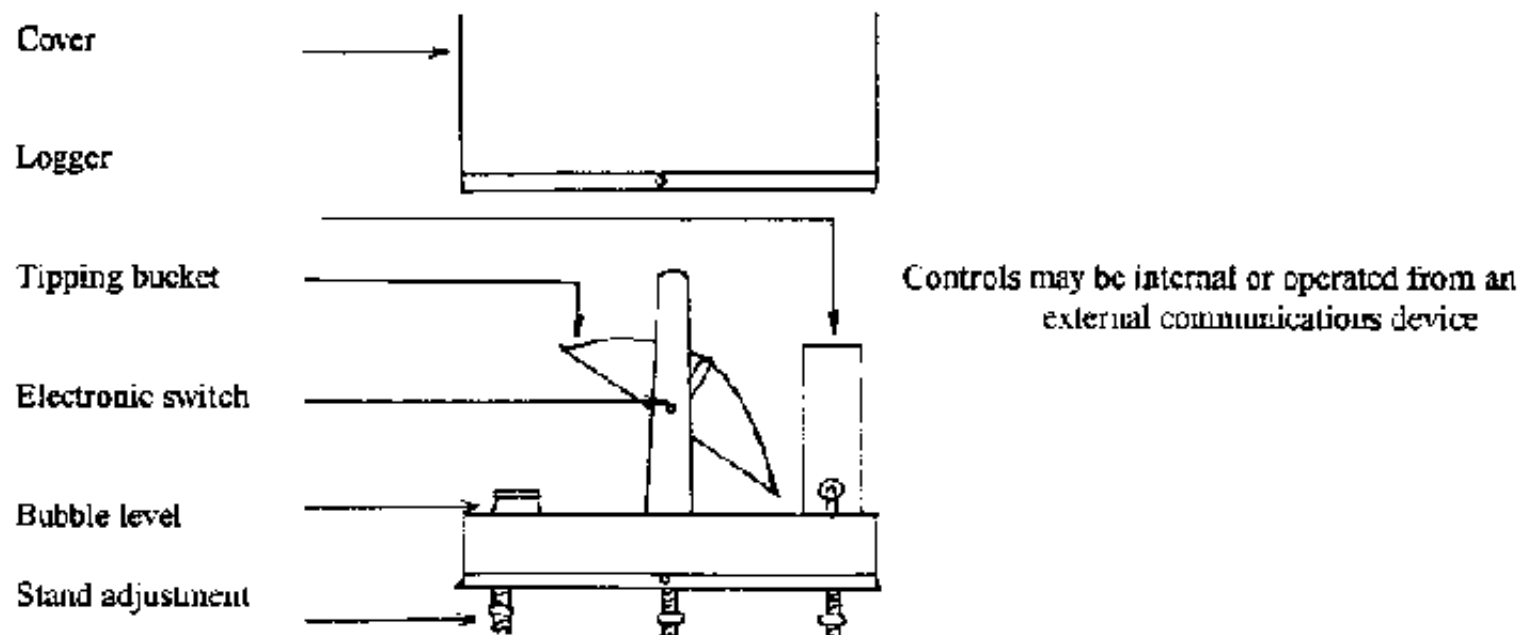


Figure 4.5: Tipping Bucket Rain gauge, Electronic Logger Type

Mechanical (Clockwork drum and chart)

Some gauges provide a chart record of the tipping instead of an electronic recorder. A permanent pen record is kept of each tip on a clock driven drum chart. Clocks usually work for about 30 days without attention, but this can be altered in most cases, by replacement of parts of the gearing mechanism.

Advantages:

- Permanent record is kept on chart, therefore cannot be lost.**
- Less likely to be affected by adverse conditions**

Disadvantages:

- Analysis of data is lengthy and must be entered manually into computer storage.**
- Needs more frequent visits**
- Not as flexible in terms of alteration of the instrument settings**

Some recorders offer both electronic and mechanical records. This gives a good back-up facility and some models even record river levels simultaneously. In some cases the tipped water runs to waste, but some gauges have the provision for the rainfall to be collected in a vessel below the gauge via a funnel and so the total rainfall for the period between readings is known. This can be very useful if the gauge or logger develop problems.

Maintenance

This is a little more complicated than for the electronic gauges, though the bucket check is the same. Clock accuracy must be tested on a regular basis, even though charts last 30 days without attention. Chart replacement should be done with an accurate pen reset and any malfunction noted. Chart drum motor may be clockwork or electrical.

b. Tilting Syphon Type

Versions of this type suitable for use in tropical countries are available. Figure 4.6 illustrates a typical syphon instrument.

Rain is collected and falls into chamber A, and raises float B. In response, the pen moves upward and its trace is recorded on the chart fixed to the drum, H. The chamber is on a pivot (C), over-balances when full and empties through the syphon tube (D). The pen is then reset to the zero position while lifted clear of the chart by the rod G. The over-balancing is controlled by the trip, E and the chamber is restored to its original position by F, the counterweight. The siphoning takes approximately 15 seconds.

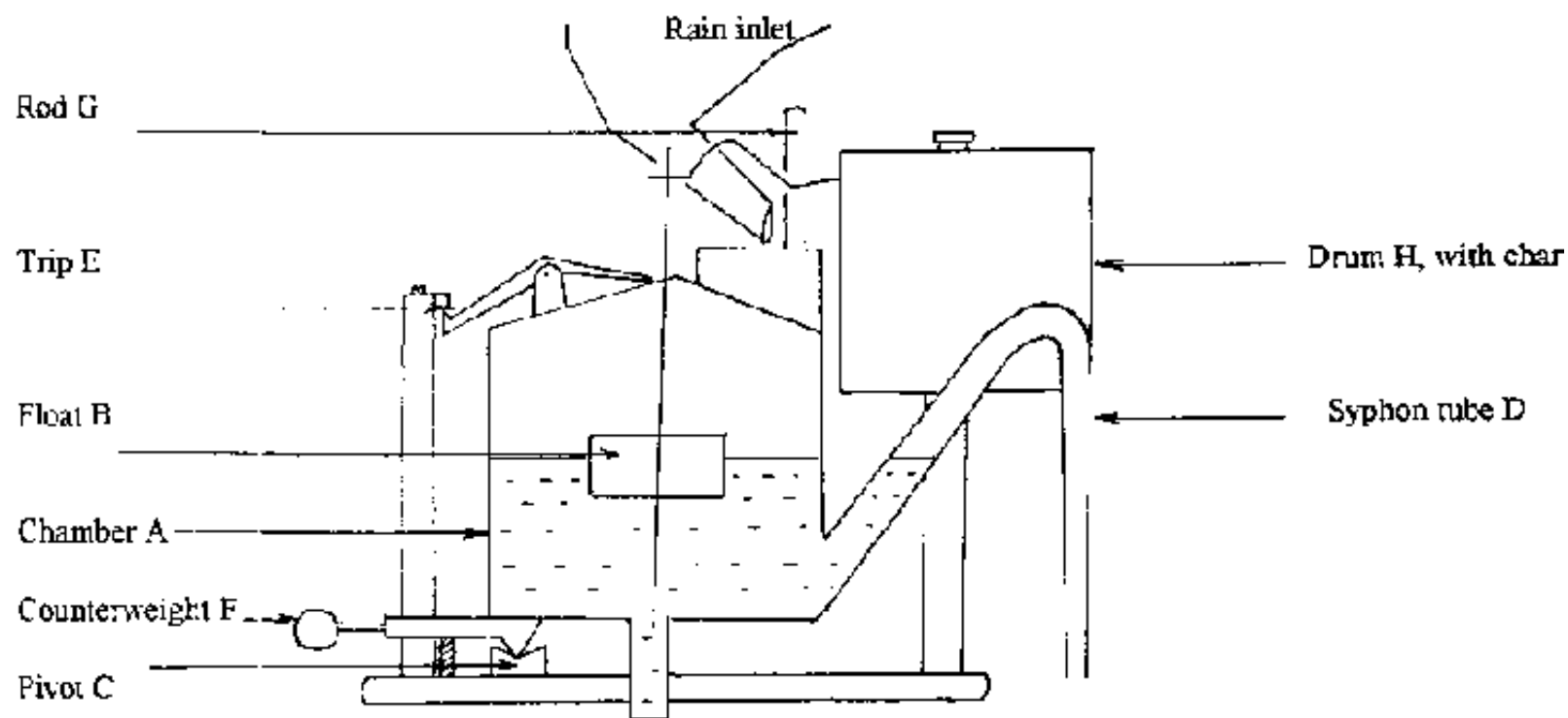


Figure 4.6: Tilting Syphon Rain Gauge

It is important to test the mechanism regularly by pouring in water through the inlet and to keep the syphon tube and its gauze filter clear of blockage at all times.

c. Weighing Type

This type is less common than the former kinds, but is especially useful where snow is frequently experienced. Precipitation is collected from a funnel into a bucket which, as the frame upon which it stands falls with increasing collection, stretches an isoelastic spring. The movement of the frame is proportional to precipitation and linked to a pen by a series of levers. This records on a clock-driven drum chart.

The maintenance of recording gauges will be the responsibility of visiting field teams who must be trained to a higher level than the field gauge reader responsible for daily gauges. Tools, loggers, charts, pens and ink will be carried routinely. Although gauges are of a type (syphon, tipping bucket, mechanical, electronic etc.), each will vary according to the manufactures' particular specifications and it is impossible here to list specific instructions for all gauges. In addition to the points of particular care noted in the sections describing the types of recording rain gauge, the manufacturers instruction manuals should be carried to the field and studied carefully.

Factors Affecting Accuracy of Rain gauges

Many factors affect the accuracy of rain gauges. These include evaporation, adhesion, inclination of the gauge, condensation and splash. However, these are unlikely to cause differences of more than about +/- 1%, whereas wind turbulence at a poorly sited and maintained station can account for much larger errors. The precise effects of wind speed is still contentious despite many years of research. Some authors predict large deficits (for example 17% with winds of 16

km hr⁻¹ and 60% at 48 km hr⁻¹) while others (see Figure 4.7 below) expect the effect of wind speed on rain gauge catch to be much less. Agreement that wind effects cause an underestimate of rainfall (and more especially snowfall) is universal, however.

Damage to the instrument during carriage should be avoided, especially denting about the orifice, which can cause discrepancies in readings. Splits and cracks in the receiving vessel can cause serious losses. Care should be taken to ensure that all the water is emptied into the measuring vessel and that the measurement is made accurately at the bottom of the meniscus.

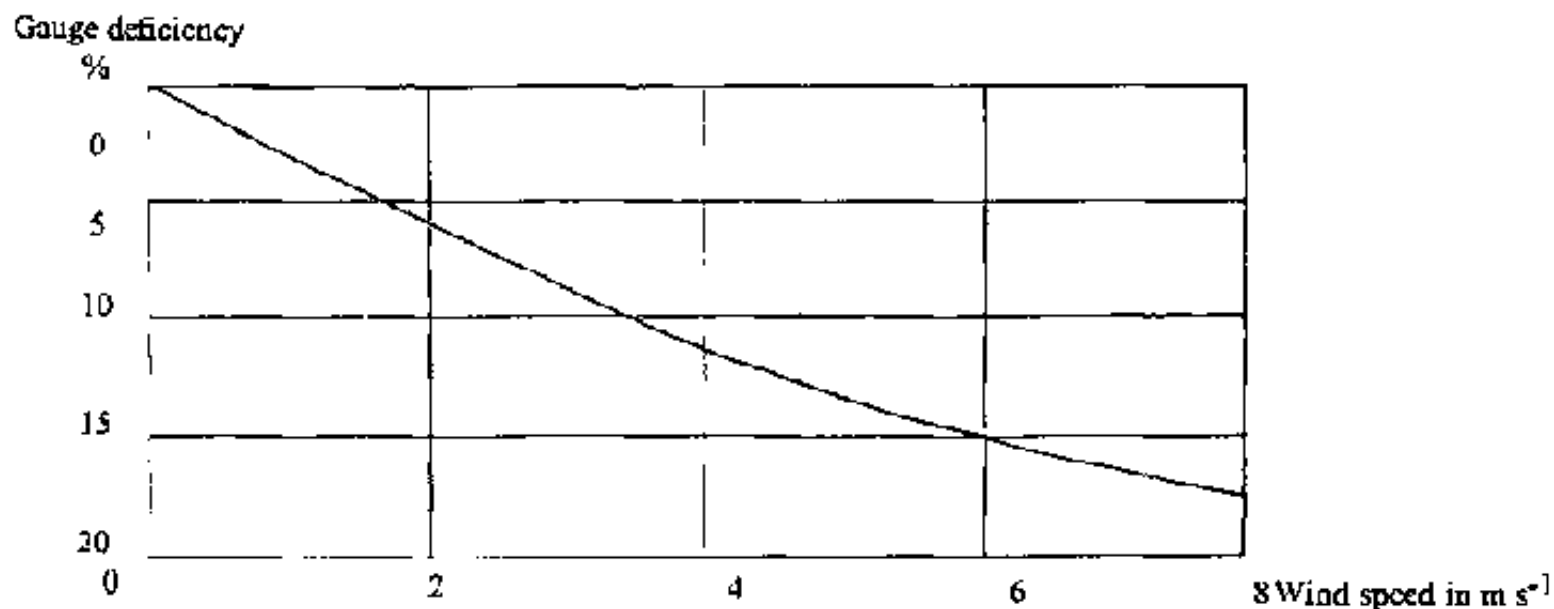


Figure 4.7: Catch versus Wind Speed

A list of errors and causes inherent in measuring rainfall from standard, non-recording gauges are given below. Other factors, notably poor maintenance, faulty resetting of loggers, pens and charts can affect the accuracy of rainfall records

obtained from intensity gauges.

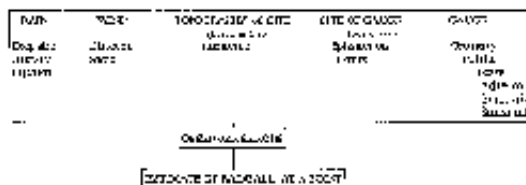


Figure 4.8: Errors in the Observation of Point Rainfall

Rain Gauge Networks

Many projects will be adequately served by the installation of rain gauges at each experimental site, when the interpolation of data between sites is unnecessary or can be easily achieved, perhaps using data from the national network. In some circumstances it may even be best to install two daily gauges, to allow for occasions when one gauge may be accidentally inoperative, though of course this must be balanced against cost. If the site area is large, these should be separated, with one gauge in the centre of the site and one at its boundary. When placed on a line at right angles to the prevailing wind, it is sometimes possible to collect information on the distribution of rainfall. If resources allow, the installation of a recording rain gauge at each site, in addition to at least one non-recording gauge, is to be preferred for agrohydrological purposes. Rainfall intensity is an important influence on runoff and its study will undoubtedly play an important part in the research agenda. However, recording rain gauges are very expensive (even "low cost" data-logger versions are currently several hundred pounds sterling, each) and project resources must be considered carefully.

If resources are inadequate for total coverage by recording gauges, then partial cover must be budgeted for. The success of partial cover will depend on a sound

instrumentation strategy, which can only be decided upon by the staff of each individual project. This will be dependent on the number of sites, their proximity to the base station and each other, the spatial variation of rainfall characteristics, the time between visits and field staff reliability. The two extreme options are:

1. to place all recording instruments at the most distant stations. This will provide event, daily and intensity data from widespread, infrequently visited sites which would otherwise give only rainfall totals from several days using non-recording gauges. However, it is wise to recognise that infrequently visited sites are always the most troublesome. Faults in and damage to the equipment will not be seen for some time, gauges can be interfered with or even stolen. Data and equipment could be totally lost. Access may be impossible at times during the wet season.

2. The second extreme option is to place all such valuable equipment at or near the base station. There is little danger of problems with the equipment not being rectified quickly, but the opportunity to collect data from diverse areas is lost. The best solution, clearly, lies between these two examples and to some extent trial and error (especially becoming familiar with the reliability of the gauges under field conditions) will be needed to determine which outlying stations are most suitable. It is essential to monitor one gauge carefully at the base station and check its readings and reliability of operation.

In many areas of convective rainfall, a statistical randomness means that over time, the average number of storms of a given intensity will be experienced at all locations within the study area. Thus it is reasonable to presume that rainfall

intensity can be extrapolated from one site to another, if certain characteristics of a rainstorm (for example the amount and duration of rain) are known. It is convenient if a statistically significant relation exists between rainfall amount and rainfall intensity and allows the substitution of one type of data for another. Figure 4.9 below shows 30 minute duration rainfall intensity against daily rainfall. The significance exceeds the 99.9% level. In some instances, a clustering into groups of data points may be seen with a strong correlation among them, probably indicating that different types of rainfall (for example low intensity frontal and high intensity local convectional) have been experienced. The 2 minute duration intensity against daily rainfall showed no significant relation for the data.

If a comprehensive raingauge network is proposed for an agrohydrological project, the number and density of instruments will depend on several factors. Those relating to the physical environment are:

- Size of area covered**
- Prevailing storm type**
- Topography and Aspect**
- Variation in seasonal rainfall**

In general, more gauges will be needed for large areas and denser networks will be needed where storms are convective and localised with high intensities (as opposed to cyclonic areas where rainfall tends to be widespread and of more uniform, low intensity). Convective rainfall is characterised by the predominance of thunderstorms. Mountainous areas, which create orographic rainfall, are

expected to have localised rainfall regimes and to need a more dense network than plateau areas (Table 4.1). However in practice, the rainfall in mountainous areas may be of a more regular distribution than extremes of elevation may suggest whereas flat plains dominated by very local convective rain storms often exhibit very large coefficients of variation of rainfall distribution.

It is important to plan with the hydrological characteristics of the area in mind. For instance it is more important to place a denser network of gauges in areas which contribute most to runoff, than in homogeneous areas which contribute little. It is possible to use correlation analysis in determining network densities. For instance if the correlation of daily rainfall between adjacent gauges is high (say, $r = 0.90$ or greater), a firm basis is provided to reduce the number of gauges in the network. Rainfall is spatially variable to a high degree and even the densest network of gauges can provide no more than an estimate of areal precipitation.

Networks are best planned in the preliminary stage of a project by the use of a map desk study to provide a picture of the overall pattern of gauge distribution. The distribution should not be random; random events are studied by a systematic arrangement of sampling points. Minor revisions of the network pattern can be made during installation, if unexpected problems of siting are found. It is useful to place some gauges outside the study area to ensure that extrapolation is possible to the boundaries of the study area. More gauges are needed if results are to be taken to other areas, rather than limited to the original study area.

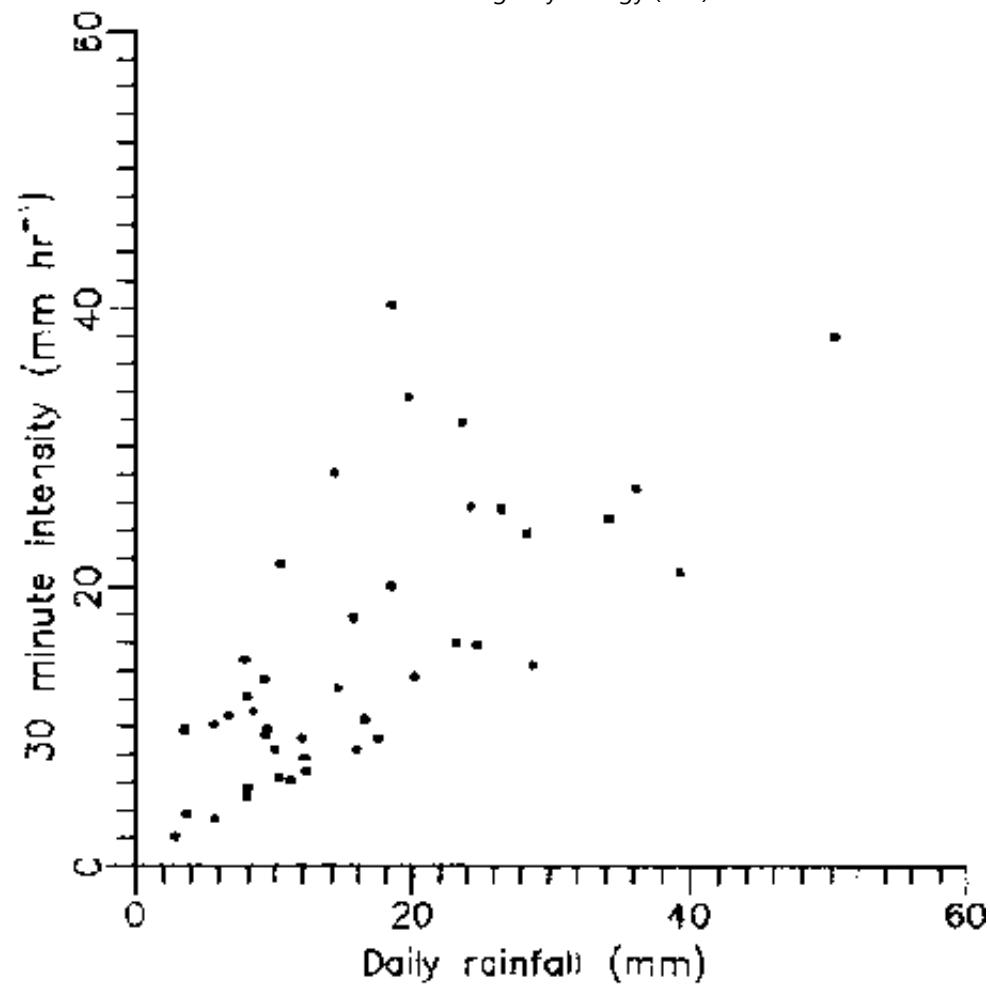


Figure 4.9: Daily Rainfall versus Rainfall Intensity

As stated above, project objectives and resources will determine, to a great extent, the level of instrumentation. A watershed study that relates total precipitation and total annual runoff yield would need fewer gauges than a study of rainfall on a storm-by-storm basis. The USDA regards the following gauge densities as suitable (see Table 4.1), but states clearly that the size of the study area is the only criterion used to determine them. Factors such as climate and

topography are not considered. These recommendations can be compared to those of WMO which take into account, in a limited way, climate and topography. However, it is likely that the needs of the project will be of paramount importance when compared to such general recommendations.

USDA		WMO	
Area	No. gauges	Climate/Topography	Area per Gauge
15 ha	2	Small mountainous island	25 square km
40 ha	3		
240 ha	4		
15 square km	10	Tropical mountainous region	100-250 square km
30 square km	15		
60 square km	20		
150 square km	30	Flat temperate, tropical mediterranean area	600-900 square km
250 square km	50		
1000 square km	100		
2500 square km	300	Arid and Polar regions	1500-10,000 square km

Table 4.1 Recommended Density of Daily Rain Gauges by Size of Study Area

4.2 Other meteorological data

Rainfall data collected for hydrological purposes will also be useful to other project members, such as agronomists and soil physicists. The same is true of other meteorological data which may be important, for example in assessing crop performance under varying climatic conditions. These other data will help categorise climate in general and are essential to estimate evapotranspiration and soil moisture conditions which can have an important effect on runoff production, soil moisture availability and crop growth. Indeed, the calculation of evapotranspiration is one the most important uses to which comprehensive

climatic data are put during agrohydrological investigations. This chapter concentrates on instrumentation; the uses and analysis of data are covered in chapter 8.

Site of Meteorological Stations

The installation of climatic instruments requires a suitable site which should be representative of the macroclimate of the study area. Where climate varies greatly, perhaps due to topography, several stations may be necessary, though the spatial variability of some meteorological variables is greater than others. The site should not be in an exposed position on a steep slope, nor should it be within the distance of four times the height of any nearby trees or buildings. In semi-arid areas, sparsely vegetated open areas make good, representative sites. The site will probably represent the greatest concentration of instruments for a project and it is essential that a suitable, secure location be selected. Advice on instrumentation should be sought from the local Meteorological service and where possible instruments of the same manufacture should be acquired.

The site can be instrumented according to particular need, but special attention should be paid to such details as shading from elevated posts and other instruments. Doors to equipment should be away from direct sunlight, areas of artificial surfaces should be kept to a minimum. The area should be well fenced and gated, not higher than 1m, with wire mesh which is fine at the foot of the fence, to deter animals. Birds can be prevented from roosting on instruments by the provision of alternative high perches. Fence posts should be of metal to avoid termite damage. It is useful to retain extra space within the compound, for instruments that may be added at a later date. Where possible recording

instruments should be used. Experience will show that records obtained manually, during holidays and weekends often appear suspiciously inconsistent when compared to weekday readings.

Records of equipment should be kept secure at the station. Loose-leafed books are more prone to damage and loss than those with permanent bindings. A site map of water pipes, cables etc. is useful if the station acquires permanent buildings. Longitude and latitude should be noted on any such map. Records of instruments added or removed are very useful as are schedules for routine repair, painting and grass mowing. Files should be kept which contain notes on the instruments; when bought, invoices, serial numbers, calibration tests, instruction manuals, repairs etc. Having this information easily at hand can save a great deal of time and frustration. Such details should be kept separate from the day-to-day records of measurements, and in a secure place.

4.2.1 Air Temperature

Air temperature is one of the most commonly measured meteorological variables. Maximum and minimum temperatures are used to calculate mean daily values for use in evapotranspiration estimations. Figure 4.10 illustrates the installation of maximum/minimum thermometers.

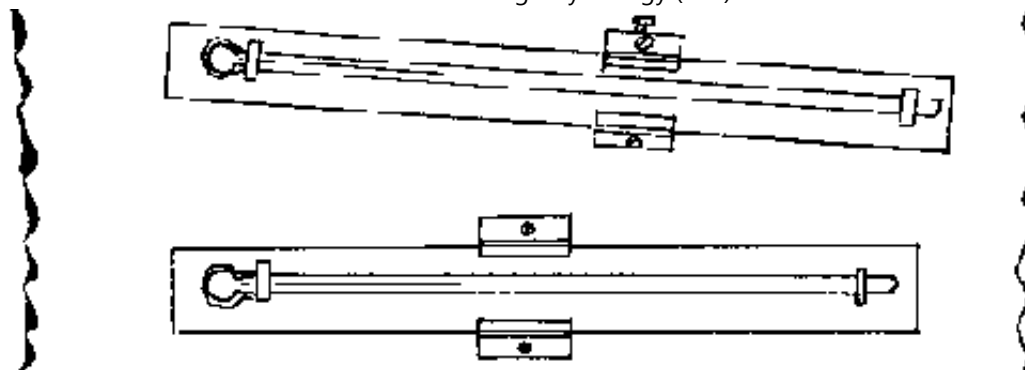


Figure 4.10: Maximum and Minimum Thermometers

They should be housed within a screened building or box.

The temperature readings are obtained from two separate thermometers. The maximum thermometer is of mercury in glass, secured so that the bulb end is 5° above horizontal. The minimum thermometer is alcohol-filled, with the bulb end about 5° below the horizontal.

As temperature drops, the alcohol retreats into the bulb, inducing an index (a small, dark dumbbell) to move within the bore of the thermometer, until the minimum temperature is reached. When the temperature rises, this index is left behind to give the minimum reading. This reading is taken at the end of the index furthest away from the alcohol-filled bulb. Both temperature readings are usually taken at about 08:00 each day and the instruments are then reset. In the case of the mercury-filled maximum thermometer, a rise in temperature forces the expanding mercury through a constriction above the reservoir. The mercury cannot return when the temperature falls and the maximum temperature is shown. The mercury must be shaken gently back into place after the reading is taken.

4.2.2 Humidity

The moisture status of the air has a strong influence on rates of soil evapotranspiration (E_t) and open water evaporation, both of which are greater when the humidity of the air is low. Relative humidity values are widely used in evapotranspiration equations and two main methods are used to measure humidity:

The first uses thermodynamic principles and measures temperature differences between wet and dry thermometers ("psychrometers"). They are set together, with the wet thermometer slightly lower than the dry and are usually housed as one unit in a secure metal frame. Around the bulb of the wet thermometer is placed a wick sheath, which trails into a container of clean, distilled water. The wick should fit tightly; dust, dirt and insects are sometimes a problem and the wick may need replacement or cleaning each week. The simplest and most common type of psychrometer (sometimes also called a "hygrometer") is housed in a screened box. The natural flow of air around the wet bulb thermometer results in it registering a lower temperature than the dry bulb thermometer. The use of psychrometric tables converts the readings into dew point temperatures and relative humidity (RH). Hand-held versions are available for spot readings, these being whirled around on a handle to encourage ventilation; others use the assistance of electric fans to achieve this effect.

In the second case the instrument uses the hygroscopic properties of a material (usually human hair) to determine humidity, and is called a "hair hygrometer". A series of hairs expand and contract according to atmospheric moisture and oscillate a pen which marks a trace on a chart, moved by a clockwork drum.

Adjustments to the instrument can be made by altering the arrangement of linking levers.

Hygrographs are usually placed on the floor to ensure stability. Shelves used for such instruments increase the possibility of readings being affected by vibrations and for this reason the housing fabric should be strong and rigid. The chart can be annotated with date of chart replacement, station, reader etc. Checks for correct readings should be made against psychrometer values when the humidity is high (early morning or during a rainy period) and low (mid afternoon). Hairs that become dirty should be cleaned with a soft brush, but eventual replacement will be necessary. Very often this instrument is linked to a temperature sensor that gives a continuous record on the chart and can be used as a check or back-up to the maximum and minimum thermometers. In this case the instrument is called a hygrothermograph. Less costly, non-recording hair hygrometers are also available.

4.2.3 Wind Speed and Direction

Wind also has an important effect on levels of evaporation and evapotranspiration. It removes humid surface air layers from above land and water and can physically damage crops. Anemometers are used to measure wind speed and duration and thereby windrun, in km day⁻¹. A standard anemometer has three cups mounted at 120° to each other on a vertical axis. The movement of this rotor closes an electrical contact which measures and records a standard distance of wind movement. A continuous record of wind speed and direction is also provided. Wind direction is obtained by the operation of a single-panel vane. Pen and chart recorders are usual, but electronic recording of these data on data loggers is now

common.

A site that is relatively level is to be preferred, with no obstructions within 100 x the height of the nearest obstacle. The World Meteorological Organisation's recommended height is 10m for general speeds, but this height imposes the need for larger and more expensive mast structures. For use in the calculation of E_t values by the widely-used Penman method, 2m is recommended. Fortunately an estimate of wind speeds for levels other than of the instrument can be obtained by the formula:

$$u_2 = (\ln z_2 / \ln z_1) a u_1 \text{ where (4.1)}$$

u_2 is the estimate of wind speed

u_1 is the known speed at instrument height

z_1 and z_2 are the heights in cm of the known and estimated wind speeds

a is an exponent between 1 and 0.6 according to ground surface roughness

Empirically this can be stated as Hellmann's formula:

$$\text{Velocity at height 'h' / Velocity at 10 m} = 0.233 + 0.656 \log_{10} (h + 4.75) \text{ (4.2)}$$

Hand-held anemometers with digital displays of wind speed are available, but these do not incorporate a wind direction sensor. Relatively compact, portable systems that can be quickly assembled at site, can be purchased. It is useful to

note that accurate readings of wind speeds less than 5 km per hour are difficult to achieve.

4.2.4 Solar Radiation

Solar radiation provides energy for evaporation and plant development. Several methods of calculating E_t use solar radiation as a key parameter, often converted to net radiation, which takes into account the portion of solar radiation that is reflected back into the atmosphere. Total incoming shortwave radiation is measured by solarimeters, sometimes called pyranometers, which sense the intensity of radiation from the sun and sky, that falls in a horizontal plane.

The portion of all radiation that is transformed into other forms of energy is called "net radiation". Net radiation is measured as the difference between incoming (downward) and outgoing (upward) radiation of all wavelengths by the net radiometer. As radiation varies between night and day, counters can be linked to the radiation measuring device to record these values separately for easy reading. Electronic pulses may be recorded on a strip chart, but increasingly (especially with small, automatic meteorological stations) the data are recorded on an electronic logger and can be downloaded directly in digital form on to a computer, for viewing and analysis.

Sites should be clear of obstructions, with a view to the horizon that is not affected by nearby trees or buildings. Under no circumstances should shading occur and artificial surfaces that can direct radiation to the instrument should be avoided. The site should be typical of conditions under study, but compromise is inevitable where conditions of vegetation type and cover, soil reflection, etc. vary

from place to place within the local area. Placing the instrument high, perhaps at 3m, increases the field of reception which is useful in areas that are heterogeneous. Cultivated field situations are more likely to give representative values that rangeland areas which tend toward heterogeneity. As a guide, an instrument set at X metres above the ground will receive 90 and 99% of its upward flux from ground areas with radii 3X and 10X respectively. Figure 4.12(b) shows a typical net radiometer that would be one component of a small, automatic weather station.

It is very important to keep the glass or plastic domes clean and undamaged. The presence of dust is a common problem. A photographer's air brush is very useful for cleaning, but if not available, soft tissues can be used. Care should be exercised as the domes are prone to scratching. The instrument should be kept horizontal at all times. Calibration is important because of deterioration of the domes and black reception faces and should be carried out every few months. However, this involves the use of a replacement radiometer to continue the record and it may only be possible to check the upper and lower sensors during an off-season period, when a break in the record may not be important. This is best done at a time of steady radiation, when the faces of the radiometer are inverted for 10-minute periods. Averages are taken and both faces should give readings within 5% of each other. Alternatively, a second instrument can be kept in good storage conditions and used only as a standard for the field instrument. As different ground conditions may be measured by the two adjacent instruments, it is as well to exchange their positions around to check the first results.

Solarimeters (pyranometers) measure only incoming short-wave radiation and are sometimes used at meteorological stations. These can be used to calibrate

radiometers. They should be shaded from direct solar radiation by placing the shadow of a black matte disc (about 1m away and held by a thin support) over the instruments. The response by both sensors should be the same such that $D_r / C_r = D_s / C_s$. As D_r and D_s , the change in response of the radiometer and solarimeter are measured (in mv) and C_s , the calibration constant of the solarimeter is known, C_r can be found. This should be done several times.

Sunshine:

Sunshine hours are commonly recorded where the cost and practicability of maintaining radiation meters are limiting. The widely-used Campbell-Stokes recorder consists of a glass sphere mounted on a pedestal which concentrates bright sunlight onto a chart and so burns a trace along it, thus sunlit periods are recorded. Instruments which are specified by ranges of operation latitude (for example 0° to 60° N or S) and the correct charts for the N or S hemisphere should be used.

Electronic meters that measure photosynthetically active radiation (400 - 700 nm) are also available.

4.2.5 Evaporation

Evaporation Pans:

The measurement of free surface water evaporation depends on air temperature, wind, humidity and solar radiation. It is a commonly measured index that integrates these meteorological factors and to some extent, illustrates the

behaviour of evaporation from water bodies and evapotranspiration from wet soil, where the availability of water is not limiting.

The most commonly used instrument, which is an international reference instrument, is the US Weather Bureau 1.22 m (4 foot diameter) A-pan. This can be purchased complete, or made from local material such as a suitable gauge, galvanised steel sheet. Pressed seams should not be allowed to cause buckling. Welded seams should be treated to prevent rusting. The pan should be mounted horizontally on a wooden platform with tamped soil below, to allow a 13 mm air space (half an inch). In humid climates the areas around the pan will be grass, whereas in arid areas, vegetation will come and go according to season. However, vegetation should not be allowed to grow above the level of the pan. Locations near areas with artificial surfaces, boggy areas or water surfaces should be avoided.

The water level in the pan can be measured by an inclined, graduated gauge staff, but accurate reading in this manner is more difficult than by using a stilling well and micrometer hook gauge. The level is measured with the hook tip, lowered below the water and then raised until the tip just pierces the surface. The mechanism is removed from the stilling well and the reading taken from the graduated vernier scale. Water levels should be kept at 5 cm below the rim of the pan (+/- 2.5 cm) and water should be added or removed to maintain this level. A reading should be taken before and after this has been done. The differences in daily levels give evaporation, with additions and removals of water and daily rainfall (measured nearby) being taken into account. Readings are taken at the same time each day, normally 08:00 hours, though alternatively a WLR could be used if the cost is not prohibitive.

Daily maximum and minimum temperatures of the pan water are often taken by floating thermometers, kept at least 30 cm from the side. Problems can occur with birds and animals drinking from the pan. Fences will keep out larger animals, but a wire screen fixed over the pan itself, may be necessary. This can effect readings by the suppression of evaporation and in semi-arid climates a correction of 16% is made to measurements. Algal growth can be prevented by a small addition of copper sulphate to the water and the pan should be kept clean of debris and insects. Figure 4.13 shows a hook gauge version of the Type A evaporation pan. Data may be lost during periods of heavy rainfall and over-topping.

Lysimeters:

Lysimeters most commonly measure evapotranspiration by changes in the weight of containers filled with soil, to which water is added. Losses by evapotranspiration are then calculated. Lysimeters can be very large, weighing several tons, while others used in field locations may only measure water losses from a few kilogrammes of soil. Crops and vegetation may or may not be grown in them. Large containers are weighed by permanent pressure transducers, though the construction of large lysimeters is normally beyond the scope of many agrohydrological projects and is not practicable under field conditions. Meteorological services may find it useful to install them.

Evapotranspiration from small lysimeters is measured by their removal and these instruments are more commonly used for field research. Suitable ones can be made from PVC water pipe with a diameter of 15 cm and a length of 20 30 cm. They are filled by attaching a steel cutting edge to the lower edge of the plastic, and a metal ring to the top. The latter prevents damage when they are hammered

into the ground at the required location, when soils are at field capacity and drainage has ceased. Jacking lysimeters into the ground may be necessary, but the appropriate equipment must be available.

The lysimeter is removed and the soil is retained by a wire mesh (5 mm is suitable), which is screwed into the base. They are then replaced into their holes which have been fitted with tubes, a few millimetres larger in diameter and deeper than the lysimeters. Lysimeters are removed and weighed every hour or so throughout the day. Because the cores are isolated from root activity, new cores should be taken every 2 to 3 days, to account for plant extraction.

There are other problems of representing true conditions with lysimeters: filling with soil can disturb the profile, edge-effects are great especially with small models, isolation leads to hydraulic continuity being lost at the sides and affected at the base. Intermittent, unexpected rainstorms can affect readings.

4.2.6 Soil Temperatures

Soil temperatures have direct effects on the germination and root growth of crops and natural vegetation, the state of which can greatly affect runoff. Soil temperatures determine the micro climate of the overlying air and are important for assessing the growing environment of crops. Soil temperatures are not only dependent on incoming and outgoing radiation, but also on the thermal properties of the soil which can change greatly with the addition of water by rain and its removal by evapotranspiration. Temperatures reach maximum some time after local noon and minimum after midnight.

Continuous records of temperature can be taken with thermocouples (thermographs) linked to pen and chart or electronic loggers. Thermocouples should be calibrated with mercury thermometers at least twice a season, at the beginning and end, and any corrections must be noted. Bent-stem mercury thermometers for 10 and 20 cm depths and encased mercury types with bulbs in crystalline wax and suspended in steel tubes for 50 and 100 cm depths, are used where recording instruments are not available. These depths are recommended by WMO. Readings are normally taken at 08:00, 14:00 and 20:00 Local Time. Good contact with the soil is necessary and accidental trampling should be prevented.

4.2.7 Automatic Weather Stations

Research applications may demand the collection of climatic information at sites in addition to base stations. In such cases the use of automatic weather stations may be more suitable than an array of individual instruments. Electronic logging is used to keep records (usually on a multi-channel data logger) and to avoid the need for frequent visits. The period between visits is determined by the number of instruments used, the frequency of record and the memory size of the logger.

Different formats for the presentation of data will be used according to manufacturer.

Station Agr Res Sta., Sebele.....											
Longitude ..36° 24'E.....						Latitude ..24° 14' N.....					
Month.....Apr'l.....			Year.....1987.....			Compiled By...K. Lesey.....			Checked...D Smith.....		
Day	Max air Temp °C	Min air Temp °C	Avg air Temp °C	Avg R.H. %	Dew point Temp °C	Wind at 10m km	Wind at Pan km	Net Rad Lym	Solar Rad Lyn	Pan Evap mm	Rainfall Precip mm
1	17	10	13.5	78	10	42	38	112	130	2.0	0.8
2
3
.
.
31	20	13	16.5	65	8	30	25	156	178	4.2	0.0
Total	2006	1379	1692.5	206.7	1394	1678	1483	6781	9004	87	75.9
Average	64.7	44.5	54.6	66.7	45.0	54.1	47.8	218.7	290.5	2.8	2.4

Figure 4.14 Example Meteorological Data sheet

Stations should be enclosed by fences in a suitable position, as discussed in the sections on individual instruments. Considerable thought should be given to the possible problems of vandalism and theft because of the cost of automatic weather stations and the ease with which the array of instruments can be damaged.

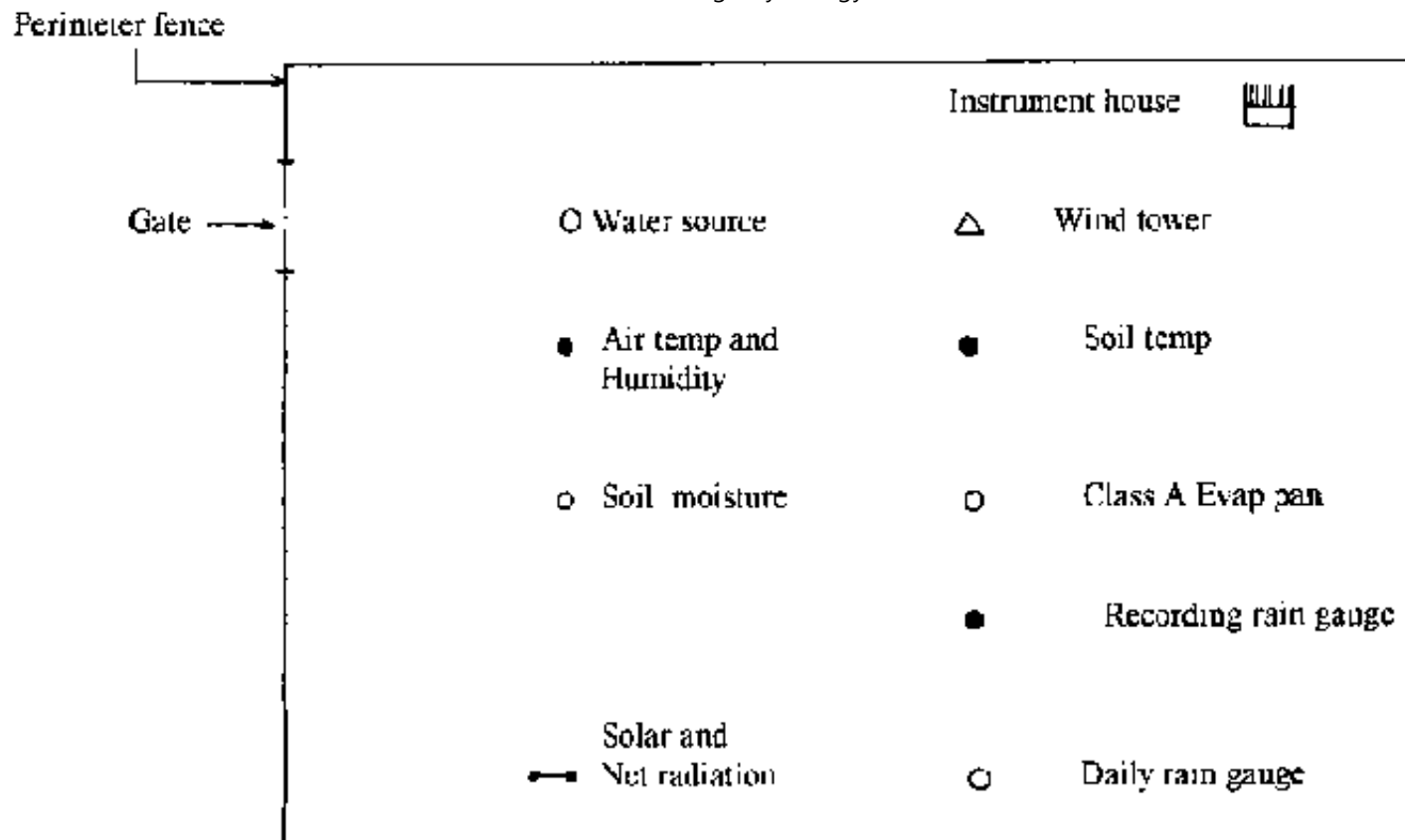


Figure 4.15: Typical Weather Station Layout

Figure 4.14 shows an example data sheet for the meteorological variables discussed above.

Figure 4.15 shows a plan of suitable weather station site, equipped with a basic list of individually installed instruments. Note that sufficient space is left within the compound to accommodate instruments that may be installed at a later date.

It is important to select automatic weather stations according to particular project needs, for example some stations place anemometers below the recommended 10

m elevation, and if wind speed and direction are important factors in the research agenda, this may not be suitable. Automatic weather stations can be very cost effective when their prices are compared to those of collections of individual instruments. Generally, the seven following meteorological parameters are measured:

- rainfall and relative humidity**
- air and soil temperature**
- wind speed and direction**
- solar radiation.**

Equipment costs

All costs of locally made equipment are approximate. The costs of raw materials and especially labour are highly variable from country to country, but a good idea of cost magnitude can be gained from the figures quoted below. The costs of manufactured equipment are based on 1993 prices. Shipping, agents' fees and fluctuations in exchange rate cannot be taken into account.

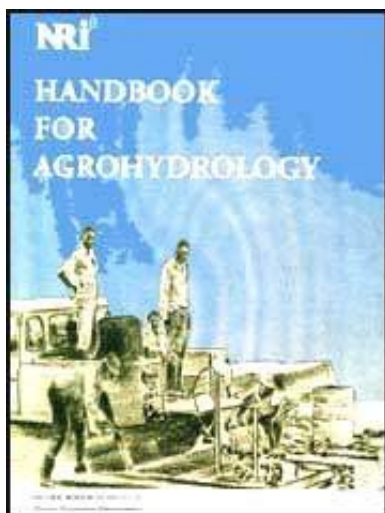
Item	Quantity	Typical Approximate cost in \$ US
Locally made Equipment		
Raingauge stands	1	20 - 50
Enclosure	1	20 -50
Manufactured Equipment		
Daily raingauges		
Brass	1	250 - 500
Plastic	1	5 - 10

Intensity raingauges		
Tipping bucket, Electronic with logger etc.	1	1500 - 2500
Extra logger	1	500 - 750
Logger reader	1	500 - 750
Analysis software	1	500 - 750
Tipping bucket, Chart	1	1500 - 2500
Weighing	1	not available
Syphon	1	2000 - 3000
Extra charts	1 year	20 - 40
Evaporation (hook gauge) pan	1	800 - 1200
Floating thermometer	1	100 - 150
Humidity		
Wet and dry bulb psychrometer	1	150 - 200
Hair (non-recording) hygrograph	1	50 - 75
Thermohygrograph	1	600 - 900
Solar radiation		
(Campbell - Stokes with 1 year's charts)	1	1500 - 2000
Wind speed		
Hand-held anemometer	1	200 - 300
Wind direction		
Anemometer and direction with battery/mains display unit	1	2000 - 3000
Air temperature thermometers	1	200 - 300
Soil temperature thermometers	1	200 - 300
Automatic weather station with	1	7000 - 8000
Logger		
Air temperature		
Soil temperature		
Rainfall		
Relative humidity		

Solar radiation
Wind speed and direction

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Handbook for Agrohydrology (NRI)

Chapter 5: Soils and soil moisture data

Soil physical, chemical and moisture properties constitute a study in their own right and it is possible that any agrohydrological or water harvesting project may have available the services of a soil specialist, but this is not always the case. The effects of soil physical properties on hydrological behaviour are very important.

Four main aspects of soils and their influence on runoff and agriculture are considered. These are:

- 1. The physical and textural nature of soils which are influential in determining runoff.**
- 2. The soil moisture status which can also influence runoff and control water availability for crops.**
- 3. How to measure soil moisture.**
- 4. The influence of these soil factors on the process of infiltration, the ability of soils to absorb water.**

In many respects soil textures and soil moisture status are closely linked; the physical characteristics of soils may change with the addition or removal of water, while the physical characteristics of soils will determine their ability to absorb and retain rainfall. In terms of the study of soils for agrohdrological purposes and the quantification of soil characteristics, it is most convenient to study these aspects separately. Methods of determining infiltration, which is strongly influenced by texture and moisture status, are also discussed.

5.1. Soil classification and soil textures

5.1.1 Soil Horizons and Their Characteristics

The soil profile, as exposed by the side of a pit is usually divided into 3 horizons which are frequently further divided into sub-horizons:

A horizon constitutes the top soil, where any organic matter is found and within which cultivation is initiated. B horizon is the subsoil, without organic matter. C horizon which is composed of weathered rock, usually the parent material.

Soil pits, dug to give an exposure of the soil to the C horizon where possible, provide a great deal of information which is used in the classification of the soil types. From the agrohydrological viewpoint however, it is the practical effects on farming and hydrology of such factors as the effective depth of soil (that is the depth that can provide a medium for roots) that are important. In most cases the effective depth is limited by the nature of parent material and the manner in which it has weathered; climatic influences are often strong. In other cases, gravel bands may be present and if tightly bound, will restrict the development of crop roots. Such bands should be noted as the limit of the effective depth. Roots may be evident in partially weathered parent material but it is unlikely that they contribute much to the intake of crop water and nutrients. Information on parent material, erosion, formation history and climate, indicate past periods of waterlogging and other aspects of the nature of the soil moisture reserve.

Topsoils

Of particular importance is the character of the top 20 cm or so of soil. This is the soil layer that influences soil surface/rainfall relations by its texture and aeration, and represents the approximate depth of cultivation. The top soil layer also determines structural stability, fertility, and the tendency for a soil to cap or erode. There are obvious limitations to digging large numbers of pits in order to determine soil characteristics; the job is a long and arduous one and pits must usually be filled in after examination. The textural definition of surface soils is

therefore more commonly assessed by working the soil by hand, when wet. Where an accurate textural analysis of soils is needed, samples are taken and analysed in the laboratory (see chapter 3). Table 5.1 below lists the characteristics of soil textural types when manipulated.

Sandy soils have high rates of infiltration and percent runoff is usually low. They tend to be infertile, relatively acid and prone to leaching. At the other extreme, clay textured soils give high percent runoff in general, though cracking vertisol soils may absorb water until the clay particles swell, the cracks close and runoff results from later rain. Fine textured soils normally have a higher water holding capacity than coarse, sandy soils and their chemical mix is more varied and nutritious for plants.

Texture	Stickiness	Characteristics
Sand (S)	Not sticky	Loose, cannot be moulded.
Loamy Sand (LS)	Very slightly sticky	Can be moulded into a ball, cannot be rolled into a ribbon between the fingers.
Sandy Clay Loam (SCL)	Slightly sticky	Can be rolled into a short ribbon between the fingers but this cannot be bent without breaking. Will not take a shine nor show a finger print when squeezed.
Clay Loam (CL)	Sticky when sufficiently moist	Can be rolled into a ribbon and be bent into a half circle before breaking. Will take a shine and show a finger print.
Sandy Clay (SC)	Very sticky	Can be rolled into a ribbon and bent almost to a circle before breaking. Takes a good shine with many sand grains showing.
Clay (C)	Very sticky	Can be rolled into a ribbon which can be bent into a circle before breaking. Takes a strong shine with few sand grains showing.
Heavy Clay (HC)	Very sticky	Very difficult to break up and work. Stiff, sticky.

Table 5.1: Soil Textures According to Manipulation When Wet

Soil depths are also important; whatever the inherent water holding capacity of soils on a unit volume basis, the absolute volume of water available to crops will be small if soils are shallow. This is an important consideration when the viability of water harvesting opportunities is being assessed, as it will be a critical factor in determining how frequently water must be added to the soil moisture reserve.

Soil textures are determined precisely and classified most rigorously in the laboratory as described in chapter 3. The FAO has now adopted the USDA soil classification triangle which categorises soils into textural types according to the percentage of silt, sand and clay components, and is shown in Figure 5.1. The "International" classification (Figure 5.2) is now used in few countries. Relatively small differences exist between them. Several approaches can be taken to the selection and collection of soil samples. Spatially, soil textures can be highly variable, so that when top soil samples are collected to assess the general textural type of a large area (for instance a whole field), samples are taken at individual points and combined well before submission for analysis. If the spatial variation of soil textures is in itself a characteristic under investigation, samples should be taken systematically on a marked grid basis, with each sample given a point reference number accordingly. This avoids subjective sampling.

Where microtopographical features are under study, sampling should take place along defined transects at every one or two metres, according to transect length. Again, the samples are referenced to the sample points and also to the elevations above a base level (a levelling survey will be necessary). Wash-ins, ploughing, crops, vegetation and faunal activity may be recorded. Loose samples (not cores) are collected and sealed in polythene bags and the depths to which they are taken are noted. See chapter 3 for details regarding the dispatch of soil samples.

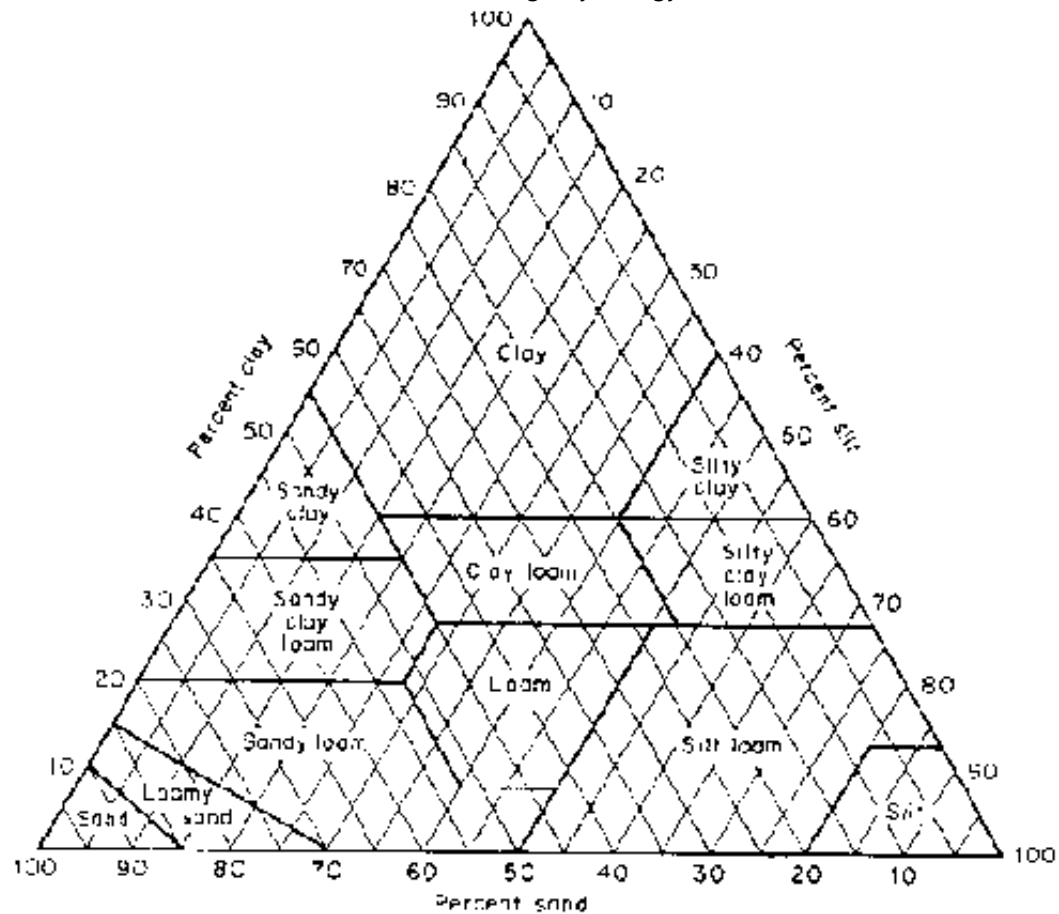


Figure 5.1: FAO/USDA Soil Classification Triangle

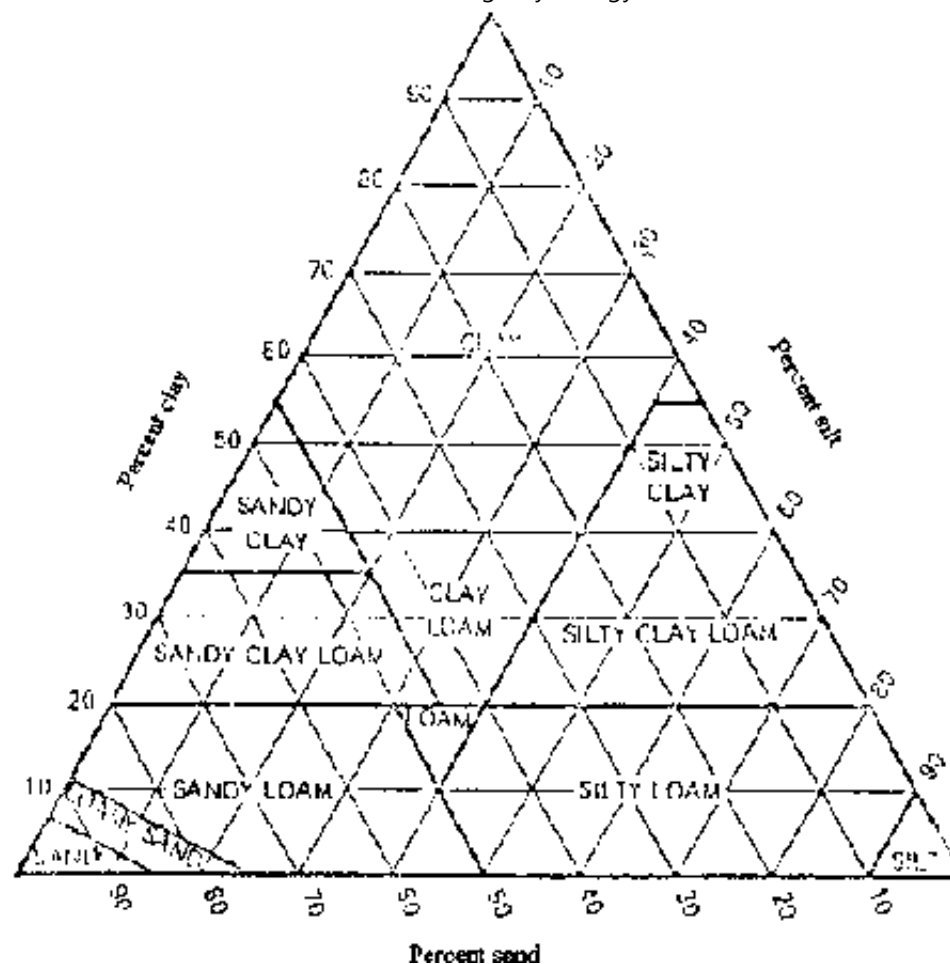


Figure 5.2: International Soil Classification Triangle

Subsoils

Subsoils affect soil water permeability and thereby runoff. In the field, permeability is usually assessed by the observation of soil physical characteristics rather than direct or laboratory measurement of hydraulic conductivity. Common terms used in descriptions are:

Compacted: Firm or hard consistency, close packing of particles resulting in a dense material with reduced pore space.

Cemented: Hard and brittle, soils which do not soften with prolonged moistening.

Deflocculated: Soils in which sodium has entered the exchange complex and dispersed the colloids. This leads to reductions in pore space, aeration and permeability. High levels of pH and electrical conductivity are found. Columnar horizons which are hard and dense may be found.

The colour of soils may give information on aeration and drainage and are described according to the standard Munsell notation. Colours may vary between and within horizons, for example:

Drainage:	Reds	Well drained
	Yellows, Greys	Poorly drained
Organic Matter:	Browns & Blacks	High in organic matter
Leaching:	Paler	More leached
	Darker	Less leached and higher mineral fertility

Wet soil colours are usually darker and there may be the presence or absence of mottles. The colours on Munsell charts that provide the standard reference (described in detail below), are arranged to give the three variables used to define all colours and are recorded in a standard order:

Hue: The dominant spectral colour (increase in redness or yellowness).

Value: The lightness of colour and total amount of light reflected.

Chroma: The purity or strength of colour (increases with a reduction of greyness)

On each card the colours are of a constant hue. The colours increase in lightness vertically and in equally visible steps. The colours increase in chrome to the right and become greyer to the left. In the field, a 1 cm fragment is selected from the sub soil, untainted by organic matter. After deciding whether it is predominantly yellow or red, a colour chart is selected and the sample compared through the most appropriate hole in the chart. Intermediate matches are not uncommon. Check that the hue is correct. Avoid sweat on the colour charts (not always easy).

Mottles, very pale and very dark colours indicate reduced permeability or groundwater near the surface. Rust-coloured mottles along root channels suggest periodic waterlogging, as does an abrupt change from reddish to greyish colouration. Grey mottles in an otherwise reddish weathered rock zone indicate a seasonal water table.

Bulk Density

Soil texture is largely responsible for the bulk density of soils, that is the weight per unit volume, most commonly expressed as g cm^{-3} . Imperial units of lb ft^{-3} may be seen. Bulk densities are found by comparing the oven dry weight of samples and their volume. Samples are taken from soil pits using standard soil sampling cores, driven into the exposed face below the top soil when the soil is neither very wet nor completely dry. The sample must not be disturbed, so as to maintain its original volume. The sample should be oven dried at $105\text{ }^{\circ}\text{C}$ to $110\text{ }^{\circ}\text{C}$

and weighed to the nearest 0.1 gram.

The bulk density (sometimes called the "specific weight") dry weight of sample/ volume of sample The bulk density in g cm⁻³ can be converted to lb ft⁻³ by multiplying by the factor 62.4.

Soils with high bulk densities have a paucity of pore space, impede root penetration, make cultivation difficult and promote runoff.

5.1.2 Pedological Classification

The pedological classification of soils, although basically created with agriculture in mind, is described only briefly here. It is relatively complex and includes an extremely wide range of soil types. Many of the terms and names derive from the Russian language. Soil surveys and maps use the orthodoxy of pedological classification, but in developing countries soil mapping is usually at an early stage or restricted to localities of special interest. Map scales are commonly 1: 250,000 to 1:1,000,000 and cannot be expected to depict the variability of soil types with accuracy.

The pedological classification of soils is broken into two main groups: Higher and Lower categories. Of the higher categories, the nature of Zonal soils depends greatly on the prevailing climate at the time of formation. Intrazonal soils not only are influenced by climate, but also localised conditions, for instance poor drainage, and therefore cross the boundaries of zones. Azonal soils such as lithosols (rocky) and regosols (dry sandy) are not zonal.

In arid and semi-arid regions, regosols, lithosols and lateritic soils (which are red

and have a high iron oxide and aluminium hydroxide content) are commonly found. Variation in soil types is wide and intrazonal soils may commonly occur due to changes in local conditions of geology and drainage. Calcareous bands may be common at depth.

Glei are indicative of impeded drainage and a rising and falling of the water table. These mottled colourations may be red, yellow or brown when the water table is low, or grey or blue when it is high, resulting from the oxidisation or non-oxidisation of iron and manganese.

Higher Categories

There are three main orders of soils (Zonal, Intrazonal and Azonal) which are subdivided into Suborders and Great Soil Groups:

Zonal Soils Suborder Great Soil Groups

1. Cold zone	Tundra,
2. Light coloured arid zone	Desert, Red desert, Sierozem,
	Brown/Reddish-brown soils
3. Dark coloured soils of semiarid sub humid and humid grasslands	Chestnut, Reddish-chestnut, Chernozem,
	Prairie, Reddish prairie soils
4. Forest grassland transition	Degraded Chernozem and Noncalcic brown soils
5. Light coloured timbered regions	Grey wooded or grey podzolic soils

6. Laterite soils of forested warm temperate and tropical regions	Reddish brown and Yellow brown lateritic and Laterite soils
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Intrazonal soils

1. Halomorphic (saline and alkali)	Saline, Soloth, Solonetz soils soils of imperfectly drained arid regions	
2. Hydromorphic soils of marshes	Humic-glei, Low humic glei, Bog, and swamps	Groundwater packed soils
3. Calcimorphic soils	Brown forest soils	
Azonal soils		Lithosols, Regosols, Alluvial soils

Further classification is beyond the scope of this book. The Unified System, developed in the USA, is concerned with the engineering aspects of soil classification, rather than agriculture.

Lower Orders

The Great soil groups are subdivided into Soil Series and then Types. Series are soils developed from the same parent material and soils within a series have the same profile characteristics except for the texture of the surface layer. Types are determined by the texture of the A horizon. Soil Phases are determined by deviation from the norm, for example a stony phase.

5.2. Soil moisture

The soil moisture content of a soil is of primary importance. Soil moisture is expressed either in percent by weight (P_W) or volume (P_V) . The relations are:

$$P_W = (W_W - W_d / W_d) \times 100 \text{ and (5.1)}$$

$$P_V = (V_W / V_S) \times 100 \text{ (5.2)}$$

respectively, where the subscripts w and d are wet and dry samples and V_s is the volume of the sample.

Percent weight of water is the most common (gravimetric) determination and is found by using samples obtained from the field. Samples can be taken with shovels, augers or soil sampling cores. Samples are best transported for immediate drying in electric ovens at 105° C in sealed cans to prevent moisture loss; this temperature removes all moisture, without driving off other volatile matter.

Balances should weigh to the nearest 0.1 g and samples should each weigh at least 100 am. With a typical weight of moisture of the sample being 20 g, the accuracy of measurement will thus be approximately 0.5 %. Several samples will be needed and areas used continually for sampling may suffer. Soil heterogeneity can be a problem, though as discussed previously, the manner in which samples are collected determines the extent of this difficulty.

The usefulness of volumetric determinations of soil moisture content lies in their easy conversion to surface units. This conversion allows comparison with rainfall and irrigation applications, although percent volume determinations are not usually obtained by direct sampling. The volume of water from soils could be

estimated by determining the weight of moisture and converting to volume assuming a specific gravity of 1.0 for water, though the volume of soil is difficult to measure. Field determination of soil moisture on a volume basis is normally found using a neutron probe. This is method discussed later.

5.2.1 Soil Moisture Potential

The soil moisture potential (SMP) represents the thermodynamic energy status of a soil and is conventionally expressed in units of bars (1 bar - 10^6 dynes cm^{-2}). Two particularly important specific points of soil moisture conditions are field capacity and wilting point. Field capacity is the condition whereby moisture is retained after the gravity drainage of a saturated soil is complete and the soil moisture tension is equal to one third of an atmosphere. Wilting point is the condition beyond which plants can no longer extract water and is taken to be a tension of 15 atmospheres. Soil moisture between these two points is regarded as that available to plants.

Total SMP is composed of three components:

Potential energy due to the force of gravity, osmotic potential and capillary potential. The latter is by far the most important and is assumed to be more or less equal to the total soil moisture potential. Equipment is available to measure this variable in the field.

Since many occasions arise when either the soil moisture content (SMC) or capillary potential can be measured, a relation between them is desirable; this could be used to describe the SMP. Unfortunately, a unique relation does not exist

for most soils and the moisture content depends not only on capillary potential, but also on previous soil moisture history. The effect of this is that two main soil moisture relations exist (see Figure 5.3), one for the dewatering of the soil (the soil moisture retention curve) and another for rewetting. These soil moisture/suction conditions are called the "hysteresis loop" and it is often represented in graphical form.

In many situations it is suitable to refer only to the dewatering branch of the hysteresis loop since this has the most profound effect on plant growth. It is obtained and applied under drying conditions. In very many cases these relations are unique to the soil, though some generalisation is permitted and related to field (and sometimes project) conditions. In some soils the two branches of the hysteresis loop may be relatively close and it can be assumed that they are the same for all practical purposes, though this is not usually the case. Table 5.2 gives typical soil moisture values for various soil types.

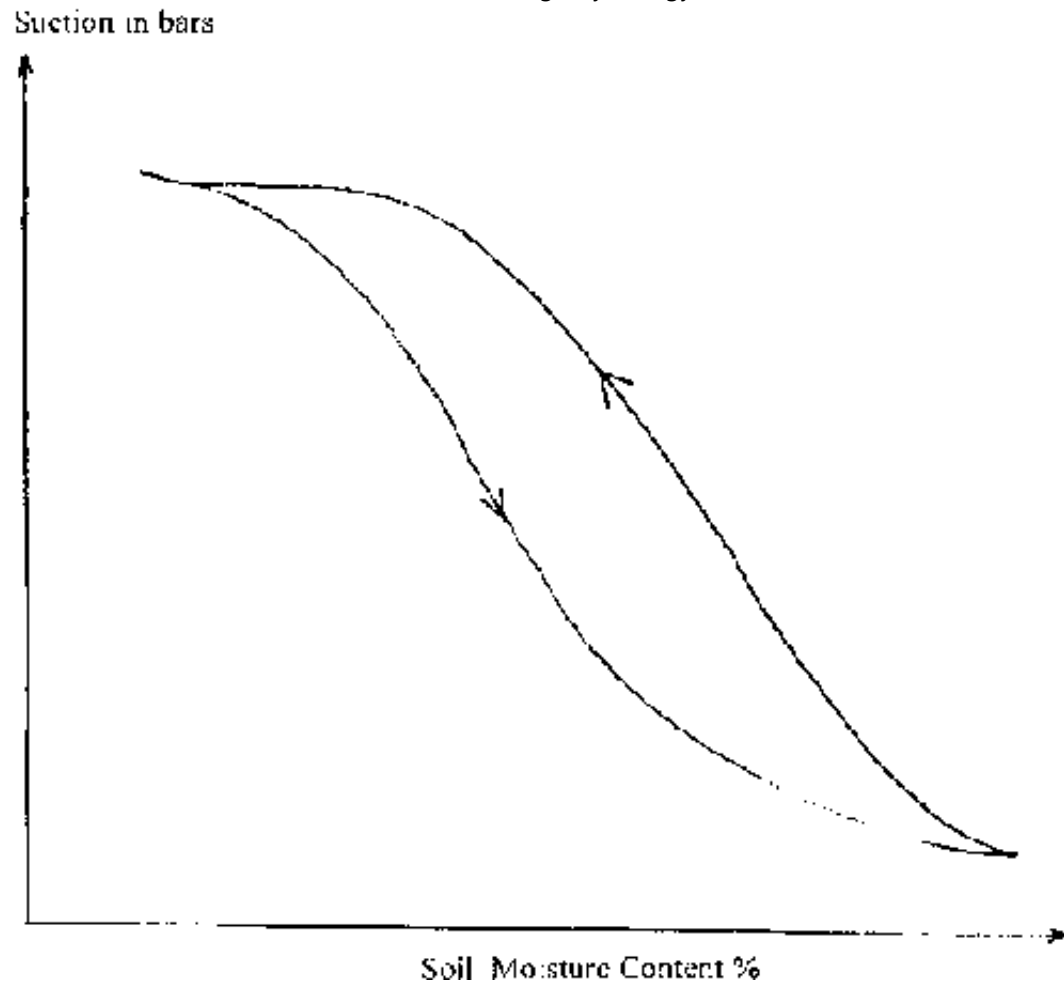


Figure 5.3: Typical Soil Hysteresis Loop

Soil type	% Dry weight of soil			Density kg m ⁻³
	Field capacity	Wilting point	Available water	
Sand	5	2	3	1520
Sandy Loam	12	5	7	1440
Loam	19	10	9	1360
Silty Loam	22	13	9	1280
Clay Loam	24	15	9	1280
Clay	36	20	16	1200
Peat	140	75	65	400

Table 5.2: Typical Moisture Values for Various Soil Types

Table 5.3 below gives a field guide for judging how much of the available water has been removed from different soils.

Equipment for the measurement of SMP, with the exception of the neutron probe, usually consists of a material that is placed in the soil to reach equilibrium with the soil moisture and as such measures capillary potential, not SMC. Care should be exercised in the use of this equipment since the hysteresis behaviour makes the step from capillary potential to SMC problematic.

The soil retention curve is sometimes called the soil characteristic and is found by laboratory analysis. Tensiometers can be used in the field, but the limit of the suction pressure that they exert is very low (about 0.8 bar), whereas the use of a pressure plate apparatus in the laboratory gives a much wider range and water content can be found by weighing the sample at each stage of dewatering. The soil moisture retention curve can then be plotted (Soil moisture in % versus suction in bars), using the data points.

With regard to sampling, spatial variability in soils is the norm and the

representative nature of sites that are selected will be limited. Microtopography, runoff, the lateral flow water within horizons and land use are some of the factors that affect soil moisture variability. A number of sites will be needed within a "homogeneous " area and the extent of study will depend on the aims and resources of a particular project. Knowledge of the degree of soil variability is in itself a useful tool in assessing the place of water harvesting and agrohydrological research in the agricultural agenda. A survey at the outset of a project, that is as comprehensive as resources permit, will usually be extremely rewarding. Where possible, work that has been undertaken previously by soil surveys and land use planning organisations should be consulted.

Soil Moisture Deficiency	Feel and Appearance of Soil and Approximate Deficiency in mm per 30 cm of soil			
	Coarse Texture	Moderately Coarse	Medium Texture	Fine and Very Fine Texture
0% Field Capacity	No free water on squeezing but wet outline on hand. 0.0 mm	No free water on squeezing but wet outline on hand. 0.0 mm	No free water on squeezing but wet outline on hand. 0.0 mm	No free water on squeezing but wet outline on hand. 0.0 mm
0 - 25%	Tends to stick together slightly may form ball on squeezing. 0.0 - 5.0 mm	Forms weak ball, breaks easily, will not slick. 0.0 - 10.0 mm	Forms ball, is very pliable, slicks readily if high in clay. 0.0 - 12.5 mm	Easily ribbons out between fingers, has slick feeling. 0.0 - 15.0 mm
>25 - 50%	Appears to be dry, will not form ball if squeezed. 5.0 - 10.0 mm	Tends to ball when squeezed, but seldom holds together. 10.0 - 20.0 mm	Forms plastic ball, will sometimes slick with pressure. 12.5 - 25.0 mm	Forms ball, ribbons out between thumb and forefinger. 15.0 - 30.0 mm
>50 - 75%	Appears to be dry, will not form a ball. 12.5 - 20.0 mm	Appears to be dry, will not form a ball. 20.0 - 30.0 mm	Somewhat crumbly, but holds together when squeezed. 25.0 - 38.0 mm	Somewhat pliable, but will form ball when squeezed. 30.0 - 48.0 mm
>75 - 100% (100% equals wilting point)	Dry, loose. Single grains flow through fingers. 20.0 - 25.0 mm	Dry, loose. Flows through fingers. Easily broken down. 30.0 - 38.0 mm	Powdery, dry sometimes crusted, crumbs on surface. 38.0 - 50.0 mm	Hard, baked, cracked, some loose 48.0 - 63.4 mm

Table 5.3: Field Guide for Judging Available Soil Moisture

5.2.2 In situ Methods of Soil Moisture Measurement

a. Tensiometers

Tensiometers are used to measure capillary potential, the sensing elements are usually porous ceramic membranes or pots. Usually a water and mercury manometer is attached to these membranes to measure potentials, though dial type gauges can be used. The manometers are housed to prevent weather damage (especially sunlight and high temperature) and a set may have perhaps six or ten sensors, each placed at different depths, to cover the possible range of plant rooting.

Various manufactures of tensiometers are available and the instruction manual should be followed carefully. The tensiometer must be saturated with water to work properly and is installed after the membrane has been boiled to remove gases, filled with boiled water and transported to the field wrapped in wet rags or in a container of water. To protect against damage the clear plastic tubing, sealed into the porous pot and placed in a plastic pipe of suitable internal diameter (usually about 2 cm) and length, is lowered into a pre-prepared hole. To ensure good hydraulic contact with the soil, some of the excavated soil from the hole is mixed with water and poured in as a slurry, to act as a seating. Careful back-filling of the hole with the soil is necessary to avoid a depression at the surface. Time must elapse before the slurry dries out, which if below the rooting zone, may take several weeks. Figure 5. 4 shows the installation of mercury manometer tensiometers

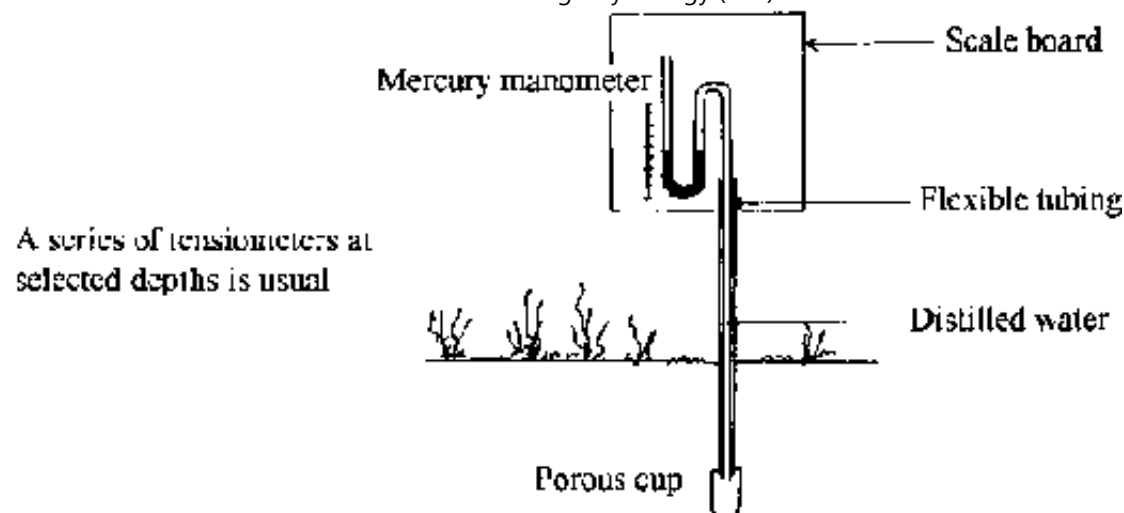


Figure 5.4: Mercury Manometer Tensiometers

The tubing is charged using boiled (degassed) water with a large syringe to prevent air bubbles. As the porous pot is in contact with aerated soil however, it is likely that bubbles in the tubing may be a recurrent problem. Modest housing fixed to metal poles with a hinged door for access and painted white to reflect heat is adequate. Installations at every 20 cm depth are suitable.

Points to note:

- **Tensiometers are used for monitoring moist soils because of their narrow range of sensitivity.**
- **Boiled water is always used for filling and flushing.**
- **Theoretically, tensiometers cannot measure negative pressures greater than one atmosphere, but in practice their limit is less than this, about 0.8 of one atmosphere, greater tensions will encourage air to enter the system.**

- In very arid conditions, appreciable volumes of water may be passed through the membrane and may affect soil conditions.**
- Tensiometers are subject to thermal variations and it is best if readings can be made during early morning, several times a week, depending on conditions.**
- Adequate regard to routine monitoring and rainfall conditions must be given for remote field sites. Flushing with boiled water to remove air bubbles (which make the manometer operate incorrectly) must be part of the routine.**
- Vacuum gauges may be used instead of mercury manometers. They are more robust, but may be less sensitive.**

b. Electrical Resistance Method

This method of measurement involves the use of blocks of porous material, usually gypsum (calcium sulphate), though sometimes units of fibreglass or nylon construction are preferred. The block material will tend toward a potential equilibrium with the surrounding medium. They are placed in good contact with the soil and the electrical resistance of the block gives an estimate of soil moisture content. Gypsum blocks are best buffered against saline soils, although they are less sensitive to changes at high moisture contents and generally deteriorate more quickly than the fibreglass and nylon alternatives. The blocks come provided with electrical connections in the form of wires or coaxial cables.

Before installation they should be saturated and a resistance reading taken for

reference. They are installed in the same manner as tensiometers, but a shallow horizontal trench should be dug with a slope away from the blocks, in which the wires can be laid, to avoid water being directed downwards them and resulting in incorrect readings. Though of simple construction and relatively cheap, the main disadvantage of these devices is their relative insensitivity to changes in the high soil moisture content range.

Points to note:

- They can be used in much drier soils than tensiometers.**
- Gypsum blocks operate best at tensions 1 to 15 atmospheres (drier soils).**
- They will last for 2 or 3 seasons.**
- Nylon and fibreglass operate best at tensions of less than 2 atmospheres, but they are more expensive and are sensitive to salts.**
- The materials are relatively cheap, but difficulties include unobserved deterioration while underground, attack by chemicals with consequent errors and a possible drift of the calibration curve.**
- More than one unit can be used per installation hole, but great care must be exercised during emplacement.**
- The blocks must be calibrated in the laboratory.**

Calibration

Two methods of laboratory calibration are in general use. The first is to place the resistance block in a small container surrounded by soil that is initially saturated and to allow the soil to dry out gradually, reaching various levels of moisture content, each of which is determined by weighing the whole system. At each level, a reading of electrical resistance is also taken. The second method is preferable, but depends upon the availability of a pressure plate or pressure membrane apparatus. The resistance block is placed in a pad of soil in the pressure apparatus which is best equipped with the facility of electrical connection through its wall, thus allowing continuous monitoring of the block. The soil is initially saturated and when equilibrium is reached (water no longer flowing out) a resistance measurement is taken. The block is brought progressively, in steps, to various levels of desorption and the measurements of electrical resistance are repeated.

Where accurate and comprehensive field soil moisture measurements are to be made, the blocks should be calibrated for both wetting and drying, to overcome the problems of hysteresis. Soils of different textural characteristics must each be calibrated separately.

c. Neutron Probe Method of Soil Moisture Determination

Neutron probes emit fast neutrons from a small radioactive source (typically 50 - 100 millicurie mCi). The neutrons are slowed when they encounter hydrogen atoms in the soil and these slow neutrons are registered by a boron trifluoride detector integral to the instrument. Unaffected fast neutrons are not detected. The detection of slow neutrons is amplified and counted by a rate-scaler. As water is the main source of hydrogen atoms in the soil (though organic matter, boron and chlorine will also slow the neutrons), the number of slow neutrons reaching the detector is a function of soil moisture content. The neutron probe has a limited

diameter of detection of about 20 - 30 cm.

The neutron probe consists of the main body which houses the cable, clamps, rate-scaler and handles. The source and detector are lowered on the cable into an aluminium access tube in the ground (aluminium being more or less transparent to neutrons). Readings are taken at required depths. The base of the probe body usually has a shield to prevent inadvertent irradiation when the cable is wound in. Figure 5.5 below shows the main components of a neutron probe.

Access tubes

The observation holes that are used for neutron probes are made permanent by casing them with a thin walled material, usually aluminium. Aluminium is almost transparent to neutrons, is durable and can be purchased in varying diameters. The latter is a significant practical point as it is important to select diameters of tubes that give the practical minimum air gap between the probe and the access tube wall, while allowing unimpeded access to the source and detector. This air gap will affect the instrument's count-rate.

Commonly, a single piece of tube is used, with the upper end protruding above the soil surface. This not only aids location, but in many cases is used as a seat for the probe. It is essential to match the outside diameter of the access tube with the inset of the neutron probe or the diameter of the tubes will have to be modified at a later date. Example (but not exclusive) diameters are: tube outside 44.45 mm, thickness 2 mm, internal diameter 40.45 mm probe diameter 38.00 mm. The surface exposed portion of the access tube should be kept to an acceptable minimum but as

it is usually used as seat for the probe body, this portion should be adequate for the purpose. To prevent dirt and water from entering, a rubber bung is fitted. Covering this with a soft drinks can gives extra protection. Usually, condensation is not a problem, but where it is, small sacks of desiccant should be hung in the tube.

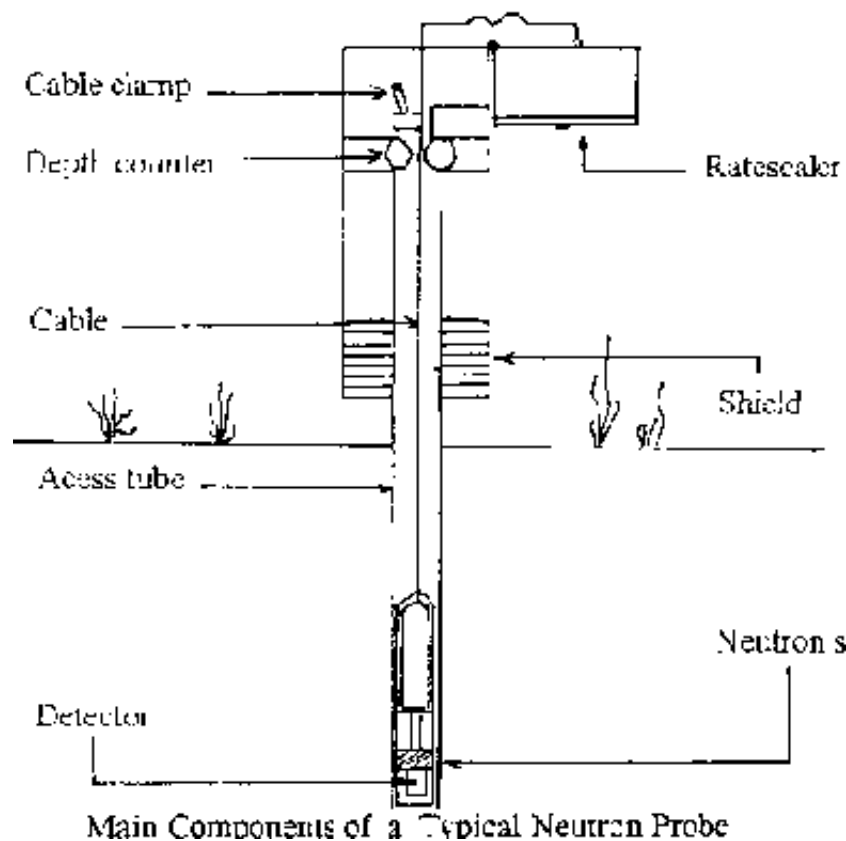


Figure 5.5: Main Components of a Typical Neutron Probe

Installation

The installation of access tubes can be one of the most difficult and time-

consuming aspects of neutron probe field work. Hard, dry soils, gravel, concreted layers and high bulk densities make observation hole excavation problematic, despite the development of mechanical installation systems. In fortunate circumstances, hand excavation by auger is possible. The main criterion of effectiveness of installation is the snugness of fit of the access tube within the hole. This depends not only on the method of installation, but also on the type and wetness of the soil. All sites around the access tube should be protected from trampling and soil compaction by the use of wooden frames or palettes, upon which the operator can stand. Vegetation around the tube should not be disturbed if it constitutes part of the actual environment.

Installation by hand:

Continually hand-auger from within the tube in depth stages of 20 cm, remove the material, then push the tube down. This should ensure a good fit. The tube will shear off a little soil material as it is placed, but the undersized hole gives snugness of fit. Some workers excavate an oversized hole and after the tube is installed, back-fill with the soil material. Whichever method is used, tropical and sub-tropical soils are often too hard for hand boring. Ingenuity of the individual worker is often required to solve the problems of installation, which may be quite different from site to site.

Mechanical installation:

One example of a field-tested mechanical installation device is given below, it was developed from a geological sampler, capable of excavating to 25 m. It is illustrated in Figure 5.6.

An initial hole is made to about 0.5 m using a guide tube and sledge hammer. Then solid steel string sections, joined by threaded joints, push down a 0.6 m cutting tube head under the force of a hydraulic jack-hammer. Depending on the soil, the excavation may proceed in stages of 20 -50 cm. It may be necessary for the cutting tube to be locally manufactured, to the same diameter as the access tubes. The cutting tube is retrieved by the use of a ball clamp and hydraulic ram which operate from the hammer compressor, powered by an 11 hp petrol motor. The soil is removed from the cutting head by screw augers, though this can be a difficult task as the soil is highly compacted. Breakages tend to occur in the string joints and the cutting tubes need to be sharpened frequently. They often how after prolonged use.

Location

The precise site location of neutron probe access tubes should be considered carefully. The radius of detection is small and combined with soil spatial variability, can lead to unrepresentative sampling. Moreover, the variability of surface water infiltration, sometimes due to soil type, but more usually due to local runoff distribution, can be very great. Soil moisture in low areas generally penetrates further and a water table may be present in such locations. It is advisable not to plumb the water table, to avoid submersion of the probe. If field crops are under investigation, access tubes to 2 m, possibly 3 m, will be adequate. Much deeper access tubes will be necessary to study deep drainage, but there is no value in placing access tubes to great depth unless this is warranted; it merely increases the cost and time involved in installation, monitoring and analysis, to little purpose. Where the interest is to monitor the variability of soil moisture due to pronounced topographic features, access tubes should be located on or within

the features. Where a field-wide representation is required, access tubes should be located on a predetermined grid, so that random sampling operates. The depth of access tubes should also be considered carefully where there is topographic variability.

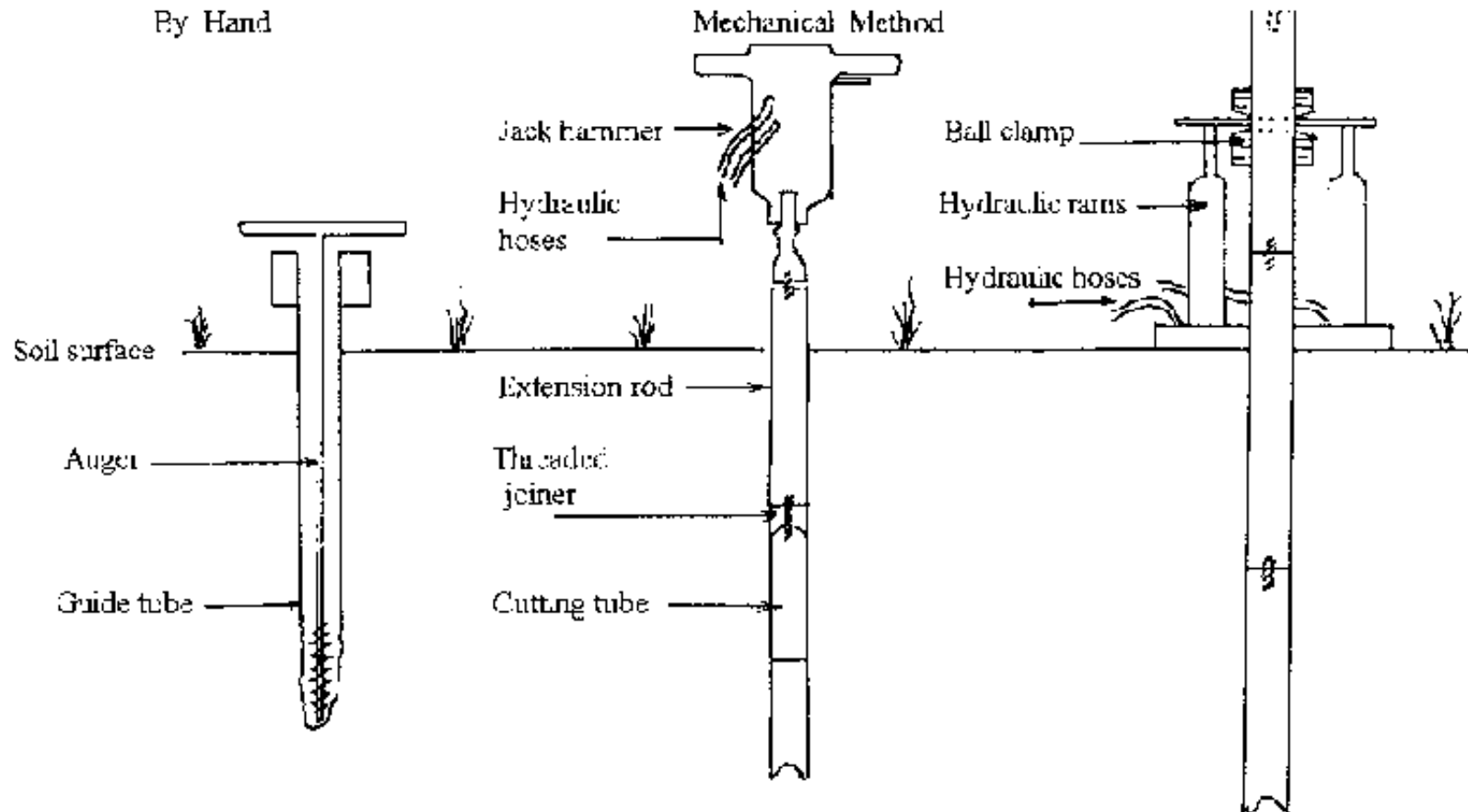


Figure 5.6: Acces Tube Instalation Equipment

Neutron probe access tubes are relatively cheap to install and the temptation to over-monitor must be resisted; large quantities of raw data will be produced which must all be quality checked and analysed. Moreover, sites require constant

monitoring on a routine basis and additional site visits after important rainfall events will be necessary. It is important not to over-stretch resources of time, transport and manpower. Access tubes that have been installed but which cannot be monitored, or backlogs of data that cannot be processed not only create a sense of frustration, but are a waste of time, money and effort that could be used elsewhere.

Operation

Standard counts are used as a routine method of checking the performance of neutron probe equipment. A variety of faults that cause a drift in the count-rate under identical conditions may be found, and standard counts should be performed before each time the probe is used and, preferably, after. An important use of standard counts is calibration between instruments, thus allowing their interchangeable use, a great advantage when planning any monitoring schedule. There are two types of standard count:

Water barrel counts are taken with the probe seated on an access tube section which can be conveniently mounted in a 200 litre oil drum. The drum should be thoroughly cleaned, water-proofed and filled with clean water. A three-barred support welded to the top edge of the barrel and holding the tube section in a vertical position is suitable. A length of tube, perhaps 25 cm, should extend out of the water. Ten counts of approximately one minute each are adequate.

Shield counts are taken with the probe retracted into the shield of the unit and with the unit standing on its carrying case. This method of checking the instrument is especially useful at field sites where water barrel facilities cannot be

maintained, though during this calibration the same prevailing conditions are not so easy to replicate. Ten shorter duration counts, perhaps 15 seconds each, are sufficient.

The equipment will vary according to manufacturer, but certain aspects of operation are common. Care should be taken in packing and in transit. The battery supplies will need constant recharging at the design rates and cannot be expected to last more than a year or so; a good supply of spare batteries is essential. The probe cable is susceptible to damage with use as it carries the weight of the probe and detector unit and is continually being reeled in and out. Spares should always be available. Jamming of the source and detector in the access tube may occur and exacerbate the problem of cable damage.

Modern probes usually have depth indicators integrated with the rate-scalers. On the whole, rate-scalers that require a manual record of count-rates are best to purchase, although some types with integral memories are available as alternatives. The latter are perhaps are less robust and more susceptible to damage. If they malfunction data may be lost and the equipment is rendered unusable for some time. Neutron probes, though generally very reliable, are complex instruments and local repairs are often impossible. It is best to have one complete unit available as a spare.

Protection to the operator must be given by emphasising that while the radioactive source used in the instrument is very small, any exposure to radiation constitutes a needless risk. Radiation badges that are pinned to the clothing of the operator should be purchased, to monitor any exposure to radiation. In developing countries, it is likely that these will have to be sent overseas to be evaluated. It is

also well worth remembering that the transport of radioactive material, in many countries, is restricted by certain laws. Although these may not pose problems for routine movement from site to site, they may require special customs clearance procedures to be effected. Before purchase is complete, the appropriate local authority should be consulted so that paths may be cleared and frustrating delays in obtaining the equipment can be avoided.

Calibration

Neutron probe count-rates need to be calibrated, as both individual instruments and soils behave differently. For example, high bulk densities give higher count-rates for the same soil moisture content. Neutron probes are supplied with calibration curves as a guide, but these are constructed in the laboratory and are seldom appropriate for the soils under investigation. Calibration curves developed by other researchers may be of interest, but generally the same problems apply. Calibration must always be undertaken if soils are suspected as being sufficiently individual to necessitate this. Different manufactures of instrument will greatly affect count-rate/soil moisture relations.

Field Calibration

In essence, calibration consists of taking count-rates at specified depths in the soil profile; for example at 10, 40, 70, etc. cm depths. Soil samples are then taken at these depths for soil moisture and bulk density analysis. It is important to note here, that compared to deep readings shallow readings may not be accurate nor consistent, because of the loss of fast neutrons through the soil surface. As a consequence count-rates tend to be lower and separate calibration curves should

be constructed for the top 30 cm or so of the soil profile. Calibration is a relatively time-consuming business and due the nature of the method, will destroy the further usefulness of the access tube location. Proceed as follows:

- An access tube is installed, or an existing tube is used.**
- Two wooden pegs are sunk 1 m either side of the tube and made level using a board and level. The soil surface is scraped level and the height above the board noted.**
- The soil moisture profile is monitored at 10, 40, 70 cm below the soil surface, first with the probe at the height that readings are usually taken above the soil surface with the instrument sitting on the access tube (where applicable), then repeated after the tube has been tapped level with the soil surface. Several readings at each depth should be averaged, to minimise random errors.**
- Duplicate cores taken close to, but on opposite sides of, the tube are removed with their mid point at 10 cm depth. At the same time a duplicate pair of cores are taken in the same way, at right angles to the first pair.**
- Excavate a 1 m diameter soil pit accurately to 20 cm below the original surface and using the level board make it so for 35 cm radius around the tube.**
- The tube is then sawn off, tapped down or replaced with a shorter one (the former has the advantage of less soil disturbance), then the 40 cm depth is re-monitored at +5 and 0 cm above the soil surface.**

- **Ten cm of soil is removed and the original 40 cm depth re-monitored.**
- **Core samples are taken once more.**
- **A new soil surface is located at 50 cm below the original and the last three steps repeated for the original 70 cm depth.**
- **The soil samples should be analysed for volumetric water content and bulk density. The analysis of particle size distribution links calibration to soil textural type.**
- **Deeper horizons can be calibrated if it is felt that a need exists.**

Each calibration, carried out as above, yields 3 points for the 10 cm calibration and 2 points for the 20 and 30+ cm calibrations. It is probably best to limit the number of such calibrations to one per day, to ensure good working practice, though two are possible and should enable soil moisture relations at all research sites to be established.

Calibration is necessary at the wet, intermediate and dry parts of the calibration curve, where seasonal variations in rainfall lead to extreme differences in soil moisture content. This can be undertaken after continual rain (or artificial wetting), after a period of drying and at the end of the dry season. Figure 5.7 shows an example of the field calibration of neutron probe data.

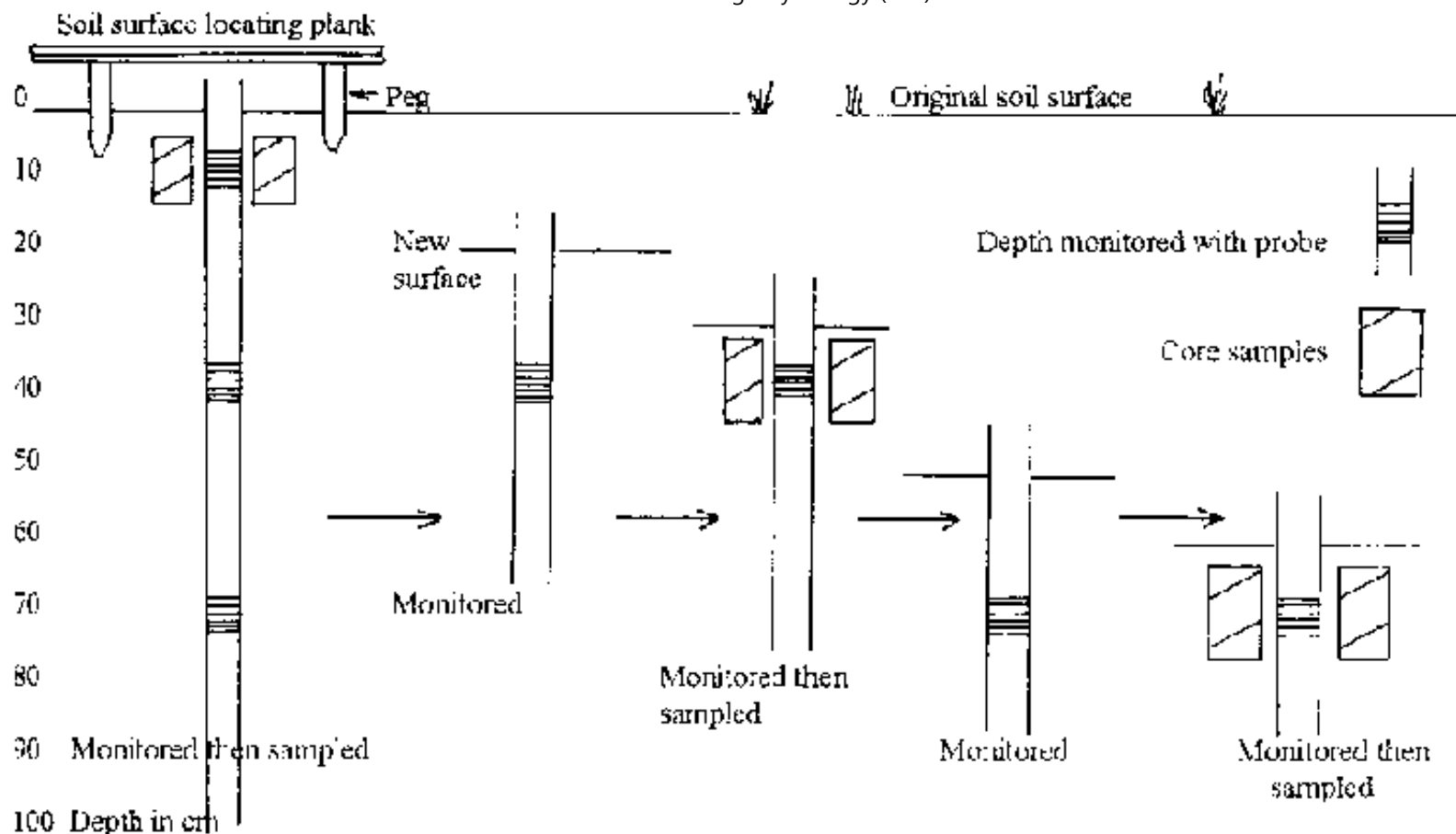


Figure 5.7: Field Calibration of Neutron Probe Data

Cross Calibration of Different Manufactures of Neutron Probe

It is quite possible that different makes of neutron probe may be used by the same project or associated projects. In this case it is preferable to compare water barrel and access tube counts rather than to repeat the procedures of field calibration for both types of instrument. Cross calibration may be regarded as essential and is undertaken as follows:

Count rates are taken in the water barrel access tube at 2 cm intervals from above the water level to 25 cm below its surface, then at 5 cm intervals to 35 cm. Averages of duplicate counts are taken for each instrument. In field access tubes, first one probe is used then the other, and this is repeated for average readings. Usual probing depths, for example every 10 cm, are used. Where instruments show accumulated count-rate totals, these must be converted to count-rate. Figure 5.8 show the 30+ cm calibration curve for some semi-arid soils, with Count -Rate/ Water Barrel Rate (the Count-Rate Ratio, R/RW) versus Moisture Volume Fraction. Figure 5.9 shows a graph comparing the countrates of one make of neutron probe with another.

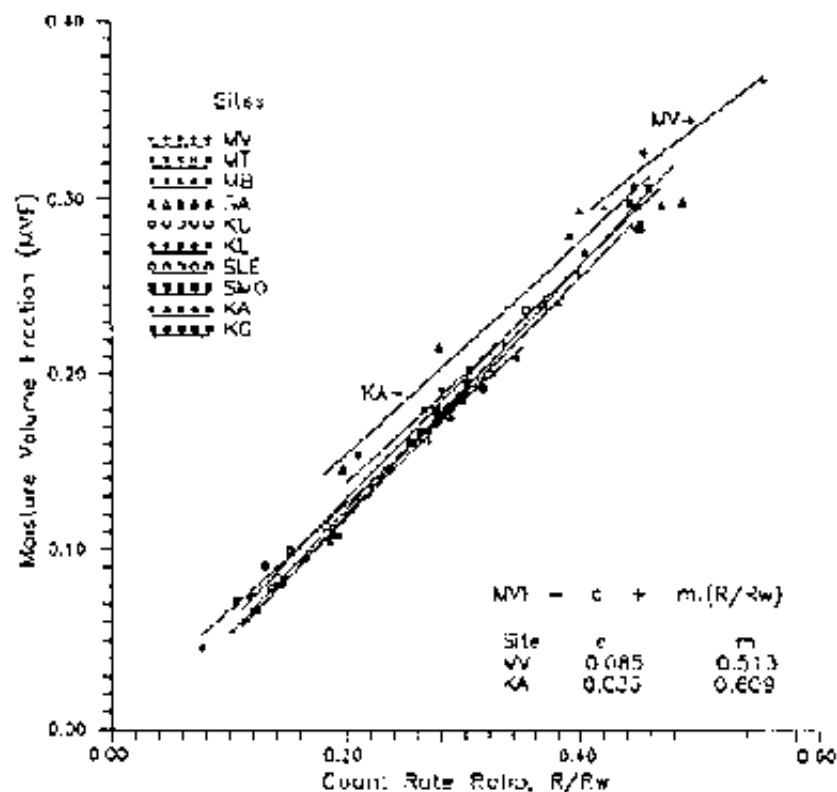


Figure 5.8: 30+ cm Calibration Curve for 10 Semi-arid Sandy Loam Soils

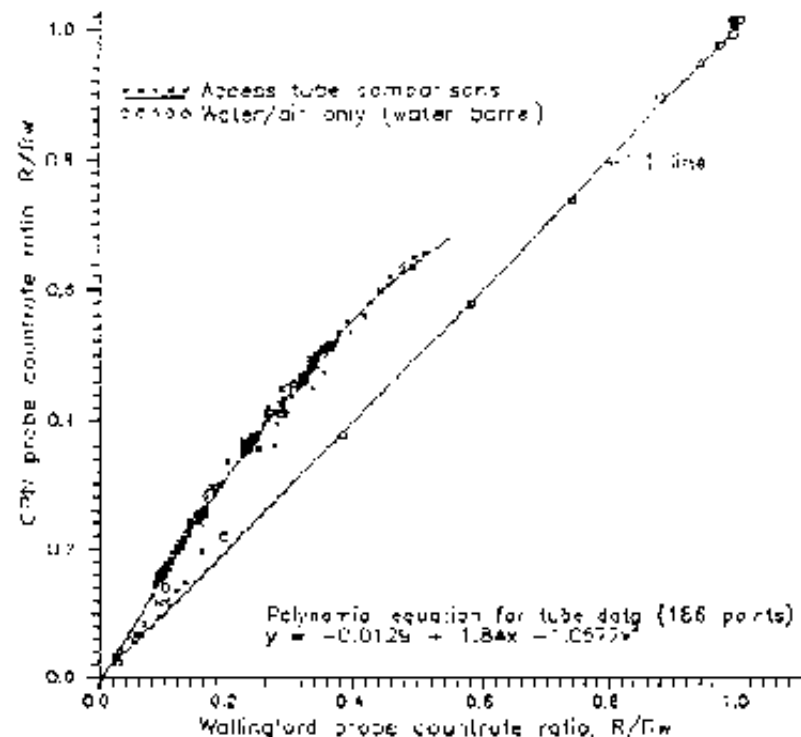


Figure 5.9: Comparison of Count-Rate Ratios for Two Manufactures of Neutron Probe

d. Time Domain Reflectometry (TDR)

TDR measures soil moisture content by utilising large differences in the dielectric properties of soils, water and air. Methods exploiting these soil/water properties have been in development for several decades, but it is only very recently that TDR has become a convenient, practical tool for soil moisture investigation.

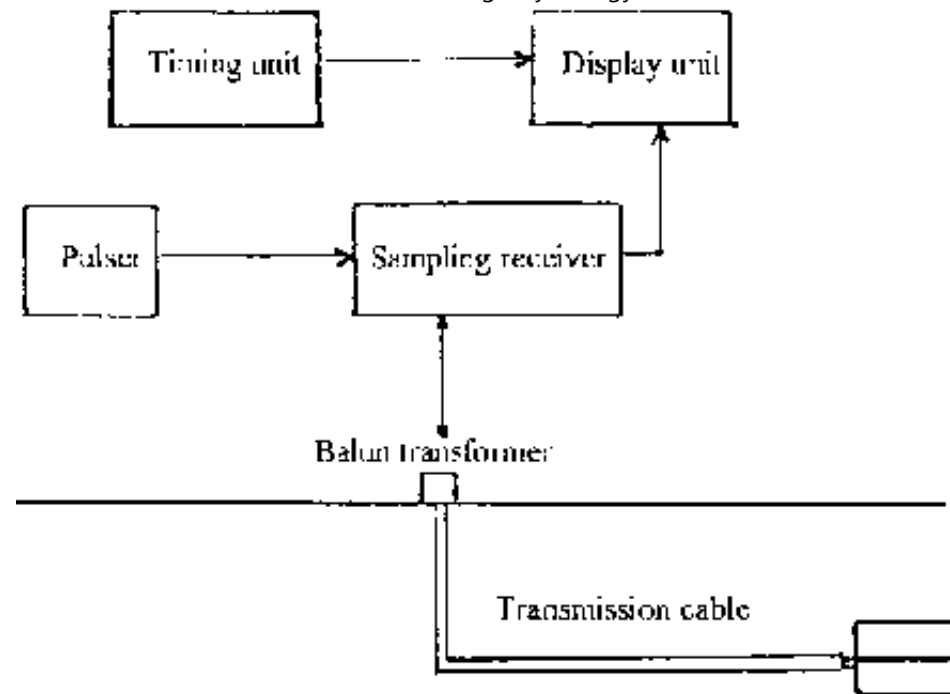


Figure 5.10: Diagrammatic Layout of TDR equipment

The advantages of TDR equipment over the neutron probe are that it can be logged continually, it is relatively non-destructive and does not utilise radioactive materials. Figure 5.10 shows a diagrammatic layout of the equipment and Figure 5.11 the idealised waveform of a wet soil.

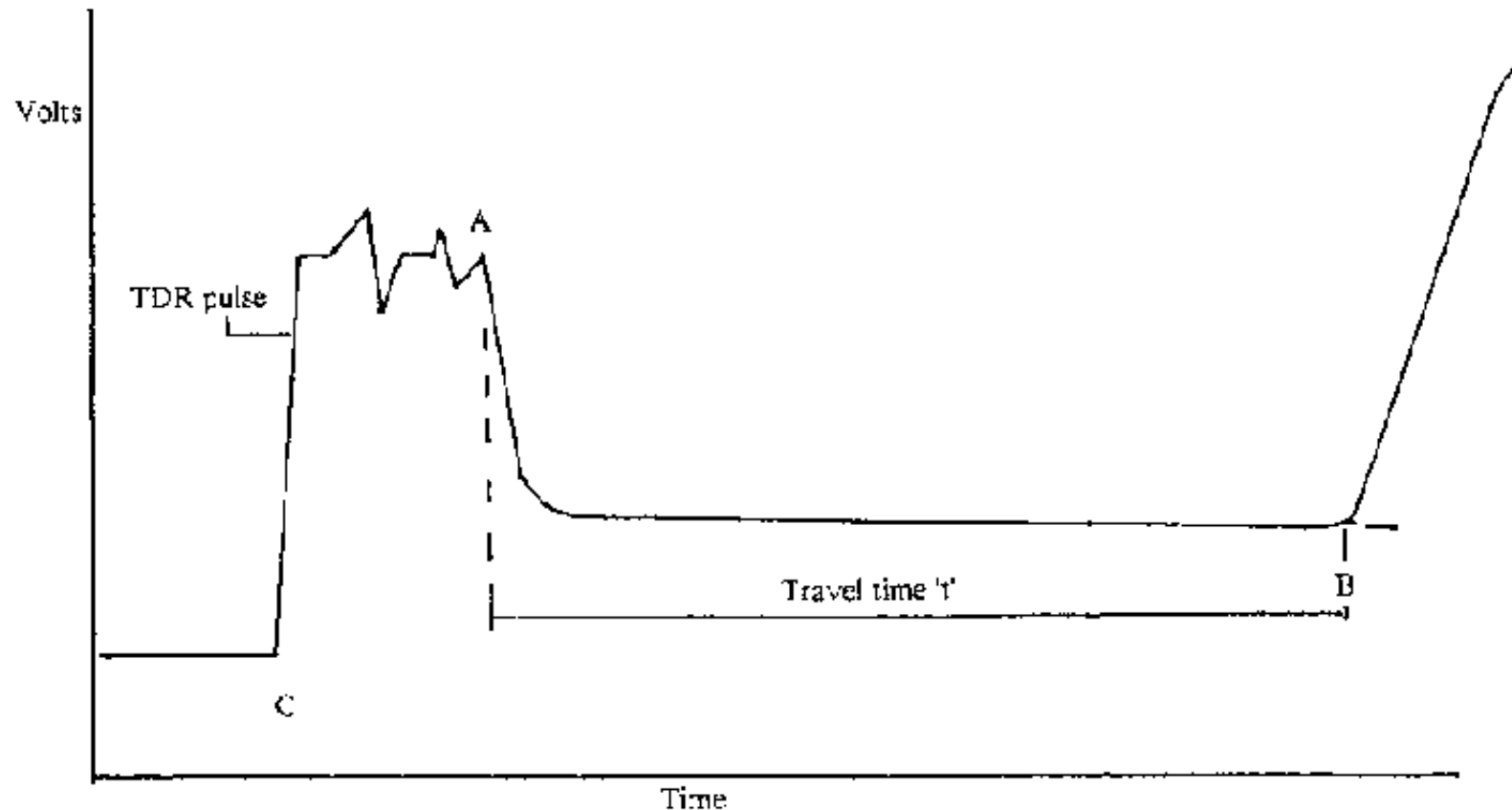


Figure 5.11: Idealised TDR Waveform of a Wet Soil

Principles of Operation

A pulse generator creates a fast rise-time voltage pulse (about 10-10 seconds) which passes through a transmission line to a balun transformer designed to achieve maximum transmission into the soil. The pulse then passes to the waveguide (a pronged "fork" or "probe") in the soil. An impedance mix-match causes part of the pulse to be reflected back to the instrument, while part is propagated to the end of the waveguide, from where it is then reflected back to instrument. The analysis of the waveform (change in voltage) of the pulse that is

reflected from the ends of waveguide is the key to measurement. The interpretation of the waveform is somewhat subjective.

Point C shows the rise-time of the pulse (usually 10-10 to 30-10 seconds), the point A represents the location of the balun transformer, the distance between C and A depends upon the length of the transmission cable. Point B is the reflection from the end of the waveguide. The travel time 't' is the time in nanoseconds for the pulse to pass from A to B and back. Pulse attenuation, the loss of magnitude of the signal, increases with moisture content. The calculation of the apparent dielectric constant, K_a , (approximately 80 for water, 1 for air and 2-4 for soil minerals) is made by:

$$K_a = (t c/L)^2 \text{ where (5.3)}$$

t = travel time

c = speed of light ($29.979 \text{ cm ns}^{-1}$) and

L = length of the waveguides in cm.

TDR instruments can be "multiplexed", utilising a variety of waveguide lengths set to log continuously on a series of channels at a given time interval and controlled by a small computer. However, it may take half an hour to obtain a set of measurements and any rainfall during the logging cycle could give confusing results if the logging cycle were short. Continuous monitoring opens many opportunities for the detailed examination of moisture changes, especially at shallow depths, but generates a large amount of data.

Much development has concentrated on waveguide ("probe") form. There are two

main types:

Unbalanced waveguides have three prongs, the inner prong, for example, carrying a positive charge and acting as the centre of a coaxial cable, the two outer prongs carrying half a negative charge and acting as the shield.

Balanced waveguides have only one outer prong and the lack of shielding is compensated for by a balun transformer acting as a shield. They are regarded as more sensitive and less destructive, but are more expensive and can hinder interpretation by inducing interference on the waveform.

Practical considerations

The sampling volume of the instrument is very small (radius of sampling within about 1 mm of the waveguide surface) and may be greatly effected by the near-surface environment, though research into these effects is as yet inconclusive. Waveguides are (usually) placed horizontally at various depths, avoiding the penetration of soil layers with different characteristics. This often necessitates the excavation of a trench behind the locations of the waveguides.

The length of the waveguide prongs is an important consideration. Usually they are between 0.05m and 1.0m long. The suitability of length is determined by the attenuation characteristics of the soils under examination. Long prongs offer a greater volume of sampling, but are more difficult to install and are not suitable for highly conductive soils. Spacing between prongs of the waveguide is usually no more than ten times their diameter, with an upper limit of 5 cm. Wider spacing leads to difficulties in the interpretation of the (greatly attenuated) waveform.

Increasing the transmission cable length decreases resolution and a maximum of 30m is recommended.

The effects of soil mineralogical composition on TDR is not fully understood. Research has indicated that heavy clay soils may give anomalous results and clay content; bound water and bulk density have been shown to influence calibration. The dielectric constant of a soil is temperature dependent and therefore diurnal variations may be significant, especially near the soil surface. The influence of organic material is not yet fully understood, though increasing organic content tends to reduce the calibration slope of K_a versus water content. Increasing the bulk density of soils of a given water content decreases the dielectric constant of such soils, and the installation of the waveguides should disturb the soil as little as possible. The reduction of air gaps due to installation may be achieved by wetting the soil before insertion, though the characteristics of soils will determine the effectiveness of the installation. TDR is unsuitable for use in stony soils, and in soils that swell and shrink. Waveguides should not become the focus for cracking and enhanced percolation, nor be sited cross distinct soil boundaries. Soils high in iron and titanium (and possibly aluminium) minerals have enhanced dielectric constants. A guide to operational difficulties in various soil types is given below.

Vertisols: Vertical installation should not be undertaken. They may have a low dry bulk density due to cracking. Organic content may be high; electrical conductivity may effect attenuation.

Entisols: Relatively unstructured soils with little organic matter.

Aridisols: Conductivity may be affected by salts and high temperatures close to the

soil surface.

Mollisols: Grassland soils, often with high organic contents in the upper horizons.

Alfisols: No obvious problems.

Ultisols: High content of iron and/or aluminium may exaggerate moisture readings.

Oxisols: Low bulk densities may reduce the slopes of calibration. Temperature effects may be seen and possible influence of oxides which increase conductivity.

Spodosols: Sharp density changes down the horizon. A high organic matter content in the A horizon may lead to the underestimation of soil moisture content.

Histisols: High in organic matter and will need separate calibration. Bulk densities tend to be low.

Calibration

Calibration is undertaken for values of K_a versus volumetric water content and may be done in the laboratory or field.

Laboratory calibration:

Research in the past has concentrated on the water content of substitute materials rather than soil, for example vermiculite, glass beads and washed sand. More recently soils have been used and indicate separate calibration curves for organic

and mineral soils. However, as for the neutron probe, laboratory calibrations do not accurately represent the spatial variability of such factors as texture, bulk density and organic content.

Field Calibration:

Field calibration may involve the measurement of soil moisture by lysimetry, neutron probes and gravimetric methods, though sampling and oven drying gravimetric methods are standard. Sampling may be undertaken in a manner similar to that described above, for neutron probes. At water contents of greater than 20%, laboratory calibrations were seen to overestimate moisture content. Differences between neutron probe and TDR measurements have been found to be large (10 - 20%) and may reflect the different sampling volumes of the two instruments. These results indicate that the use of TDR should be carefully examined and TDR may not be suitable for water balance studies, though its more precise and accurate measurements can be effectively used in rooting zones and areas of marked changes in water content over small distances.

Waveform Interpretation

Interpretation determines the value of K_a that is derived from the waveform. An example of a K_a calculation is given below, with the use of Figure 5.12.

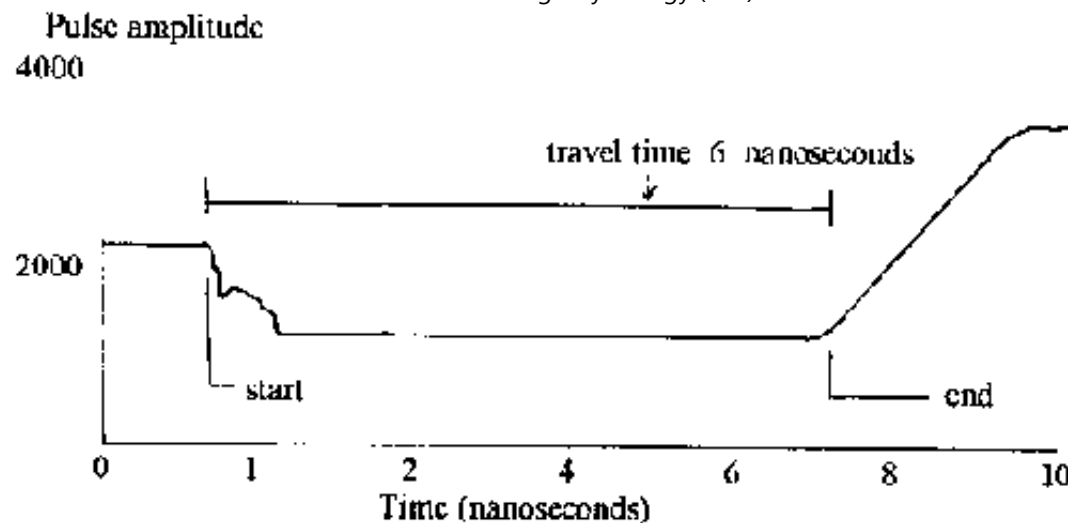


Figure 5.12: Hypothetical Waveform

From equation 5.3, $K_a = (6 \times 29.979/20)^2 = 80.89$ (the value of K_a is dimensionless)

Figures 5.13 (a), and 5.13 (b) show the idealised waveforms of air, water.

Figure 5.14 shows how the waveform changes with increasing and soil moisture content.

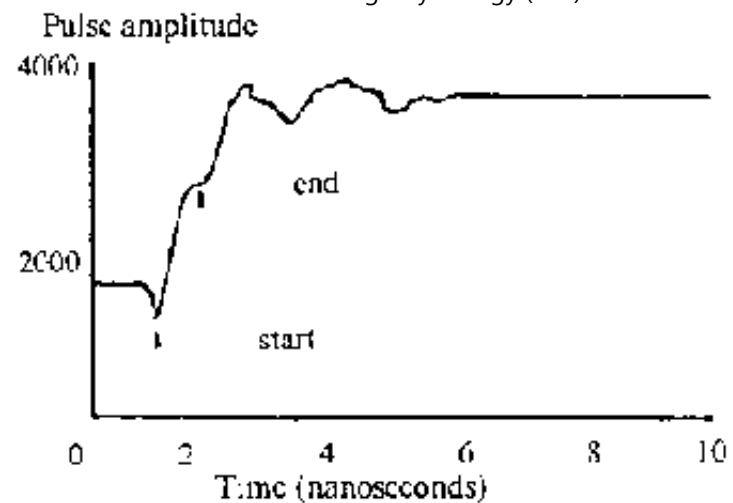


Figure 5.13 (a): Waveform, Air

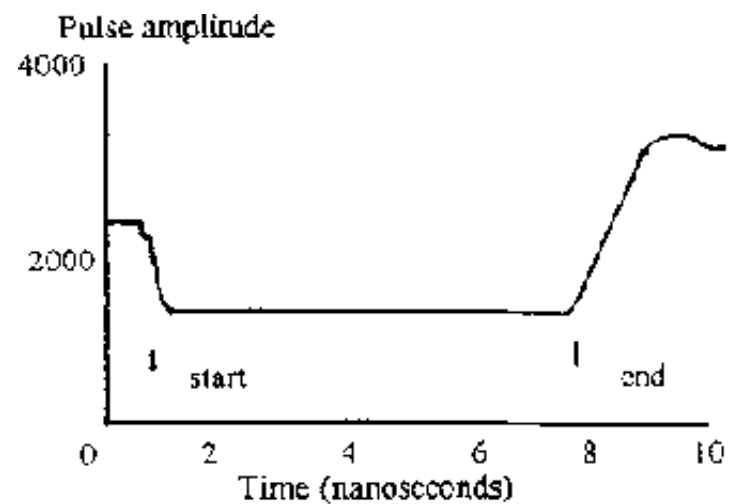


Figure 5.13 (b): Waveform, Water

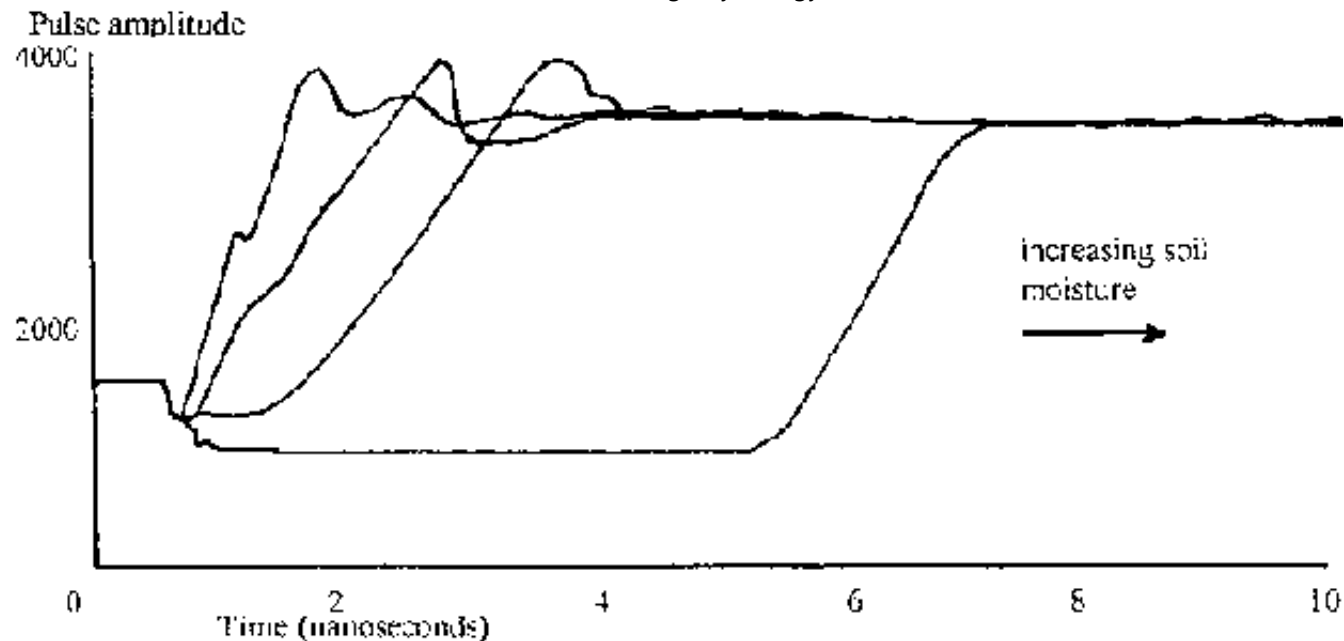


Figure 5.14: Waveform, Different Soil Moisture Contents

Below are example waveforms for different soil conditions that may be encountered. Figures 5.15 (a) shows the waveforms of a waveguide installed vertically through a wetting front and (b), a dry zone over a wet zone.

Soils with increasing percentages of iron oxides display a waveform of decreasing amplitude and increasing travel time. Relatively dry, clay mineral soils frequently exhibit a large amount of background noise which makes interpretation difficult, while wet clays often exhibit large pulse attenuations, probably due to high pore water electrolytic concentrations.

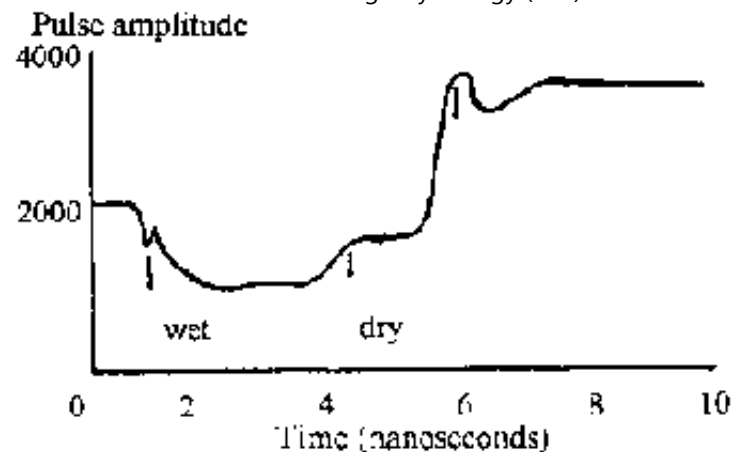


Figure 5.15 (a)

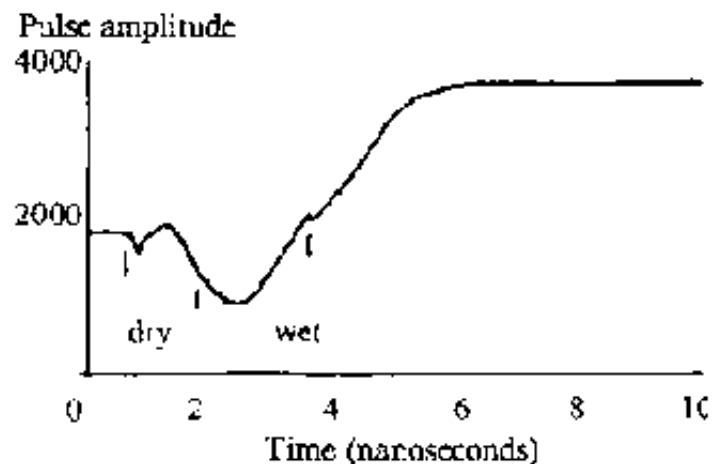


Figure 5.15 (b)

Figure 16 shows the effect of increasing salinity. The travel time is not changed, but the magnitude of the pulse decreases as the salinity, and conductivity, of the soil increase.

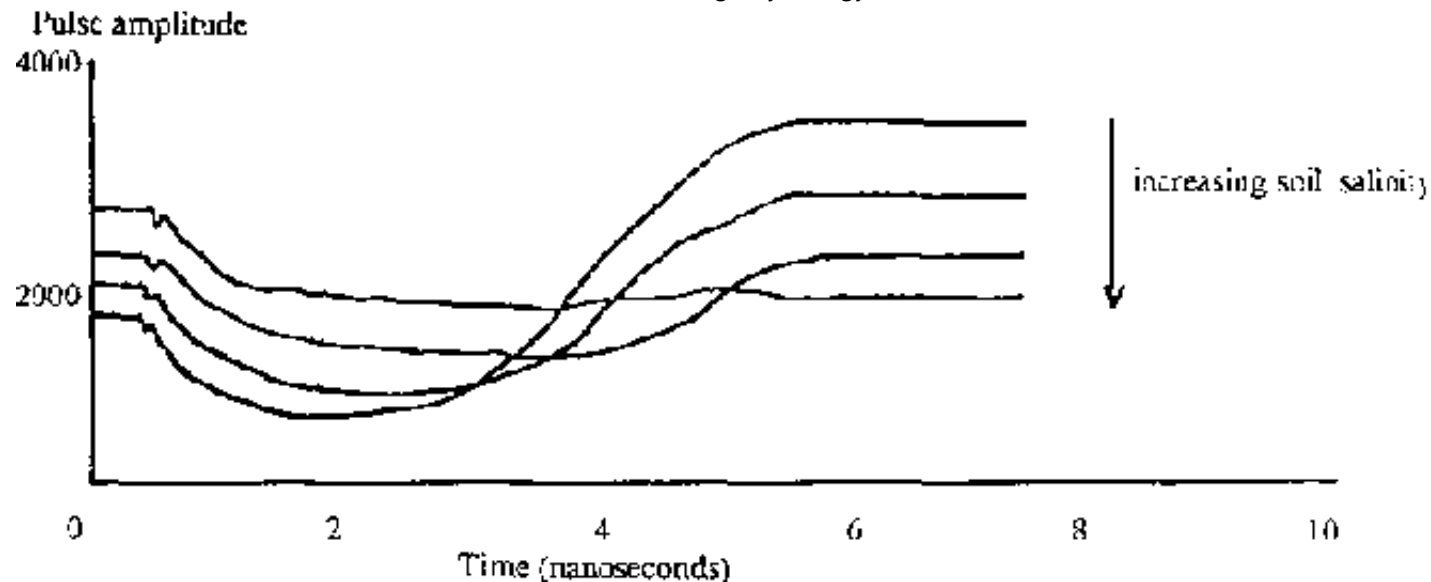


Figure 5.16: Waveform, Increasing Salinity

e. Capacitance Probe

Capacitance probes, like TDR, have only recently become practical field instruments for measuring soil moisture. They also measure the dielectric constants of soils, like TDR, but are relatively low cost. They usually utilise an access tube, but recent designs will allow direct insertion into the soil and readings are usually taken from a hand-held meter. It is necessary to calibrate these probes for individual soils.

5.3 Infiltration

Infiltration is the process whereby water on the soil surface percolates downwards. Infiltration rates represent the speed at which this percolation occurs and are expressed in mm in-1. The maximum rate for any given soil condition is

termed the "infiltration capacity". The controls on infiltration rates are many: soil texture, cavities and impermeable layers, vegetation cover, air spaces, soil wetness and topography all may be influential. Land use and cultivation can also be extremely important as they affect the quantity of suspended material in surface water which in turn influences rates of infiltration, because the suspended material blocks pore spaces and increases runoff. Measured rates of infiltration lump the effects of all these influences together.

Runoff is the proportion of rainfall that does not infiltrate, at least immediately, but infiltration rates vary greatly with time, especially during rainstorms when the soil becomes progressively wetter and as rainfall intensities vary. Thus the supply rate of water to the soil surface and the rate at which it infiltrates are never constant. Despite the fact that infiltration rates represent a gross generalisation of soil/water behaviour, they are often important components of hydrological and runoff models.

Extensive field trials have shown that infiltration rates decrease with time, in the general form:

$$I = (aT^n + b) \text{ where (5.4)}$$

I = infiltration rate

a, b and n are constants and

T= Time elapsed

Thus infiltration rates are exponential. As the rate of infiltration decreases it approaches, and sometimes achieves, a terminal value. For clay soils, the value of

'b' in equation 5.4 may be almost zero, while for sandy soils it will be much greater; soil texture plays an important part in the determination of infiltration rates. Infiltration rates are used not only to estimate the likelihood of runoff, but are also quoted as a general soil characteristic, but it should be noted that the infiltration rates of soils are notoriously spatially variable, even over distances of a few metres.

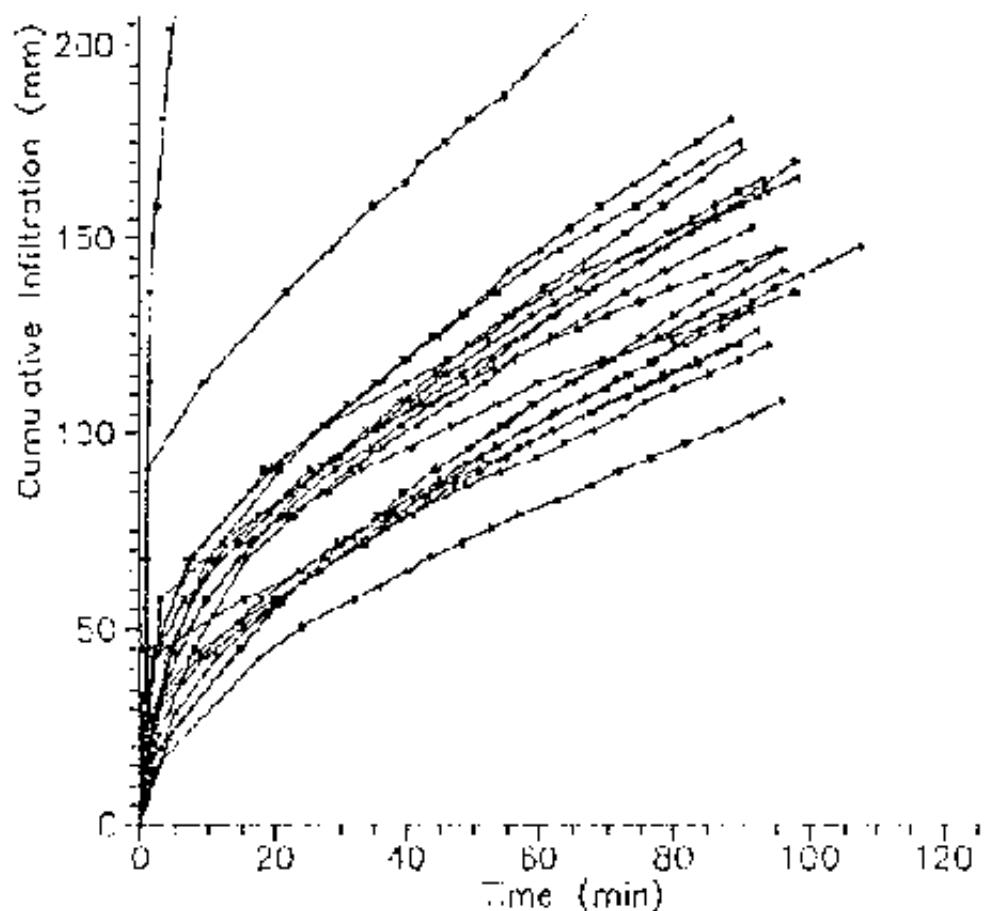


Figure 5.17: Variation of Infiltration Rates Over Small Areas

Variability is important because it can be associated with microtopography in

fields and cultivation practices which alter the location of topsoils. For example Figure 5.17 shows the variation of infiltration rates of 20 tests undertaken over an area only 15 m square, on an apparently uniform soil. Variation will affect the redistribution of runoff at the local scale and will be influential in determining the soil moisture that is available to plants. Important differences between rates on cultivated and uncultivated land are likely to be seen.

Figure 5.18 shows example infiltration curves for different soil textwal types: sandy, loamy and clay soils.

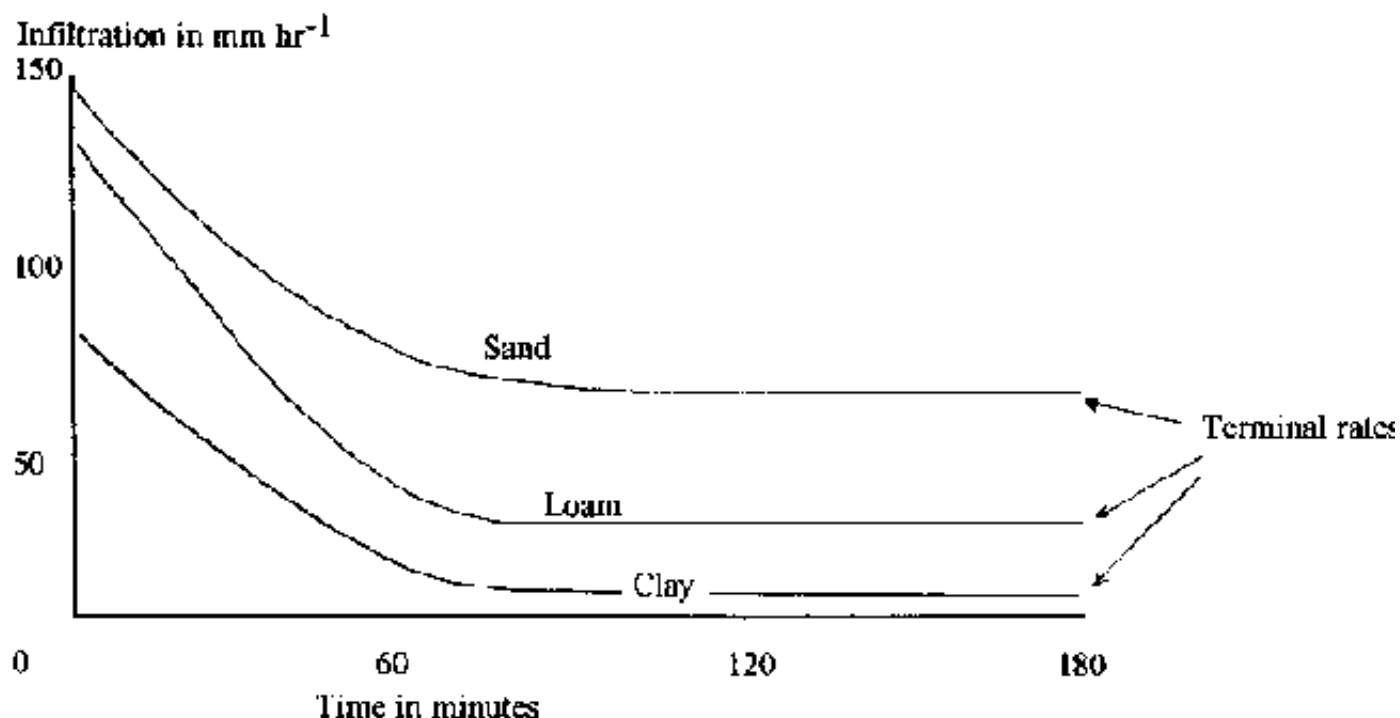


Figure 5.18: Example Infiltration Curves for Sandy, Loamy and Clay Soils

Three methods of measuring infiltration are discussed below.

5.3.1. Equipment and Methods of Measurement

a. Double Ring Infiltrometers

Double ring infiltrometers consist of two concentrically placed rings, both filled with water. The rate of infiltration into the ground within the inner ring is measured, while the water in the outer ring provides a buffer to ensure the direct downward movement of water below the inner ring.

Infiltrometers can be purchased, but as they are simple to manufacture to specific requirements, this may be preferred. They can be made by using metal water pipe cut to a suitable length and given a bevelled, sharpened edge at one end. Example dimensions are: length 30 cm, diameter 60 cm, the smaller ring should be of the same length, but half the diameter, also sharpened. The two different sized rings are inserted into the ground, the depth of insertion will depend on the hardness of the soil. A more or less constant head of water is maintained at a measured and marked level (10 cm is suitable) above the ground surface. Water is poured into both rings until it reaches this level and then another 1 litre of water is added. When the water in the inner ring falls to the marked level another litre is poured in, to compensate for infiltration. If infiltration is slow, only 0.5 litre need be added; the water in the outer ring is also kept to the level and generally it will take one or two hours to reach a constant (terminal) rate of infiltration. Figure 5.19 shows the installation of the double ring infiltrometer.

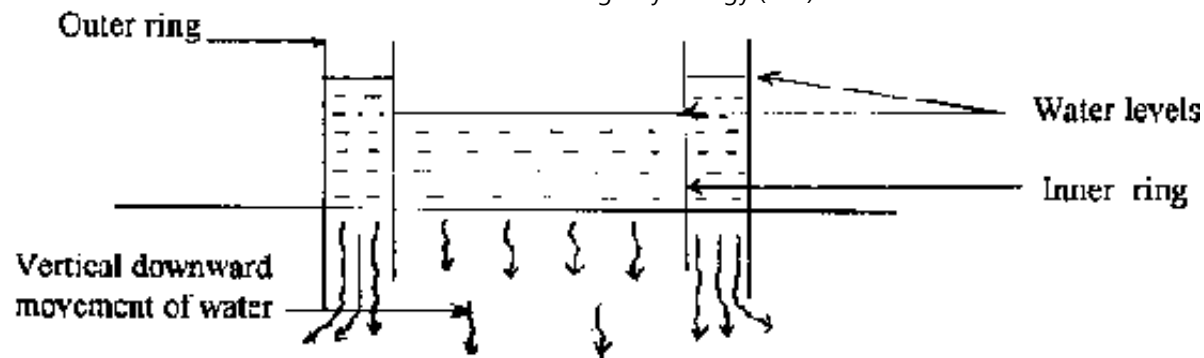


Figure 5.19: Double ring Infiltrometer.

The accuracy of the data collected from infiltrimeters may be affected by the insertion of the rings, which can alter the physical characteristics of the soil. Soils with macropores, burrows etc. may show extreme rates of infiltration and prove to be unsuitable for study by this method. Crusted soils can be prepared by cutting the crust with a razor blade and inserting the rings through the cuts. The gap between the ring and soil may be sealed with gypsum paste or hydraulic cement.

The major difficulty with data that are obtained by the double ring method however, is that they do not represent infiltration under rainfall/runoff conditions: rainfall impact may increase or reduce infiltration at the soil surface; rain storm intensities vary greatly and the standing heads of water used by these infiltrimeters are not usually representative of real conditions. Runoff flows away and is not impounded. The spatial variability of infiltration capacities is common and results may be relevant only to very small areas. It is important to note, however, that data from double-ring infiltrimeters are widely used and it may be essential to collect this information if comparisons between a range of sites and soils are to be made. The data's main value is as a reference for comparison rather than the provision of absolute, true infiltration rates, though these may be of

direct application to irrigation practice. To overcome the unrealistic results that double ring infiltrometers often provide, sprinkle infiltrometers have been developed. An approximate replication of natural rainfall can be obtained by the use of sprayers and sprinklers. Sprinklers can be complex systems that simulate duration, rate, drop size etc., but even the smallest versions of these instruments are not really portable and often require large volumes of water to operate. Knapsack sprayers are low cost alternatives that can be easily transported and when used carefully, provide relatively good data.

b. Knapsack Spray Infiltrometer

Spray infiltration tests using knapsack sprayers improve upon the results from ring infiltrometers by minimising the effect of a standing head of water and by applying the water as a spray. Further emulation of the rainfall/ infiltration process is not attempted. Although this method is only a rudimentary attempt to simulate rainfall it does provide an improved technique which is portable, easily replicated and inexpensive. Typically, a quadrat is sprayed at a designated rate which is reduced upon the evidence of standing surface water. Details of the method are as follows:

An area 1 m square is marked and within it a 50 cm × 50 cm × 5 cm deep wooden quadrat is placed centrally, pushed 2 -3 cm into the ground. The central quadrat is divided into quarters. Wind shields should be provided around the site if necessary. The sprayer is best fitted with a fan nozzle to provide a wide, even spray. The 1 m² quadrat is sprayed with an even application every 30 seconds. The period of pumping to prime the sprayer and the duration of application are regulated (for example pumping and application for 5 and 10 seconds

respectively) to give a similar intensity spray each time. When water is seen standing on two or more of the central quarters, the next spray is omitted. This may be continued for an hour or more, or until a recognisable uniformity of application indicates stability of the infiltration rate. The amount of water delivered is quantified by repeating the spray test procedure into a measuring vessel before and after the test. The total volume of water that can be applied during the test will depend upon the nozzle aperture, but depths of 40 mm can easily be achieved. Variations on this method may be designed to account for local conditions.

This method, though not representing true rainfall conditions, is relatively simple and easy to replicate and the data obtained discriminate clearly between soils of different texture. The rates also resemble those that might be expected under rainfall conditions. Figure 5.20 shows a vertical view of equipment layout used in running tests with a knapsack sprayer.

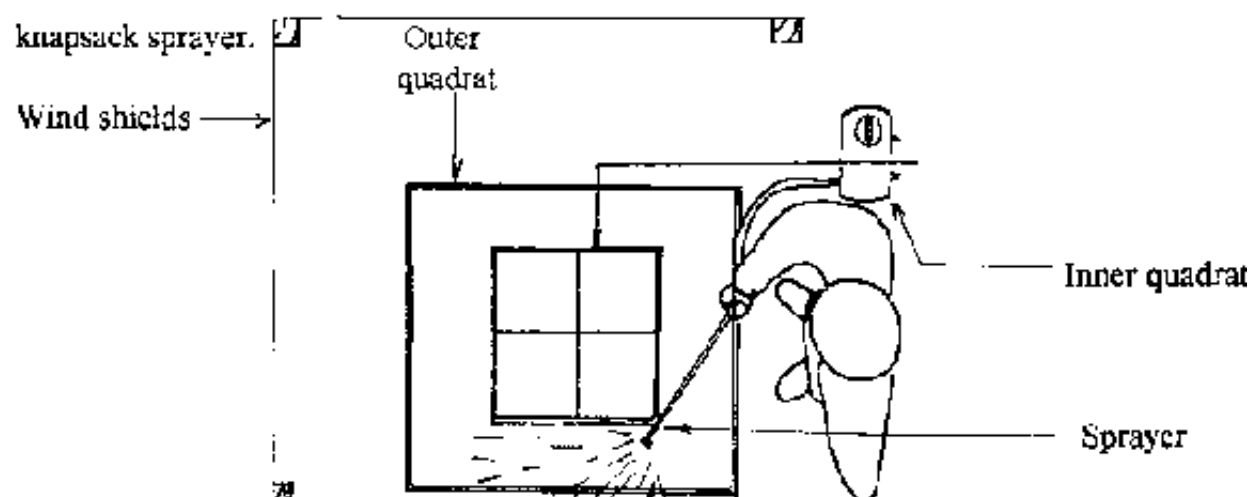


Figure 5.20: Knapsack Infiltration Test Equipment (Vertical View)

Testing may be undertaken on cropped, range or fallow land. The simulation of ploughing effects can be achieved by digging the soil over; this may be necessary in regions that have distinct dry and wet seasons. In these areas testing is most suitably undertaken during the dry season, when the influence of rainfall and high levels of antecedent soil moisture are not evident. It is likely that fields will not be cultivated at this time.

c. Sprinkler infiltrometers (Rainfall Simulators)

Unlike double-ring infiltrometers, sprinklers are not limited to the study of infiltration rates. They are often used to investigate such influences on the rainfall/runoff process as soils, slopes and tillage practices and may be used to measure rates of soil erosion. It is convenient however, to discuss this type of equipment here. Sprinklers attempt to simulate the process of rainfall, while allowing a control over the amount, intensity and drop size of applications in a manner that is not possible with natural rainfall. Though they represent rainfall/runoff conditions more realistically than ring infiltrometers and portable knapsack sprayers, sprinkle infiltrometers also have limitations. An important question to ask when considering sprinkler design is which of the main rainfall characteristics should be simulated most closely: for example drop-size or terminal velocity? Variations in intensity or uniform applications? Difficulties exist even in measuring the characteristics of the natural rainfall that is being simulated.

Design

Sprinkler design can be esoteric, and "production models" are not common. Most

sprinklers cannot be considered at all portable, they are too large and even small sprinklers may need compressors, pumps or a mains water supply to operate. Large, boom type sprinklers are usually confined to agricultural research stations where there is a plentiful supply of water and where it is preferable to maintain tight control over the soil, slope, cover and tillage variables under investigation. As runoff and soil loss are related to rainfall kinetic energy per unit area, this is a useful parameter by which to make comparisons of sprinklers. In general two types of rainfall simulations are adopted.

The first type are those using nozzles which most easily reproduce a drop size distribution akin to natural rainfall, but which have complex intensity reducing systems.

The second use drop-formers and are simpler in construction, but the drops do not reach terminal velocities until falling 5 m or more.

It is far beyond the scope of this book to describe the many individual types of sprinklers that have been developed, often for particular research purposes, and which are not easily nor commercially available. A comparative list of such equipment is given in Part 1 of monograph no. 9 of the American Society of Agronomy and Soil Science Society of America (1986). Most sprinklers of the "portable" kind apply water to relatively small areas, usually about 1 m². The problem of spatial variability of soil characteristics is therefore often as great with these devices as it is with ring infiltrometers and knapsack sprayers, and the extrapolation of results beyond the locality of application needs careful consideration.

The Type F infiltrometer has been used in the USA on larger plots of approximately 2 m × 4 m in size and is not regarded as portable. the type FA operates over a smaller area. The manner of operation to obtain infiltration data is recommended as follows:

- First, several calibration runs are undertaken with the test area covered by a waterproof sheet, to measure the simulated rainfall application rate.**
- A test run is then started with the sheet removed and continued until the rate of runoff becomes constant (as does the rate of infiltration).**
- The analytical run is started when application ceases and runoff stops, but before any recovery of the infiltration capacity has occurred. The rate of infiltration is therefore constant throughout the run. Runoff is measured. Any difference between the application minus infiltration and runoff is due to depression storage and detention storage.**
- The effects of these on the runoff process may be investigated for various land conditions, if required.**

The relation between runoff and infiltration data is discussed further in chapter 8, Data Analysis

Manufacture

The most important components of sprinklers are the nozzles that control the characteristics of the water that is applied to simulate rainfall. The testing of suitable nozzles on an individual basis was undertaken by the Ministry of

Agriculture, Harare, Zimbabwe as part of research into the SLEMSA soil erosion model (see chapter 3), over 1 m² areas. Of the various nozzles tested, several performed well and details are given in the Ministry's Research Bulletin no 25 (1980). The construction details of a mobile sprinkler taken from this bulletin are given Appendix C.

Equipment costs

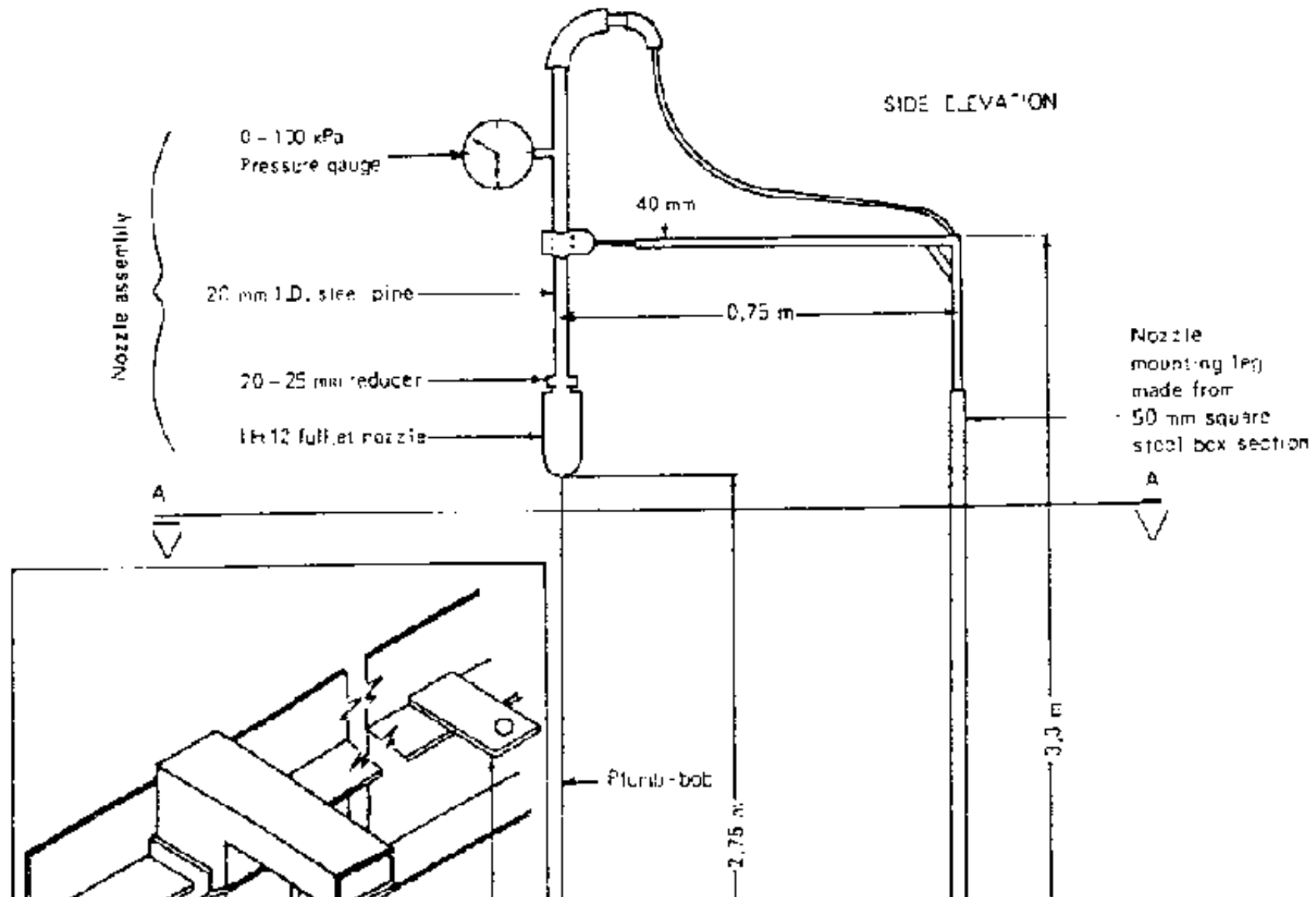
All costs of locally made equipment are approximate. The costs of raw materials and especially labour are highly variable from country to country, but a good idea of cost magnitude can be gained from the figures quoted below. The costs of manufactured equipment are based on 1993 prices. Shipping, agents' fees and fluctuations in exchange rate cannot be taken into account.

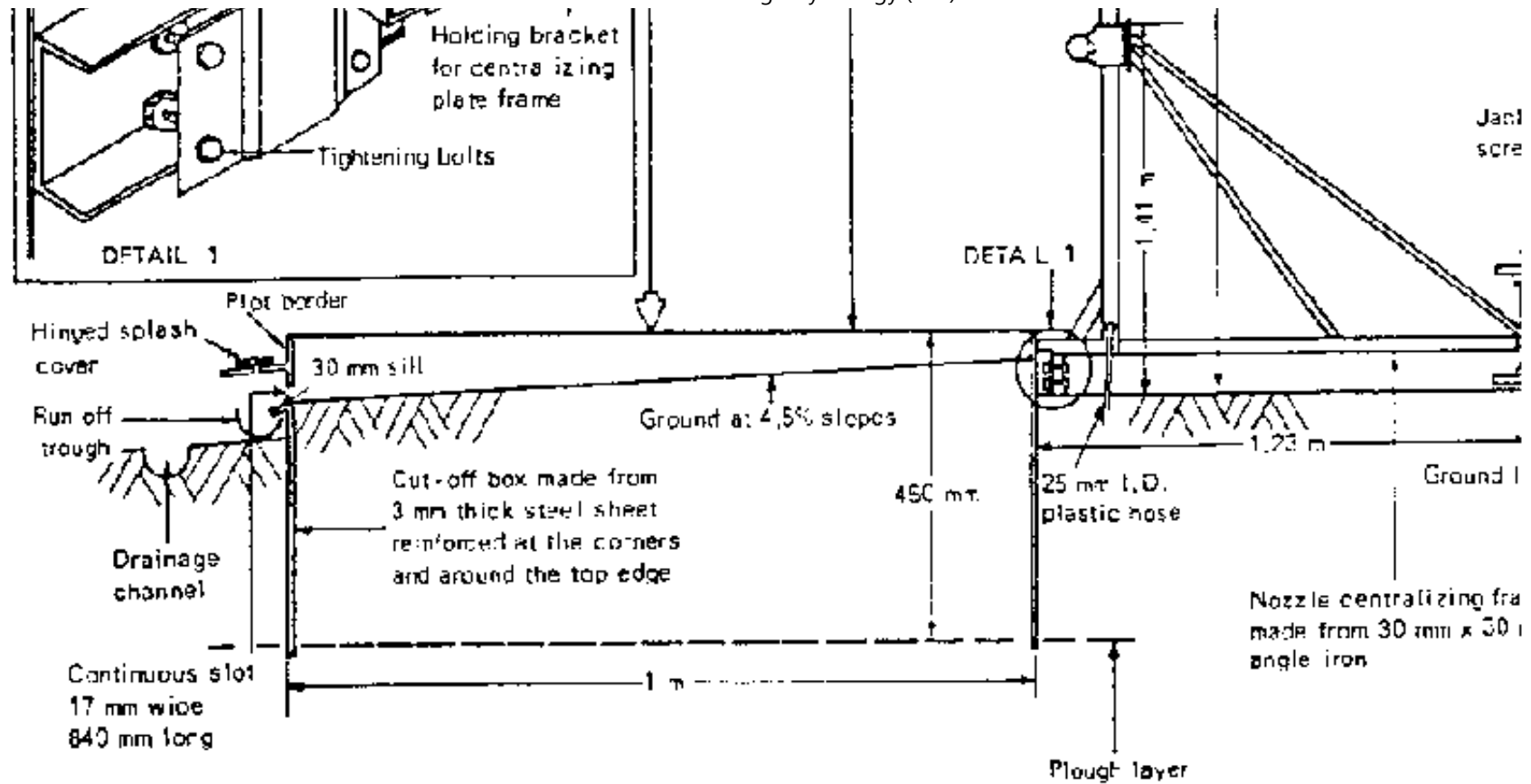
Item	Quantity	Typical	Approximate cost in \$ US
Locally made Equipment			
Set Infiltration rings	f		10 - 20
Simulators / Sprinklers			Too varied to list
Manufactured Equipment			
Set infiltration rings	1		200 -300
Neutron probe with ratescaler etc.	1		8000 - 9000
Access tubes	10 m		80 - 100
Mechanical installation equipment	1		not available
Manual installation equipment			
augers	1		
50 mm Dutch head	1		50 - 100
100 mm head	1		50 - 100
rods and handle	1		30 - 60
Soil sampling set with auger and sample rings	1		1000 - 2000
Tensiometers	1		100 - 200
Manometer type	1		highly variable
Dial gauge type	1		not available
Gypsum blocks	1		"
Nylon/fibreglass blocks	1		"
Multiplex TDR system, complete	1		10000 - 15000
Knapsack sprayer	1		200 - 300
15 bar pressure plate apparatus	1		3500 - 5000
High-low pressure manifold	1		1500 - 2000
Electrical compressor	1		4000 - 6000

Table

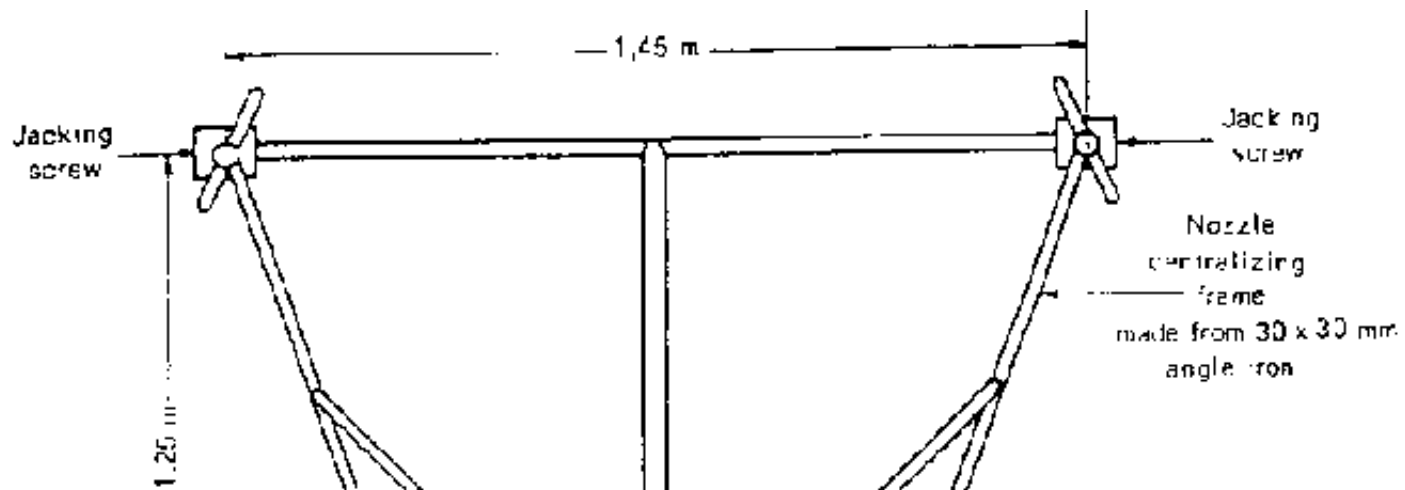
Appendix C: Soils and soil moisture

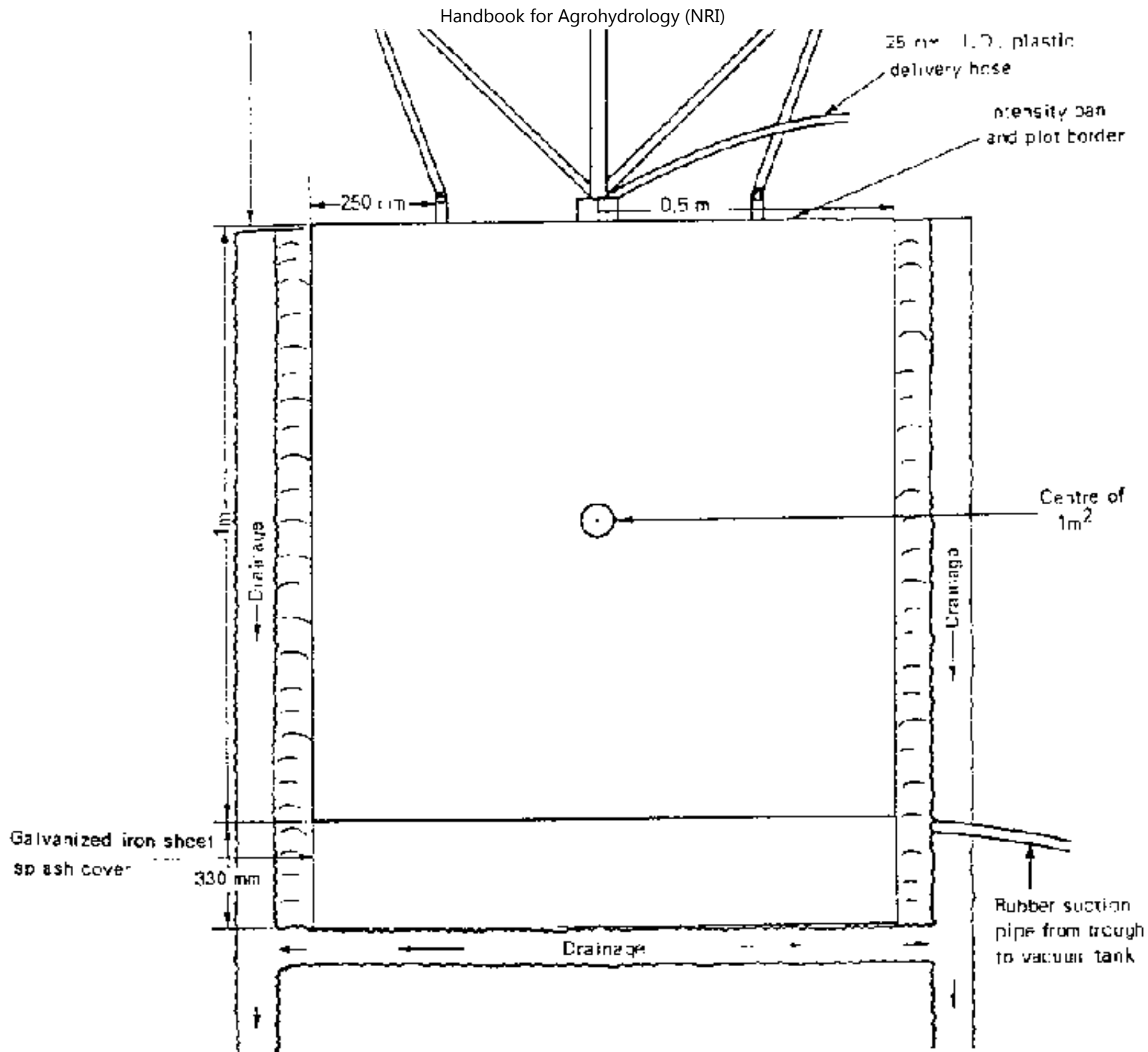
Appendix C1: Construction details of a mobile sprinkler system



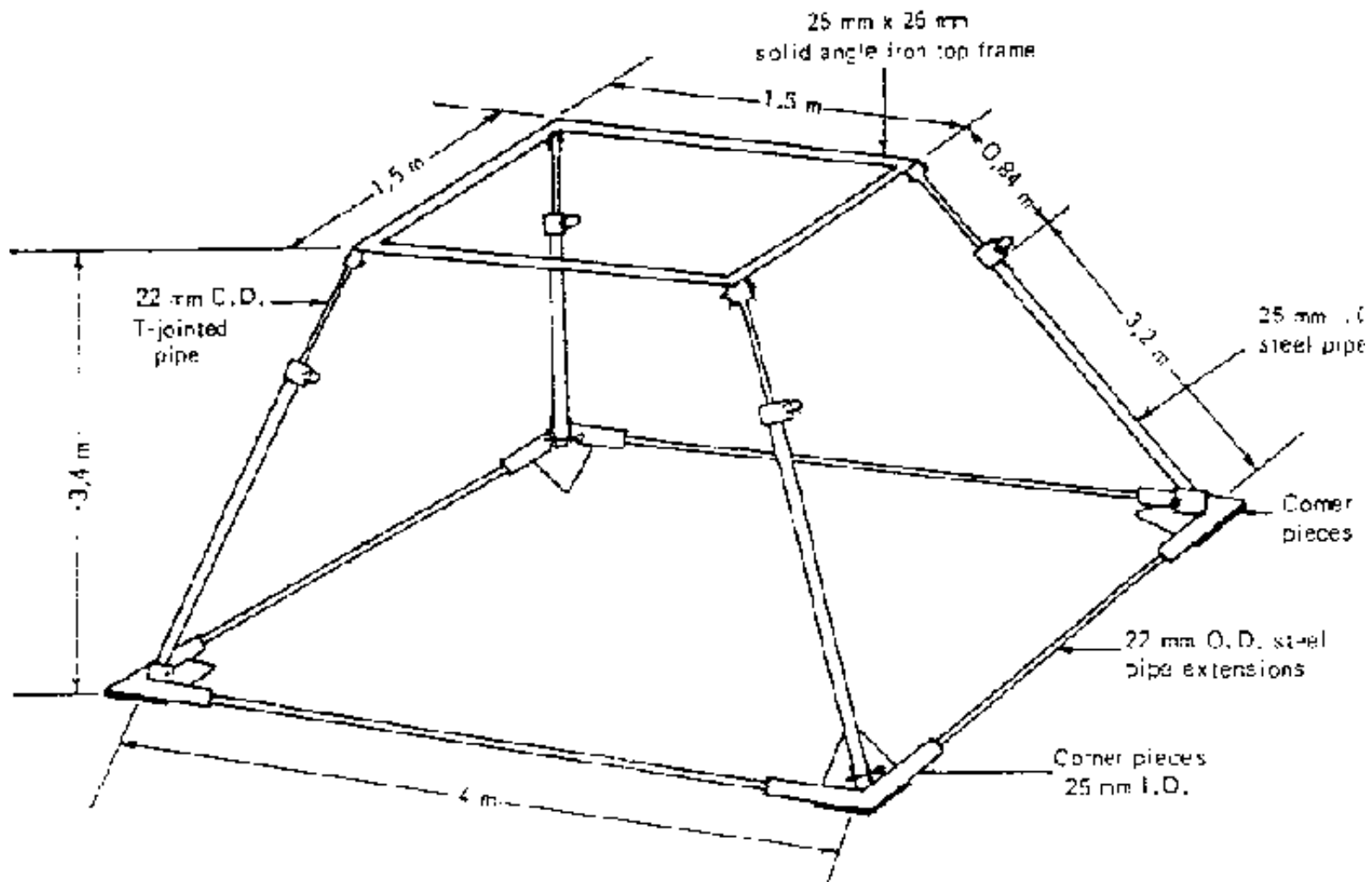


Figure

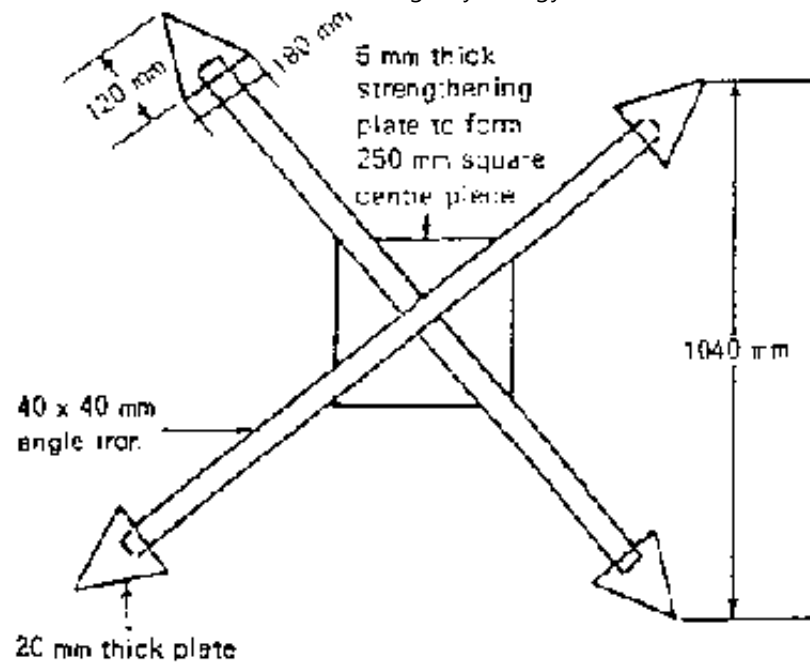




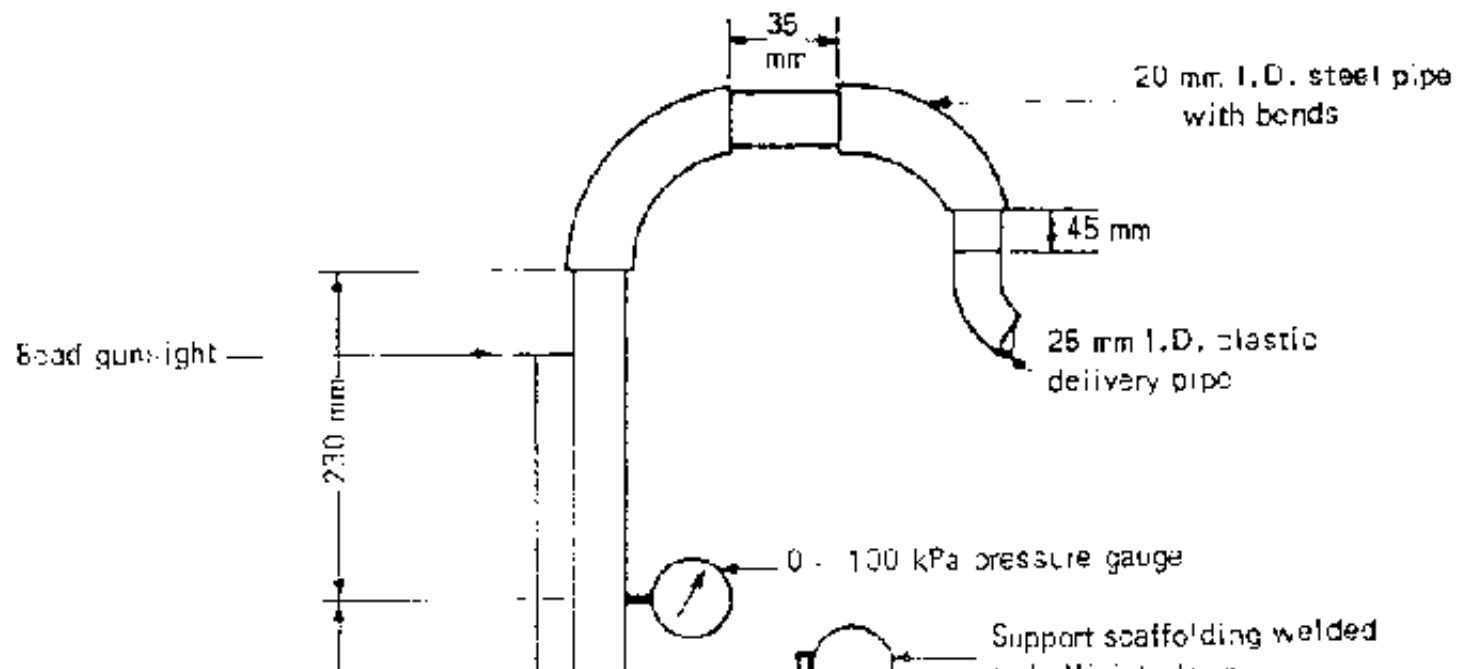
Plan on A - A



Windshield frame



Plan on driving dolly



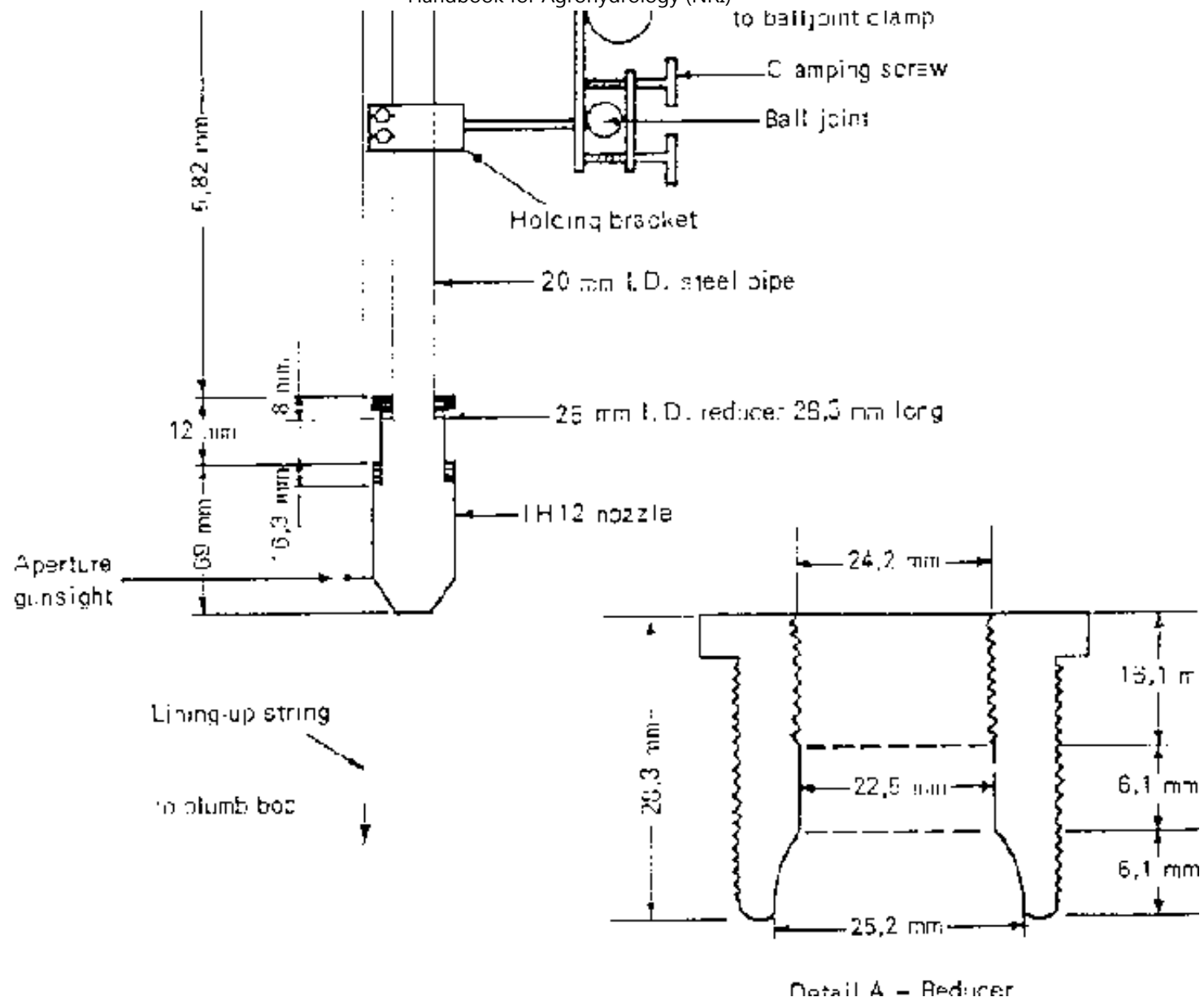
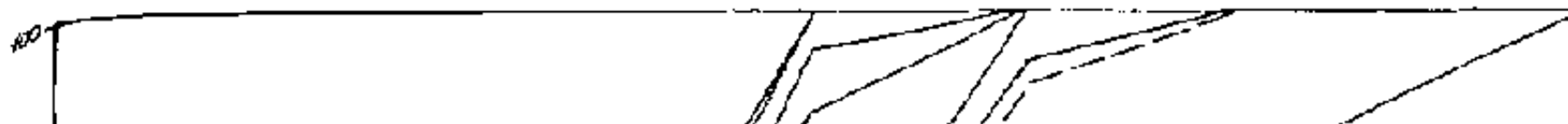
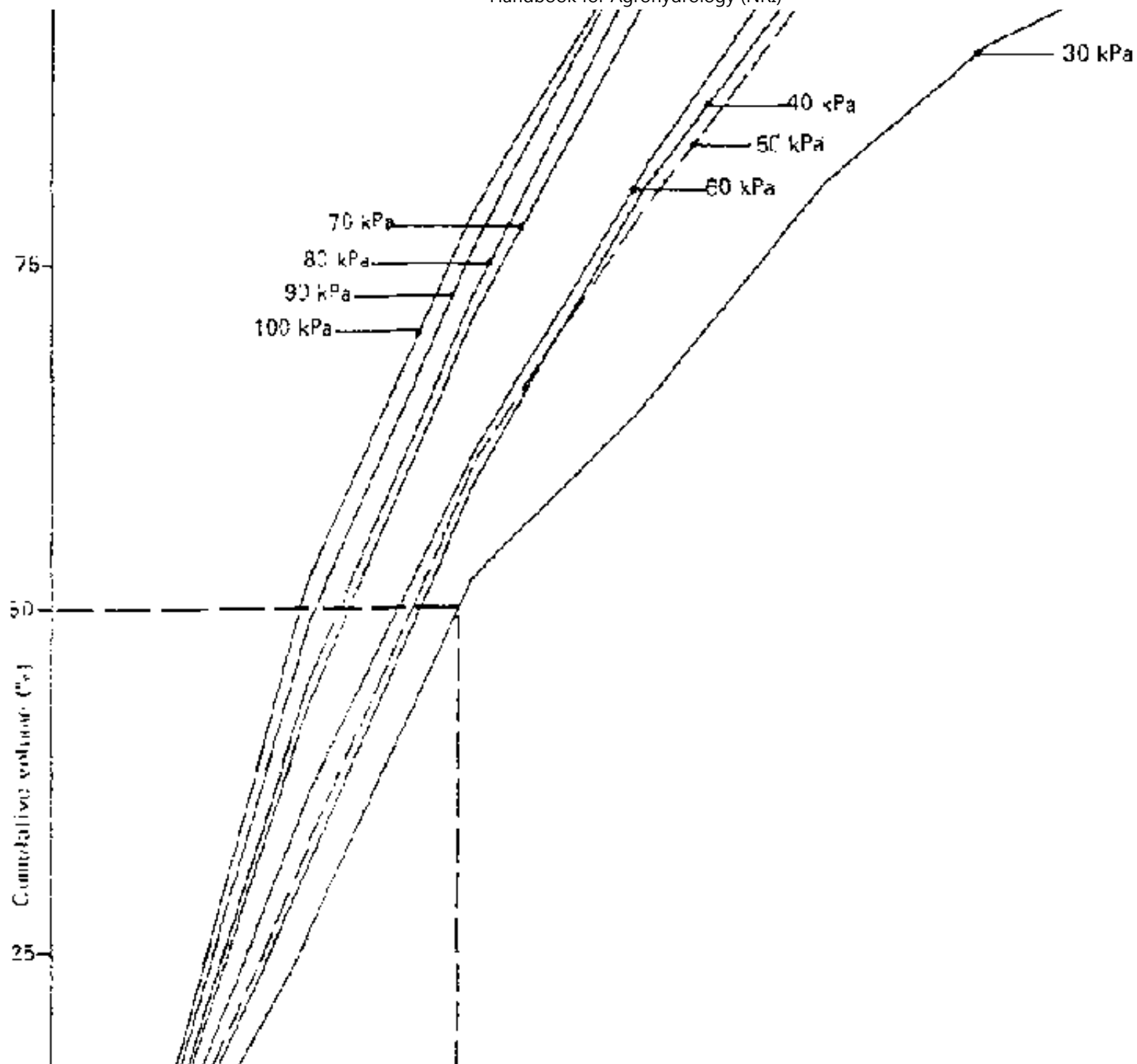


Fig. 1: IH12 nozzle assembly details





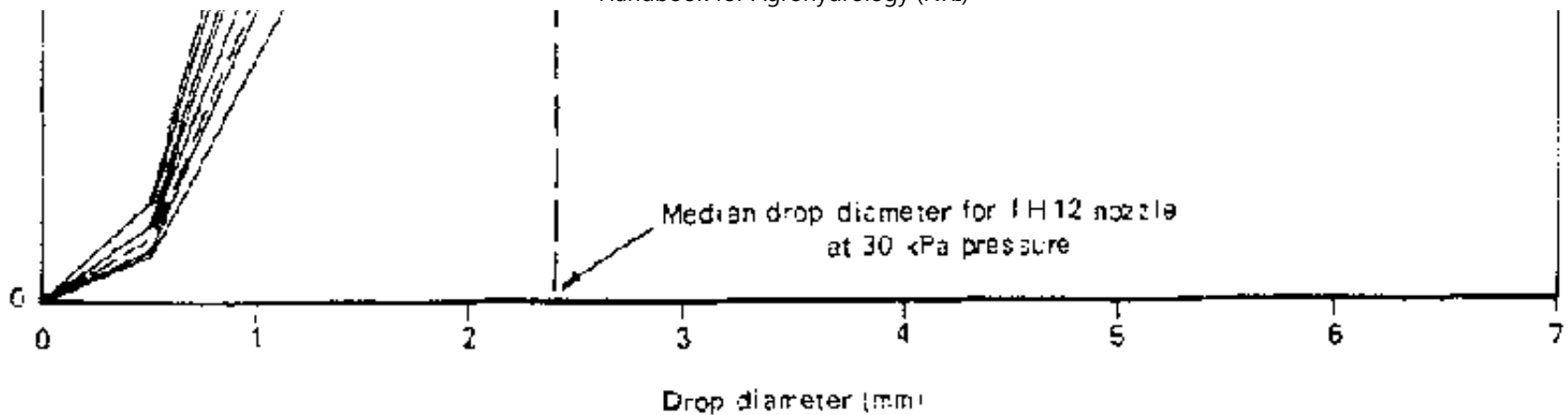
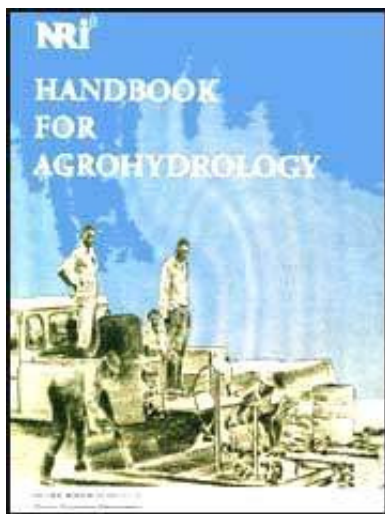


Fig. 2: Size distributions for IH12 nozzle

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Handbook for Agrohydrology (NRI)

Chapter 6: Catchment characteristics

(introduction...)

6.1 Natural vegetation

6.2 Interception

6.3 Catchment size, slope and topography

6.4 Field orientation

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Handbook for Agrohydrology (NRI)

Chapter 6: Catchment characteristics

Catchment characteristics interact with variable patterns of rainfall and determine the character and size of runoff volumes and peak flows. This is true for both natural catchments where human activity is absent or unimportant and runoff plots upon which tillage or other agricultural treatments are being tested. Generally, there is a hierarchy of influence imposed by different characteristics, but this hierarchy is often difficult to sort out and understand. For example, where a catchment has high slopes and lime vegetation, slope will play a major role in determining the runoff regime. This regime would tend to exhibit high runoff proportions; rapidly increasing flows to high peaks and equally rapid falls. Were this catchment to be of a more linear form and were its vegetation cover to increase, then peak flows would be smaller but more prolonged and total runoff volumes would probably be less. Were the slope lower, runoff would probably be less.

In the case of catchments with low slopes, the effects of vegetation cover and microtopographic features often exert a stronger influence over runoff than the overall land slope. Local slopes are often relatively high and they may direct runoff either into basins where it can infiltrate or to channels by which it can easily leave the catchment. Heavy textured soils tend to give a higher proportion of runoff. Soil textures are related to slope as well as to parent material, and the climatic regime under which the soil formed will often have been a determining factor of the soil textural type. Where human interventions have been imposed the natural

conditions of a catchment may have been altered radically; grazing, tree-felling and clearance are obvious examples. Agricultural techniques; ploughing, bunds and microcatchments are introduced to reduce runoff and usually they do, but the removal of natural vegetation and badly managed systems can have the opposite effect. Below, the main catchment characteristics and their influence on runoff are discussed.

6.1 Natural vegetation

Natural vegetation can be very important in determining runoff amounts; in many instances it is the most important influence of all, after rainfall. Areas bare of vegetation can lose more than 40% of seasonal rainfall through runoff and for intense, individual storms the loss can be much greater. Areas with dense grass cover and tree canopy cover can retain as much as 99% of the rainfall that reaches the ground. Vegetation reduces the energy of raindrops making them less erosive and intercepts rainfall which is then re-evaporated. Thus natural vegetation works against the occurrence of runoff in several ways. The same can be said of crops, but most crops provide only temporary cover and their densities, especially at ground level, rarely attain that of natural vegetation. Examples of increased runoff, soil erosion and subsequent land degradation due to the removal of natural vegetation, are common throughout the world and the literature. Consider the data presented below in Table 6.1, which compares runoff from different rangeland catchments of the same size catchments, but with various densities of vegetation cover.

Catchment No	Season 1		Season 2		Season 3		Average of Available Data	
	Cover (%)	Runoff (%)	Cover (%)	Runoff (%)	Cover (%)	Runoff (%)	Cover (%)	Runoff (%)
1	67	16	64	15	75	17	69	16
2	59	22	44	11	50	27	51	20
3	56	24	71	5	68	25	65	18
4	80	1	65	2	75	15	73	6
5	85	4	na	1	retired			
6	99	3	92	1	na	na	96	2
7	89	2	90	0.1	93	7	91	3
8	72	na	74	0.6	75	6	74	2
9	76	1	82	0.1	retired		79	1
10	94	0	93	0	retired		94	0

Table 6.1: Comparison of End of Season Vegetation Cover and Seasonal Runoff

Note that no account is taken of other factors that influence runoff production and that the coefficient of correlation between runoff amount and vegetation cover is 0.91.

6.1.1 Measuring Vegetation Cover

Plant biomass represents the total quantity of vegetation over a given area at any time and may be variable both within and between seasons. It might be expected that the quantification of biomass is the best indicator of vegetational influences on runoff. However it is probably not the most practical index for runoff studies, because the quantification of biomass is extremely time-consuming; a large number of samples must be taken and mapped in detail and size/mass relations must be determined by the destructive sampling of trees. Generally, the assessment of total plant biomass is unlikely to be relevant to agrohydrological and water harvesting projects.

The form of vegetation; leave-shape, density, branching pattern, etc., is highly variable between species and groups of plants. Although these differences are implicit within the classification of plant species, their effect on rainfall/runoff relations are very difficult to quantify. Research into commonly-occurring trees (and crops) has been undertaken, but the results of this work is understandably limited in its applications. Moreover, biomass and vegetation cover are usually very closely correlated and the use of vegetation cover as a proxy for biomass in runoff analysis, is a legitimate substitution.

Figure 6.1 show example correlations between biomass and vegetation cover and vegetation cover and runoff.

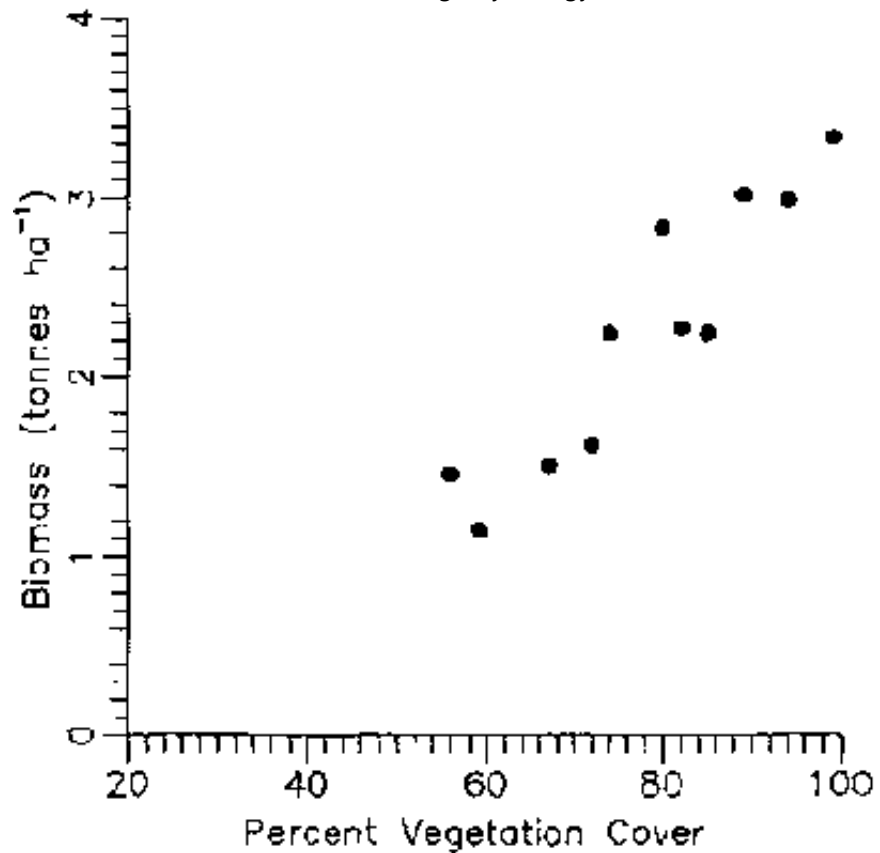


Figure 6.1: Biomass and Vegetation Cover

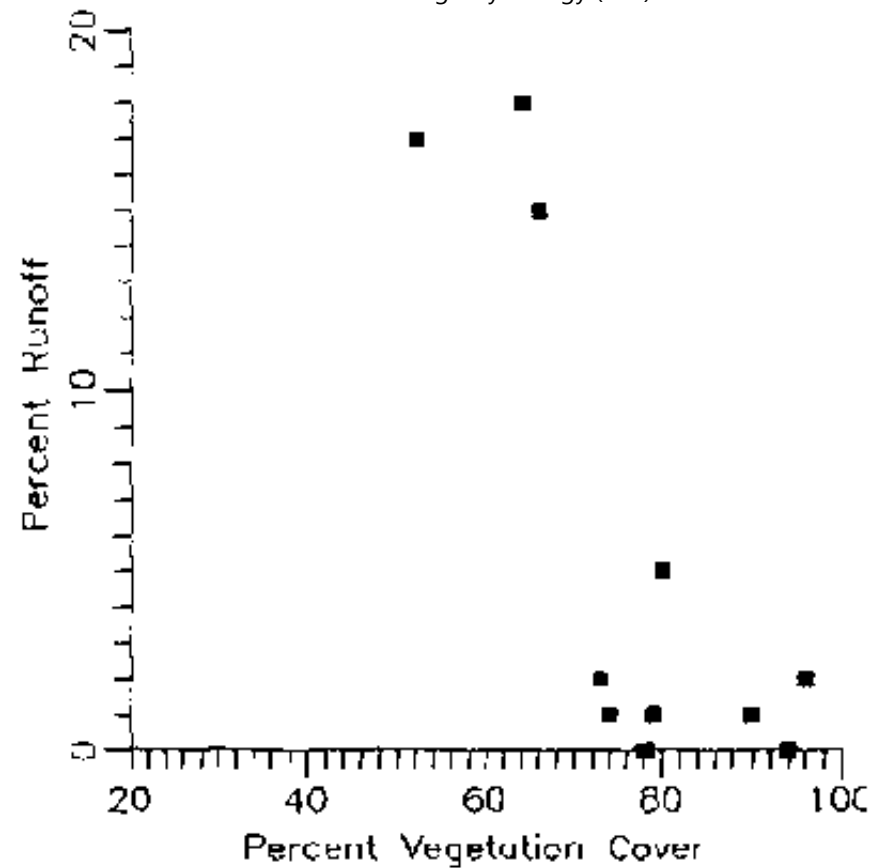


Figure 6.1: Vegetation cover and Runoff

In contrast to biomass measurement, there are rapid methods of quantifying the areal extent of total vegetation cover and even though effects due to vegetation type are not always accounted for, this index makes a good indicator of the influence that vegetation can have on runoff.

Vegetation cover assessments may be undertaken on a frequent basis to study its effect on runoff, almost storm by storm. Alternatively, assessments may be made only a few times each season, to understand its role in the production of runoff over longer periods. The latter case is most common, because the variation in

influence of vegetation cover is not dramatic in the short term, except where wholesale removal is involved. Vegetation cover is not closely correlated to other factors that influence runoff (except perhaps seasonal changes in rainfall and temperature), but may considerably alter the soil moisture status by evapotranspiration. The relative independence of vegetation from other variables makes it a suitable factor for use in regression analysis. On the other hand because it does vary with time, unlike factors such as slope, soil type and catchment size, vegetation cover can provide an extensive range of data points for individual catchments. It lends itself well to and is often used in, runoff modelling. In general it has been recognised that the amount of vegetation cover present is a more influential factor than the type of cover. The pattern of spatial distribution of vegetation cover may also be very important.

For agrohydrological purposes, there are two conventional methods of cover measurement, though the collection of aerial photography and satellite remote sensing data are also discussed below.

a. Quadrats.

A quadrat is a defined area. For field purposes, quadrats are usually permanent sampling areas retained throughout the season, within which the extent of vegetation cover is assessed. Prior to the field visit that will install them, a suitable number of quadrats is decided upon and these are placed on a site map using a fixed grid pattern. The quadrats are laid out in this predetermined, regular manner to overcome subjective bias and attain a random sampling of the area. No strict percentage sampling is required, though the more quadrats, generally the better. Ten 2m × 2m quadrats to sample 1 ha (10,000 m²) would be adequate. The

quadrats are then subdivided into four sectors to facilitate accurate assessment. The quadrats can be made easily by using steel rods driven into the ground with perimeters defined by nylon rope or string. Each quarter of the quadrat is individually assessed by eye for percent total ground cover of live and dead vegetation. The overall estimate of cover is made as an average of all sectors and all quadrats. In cases of natural vegetation, any trees are included in the assessment. Assessments should be made as frequently as is feasible throughout the season, though the rate of growth will largely determine the need for inspection. The estimates are to some extent subjective and it is a good idea to compare those made by different field staff, under the same conditions of cover.

This method is suitable for small areas and can be completed quickly, but projects that need to quantify cover accurately on large plots and small catchments can utilise a rapid method that is detailed below.

b. Wheel Point Method

This method is based on the simple equipment shown in Figure 6.2.

A bicycle handlebar is fitted with an extended fork assembly. The extension is made long enough to allow the passage between them of strong, sturdy spokes. The spokes, made of 5 - 10 mm diameter mild steel, are welded to a supporting plate which in turn is fixed to the axle. The forks can be any convenient length, but it is advantageous that when the observed, marked fork completes a revolution, it travels an easily recorded horizontal distance, for example 1.0 or 1.5 m. Versions with longer or shorter spokes can be made, according to whether the areas to be covered are small or large, to maintain a sufficient number of data points per unit

area of catchment.

As an example, consider a plot 100 × 40 m in extent. A tape measure is stretched across one of the longer sides of the plot, 5m from the end. The apparatus is held with the marked spoke at the start of the tape and then walked along using the tape as a direction indicator.

If the marked spoke hits a bare area on touching the ground, this is called out. If it touches a vegetated area on the ground this (and if required the type and species of plant) is called out. A second person notes the call. The tape is then moved on 10 m and the process is repeated, until the whole plot has been covered, the last transect being 5 m from the other end of the plot.

This procedure gives approximately 600-700 data points for each hectare that is surveyed and takes about one hour. Less frequent sampling by using more-widely spaced transects is permissible in areas where the vegetation cover is relatively uniform. This method is also easily adapted for larger catchments and can be used in difficult and wooded terrain, with practice.

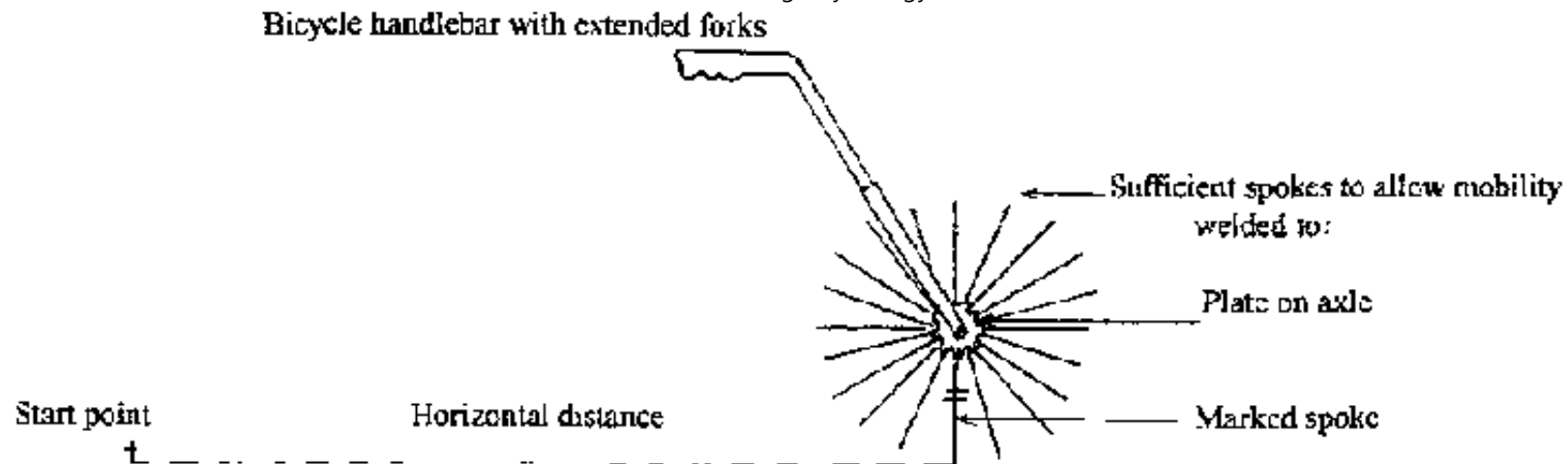


Figure 6.2: Wheel Point Apparatus

c. Aerial Photography

Aerial photographs and, more recently, satellite imagery can play an important role in the assessment of many aspects of agrohydrology, vegetation cover being one of them. Clearly, this method is inappropriate for small runoff plots, but for natural catchments it can be very useful. Large areas can be viewed quickly and catchments that are otherwise difficult to survey on the ground (those with dense tree cover, or that are inaccessible) can often be mapped much more effectively and cheaply. Additional information on surface flow routes, areas of flooding, land use, microtopography and agricultural features can also be obtained at the same time. The simplest methods of obtaining and using aerial photographs are discussed here.

Aerial photographs are used to compile maps and are often available from local survey departments. The main advantage of this is that once obtained, no further effort is needed before assessment can begin. There are, however, some serious

drawbacks:

- **Aerial photographs are often restricted material in many parts of the world and you may be refused them.**
- **When available, they are often at a scale of 1:50,000 or smaller. This is often unsuitable for detailed mapping.**
- **Enlargements can be made, at conventional scales, for example 1 :10,000 or 1:5,000. These are much more useful, but facilities for enlargement may not be available.**
- **They are almost always in black and white panchromatic format, which is poorly suited to vegetation studies. - In areas with marked seasonal differences, they will almost certainly be taken during the dry season when conditions for photography are best, but little information is available on vegetation or crop cover.**
- **Photographs for mapping purposes are not taken frequently and different sets of photos may be decades apart, ground conditions may have changed radically since they were obtained.**

If suitable orthodox photographs can be obtained, fine, but it is well worth considering obtaining your own. This is much simpler and cheaper than may be expected and has several advantages:

- **Photographs can be obtained at the most useful scales. The use of slides allows a range of scales to be obtained.**

- Colour or infra-red photographs can be obtained (though the latter film may be difficult to buy and have processed).**
- If slides rather than prints are taken, these are very useful for projection, mapping and conversion to prints.**
- They can be taken at critical times during the season.**
- Particular sites or areas can be selected.**

It is unlikely that the precision of scale and lack of distortion of map survey photographs can be equalled, but in most cases these are of minor importance compared to the advantages listed above. The general conditions to obtain good quality photographs economically are as follows.

Any light aeroplane (2-3 seat) can be used. Enquire if a glass panel can be easily inserted into the floor to give a vertical view or if this modification has been made previously. If not, a door will have to be removed and a wind shield fitted (this is not unusual, but vertical photographs will be more difficult to take). Plan the most economical route to all sites and submit a flight plan to be discussed with the pilot. As a guide, three sites situated within a 50 km radius of the airport can be covered in little more than one hour.

A good 35 mm single lens reflex camera (through-the-lens viewing is essential) is adequate. The type of lens is a point of preference and the aims of the photography will play an important part in the choice, because although the focal length of the lens will determine photographic scale and is technically important, the ease of use in the confined space of the cabin, the ability to work rapidly and

the need for different scales may be paramount.

A 70 - 210 mm focal length zoom lens will probably be suitable for most occasions since it gives approximately $\times 1.4$ to $\times 4.0$ magnification. This flexibility of magnification means there is no need to change the lens to cover different sized areas efficiently. Unless very small areas are to be studied, a 35 - 150 mm zoom would also be suitable and in this case slightly wide angle views can also be obtained. Another advantage that zoom lenses have is that their magnifications obviate the need for the aircraft to change altitude.

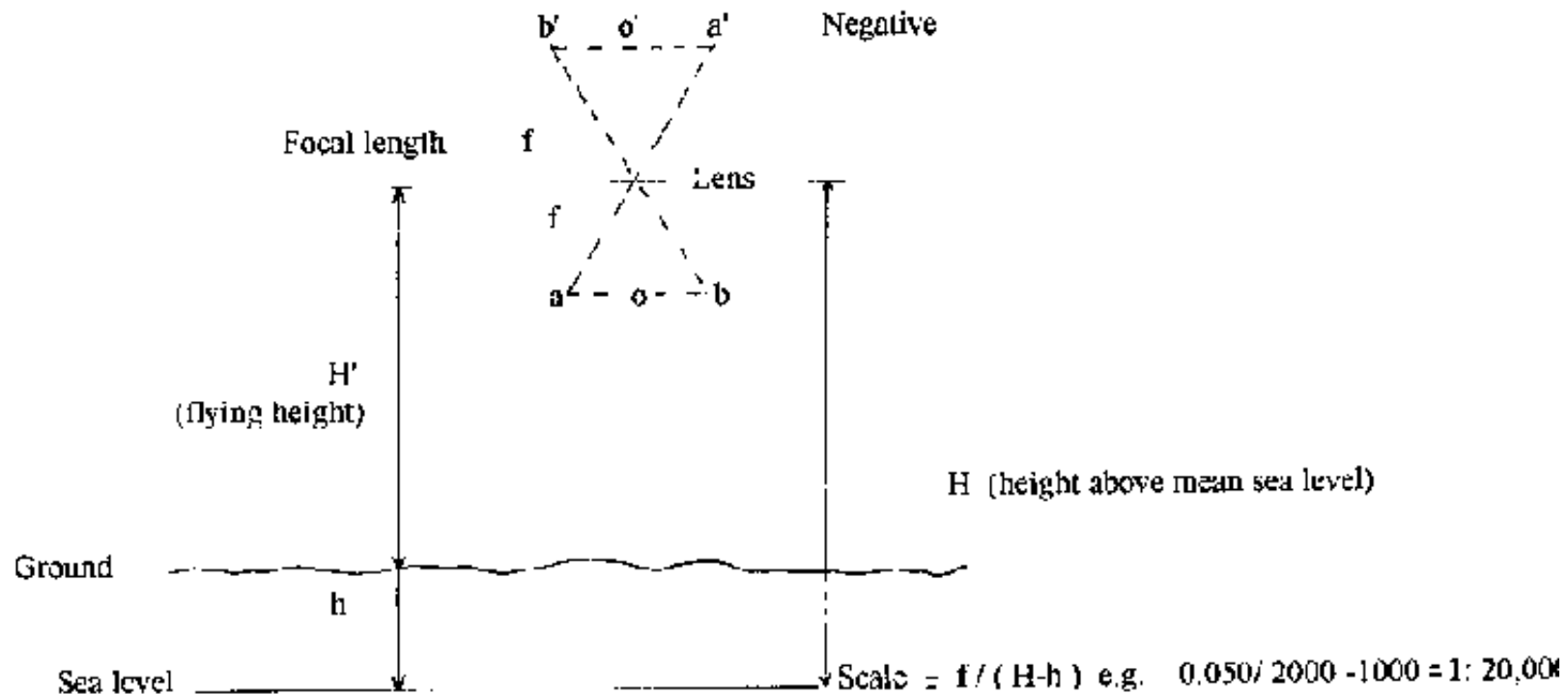


Figure 6.3: Scale of Vertical Photograph Over Flat Terrain

In a small plane this can take a long time and can add considerably to the cost

when several sites are being photographed at different scales. It is important to remember, however, that with zoom lenses, the exact focal length currently in use may be unknown and the scale of the photograph cannot be calculated, unless ground reference points of known dimensions are available. Sometimes it is best to preview the area to be photographed and tape the focal length of the lens in a fixed position with adhesive tape. It is not necessary thereafter to be continually manipulating the lens and the tape prevents it from accidentally sliding out of position when held vertically downwards.

Lenses of single focal lengths overcome these problems, but time must be allowed to change them and only a limited range can be used. As sunny conditions will undoubtedly prevail, film and shutter speeds are not usually a problem. A large depth of field is not needed, so wide aperture stops can be used to give high shutter speeds. To prevent blurring due to vibration, 1/500 th or 1/1000 th of a second exposures are recommended. Fast films (ASA 400 and above) should not be necessary and may not be available nor be easily developed. They tend to be grainy when enlarged.

Films should be at hand and clearly marked with date and location. Ground location markers may be necessary for site identification. At 2,000 - 3,000 feet (650 - 1000 m) above ground level, a good operating altitude for light aircraft, strips of white paper about 30 cm wide and 10 - 20 meters long are clearly visible. If they are set to known lengths, they make good ground reference markers for obtaining scales.

Photographs taken over terrain of widely varying altitudes exhibit varying scales and tilted photographs have nonuniform scales.

Table 6.2 below gives a guide to ground coverage with various altitudes and focal lengths . This is the actual area on the ground that will be captured by a 35 mm negative or slide diapositive of size 25 mm × 36 mm.

f	25 mm	28 mm	35 mm	50 mm	70 mm	100 mm	135 mm	200 mm
H' in (m)								
500	500 m x 720 m	438 m x 645 m	358 m x 515 m	250 m x 360 m	179 m x 258 m	125 m x 180 m	93 m x 133 m	63 m x 90 m
750	750 m x 1080 m	670 m x 965 m	535 m x 770 m	375 m x 540 m	375 m x 540 m	188 m x 270 m	140 m x 202 m	94 m x 135 m
1,000	1000 m x 1440 m	893 m x 1285 m	715 m x 1030 m	500 m x 720 m	358 m x 515 m	250 m x 360 m	185 m x 266 m	125 m x 180 m
1,250	1250 m x 1800 m	1115 m x 1605 m	893 m x 1285 m	625 m x 900 m	447 m x 643 m	313 m x 450 m	234 m x 335 m	157 m x 225 m
1,500	1500 m x 2160 m	1340 m x 1930 m	1073 m x 1544 m	750 m x 1080 m	670 m x 972 m	375 m x 540 m	278 m x 400 m	188 m x 270 m
2,000	2000 m x 2880 m	1785 m x 2570 m	1428 m x 2056 m	1000 m x 1440 m	714 m x 1028 m	500 m x 720 m	365 m x 526 m	250 m x 360 m

Table 6.2: Ground Cover Area for Different Altitudes and Focal Lengths

The largest area covered in the table above is between five and six square kilometres. This is the size of a small catchment, but details on the ground are not easy to see.

A mosaic of photographs, or a continuous transect of frames that cover a large area but which also show fine detail are possible, but not easy to obtain. Transects

can be planned on maps and air speeds calculated so that photographs may be taken at counted time intervals, without taking account of the view below. In practice, pilots find it difficult to keep a straight course with only a visual marker on the horizon and airspeeds vary due to wind. Drifting causes further problems. To some extent trial and error must play a part, but care and acute observation must be exercised to obtain reasonable coverage using transect flight paths.

d. Satellite Remote Sensing

During the last two decades or so, satellite imagery has become more widely used for water resource projects, among others. The importance of such imagery cannot be overstated, but the area of satellite image analysis is a very complex one and can only be covered here, very briefly.

The three main factors that dictate the usefulness of satellite imagery to a project are:

Orbital parameters

These define the potential repeat period for the coverage of an area. For example the polar orbiting NOAA satellites can obtain imagery at least once per day per satellite. The Landsat satellites have a repeat period of about two and a half weeks. The altitudes of various satellites are also greatly different and will affect ground resolution and size of coverage.

Sensors:

Satellites, their orbits and sensors are designed for particular purposes. For

example, Landsat satellites were designed for terrestrial research, Seasat for oceanographic study and Metsat for meteorological investigation. Different sensors are used to give the best results within a particular environment and may have restricted use outside that environment. Visible, infra-red, near infra-red and micro-wave (radar) sensors are commonly used, each of which is most suited to a particular application.

Resolution:

The size of an object that can be detected from a satellite, depends upon the resolution of the sensor, this may vary from a few metres, or even less, to several kilometres. It will also depend on the kind of sensor that is deployed and the spectral characteristics, shape and surroundings of the object that is viewed. In general, the area of coverage is smallest when resolution is finest, but in all cases coverage is "regional".

Imagery comes in two formats; hard (usually photographic) copy and computer compatible tapes (CCTs). The former may be colour (a combination of bands) or black and white (single band) and is relatively cheap and easy to work with. It will be purchased in a form that has been geometrically corrected for changes in satellite velocity, altitude, attitude and for Earth rotation and curvature. CCTs must be viewed using special computer facilities, desk-top versions of which are now widely available. These images can be extensively processed and enhanced and are the source of hard copy images. They and the equipment to process them are usually very expensive, though research institutions can in some cases, gain the image material for no, or little, cost.

Vegetation cover assessment is commonly undertaken using satellite imagery and the physical characteristics of catchments, their soil moisture status and hydrology can also be studied. However, the selection of satellite, imagery and waveband; the selection and utilisation of techniques for analysis is extremely complex and specialist literature should be consulted.

6.2 Interception

Interception can only be loosely defined as a catchment characteristic as it is the combined effect of several influential factors such as rainfall, climate and vegetation cover. However, in other respects it falls conveniently into this chapter and so is discussed here.

Losses from interception, the rainfall that collects on vegetation and is re-evaporated, can be highly variable and depends mostly on vegetation type (size, shape and disposition of leaves and branches); rainfall amount, intensity and drop size; wind speed, temperature and eddying. Interception is difficult to measure, especially for crops. It can be attempted by placing rain gauges under vegetation either randomly to sample average interception, or by the selection of specific target areas. In wooded catchments, rain gauges should be attached to tree trunks to assess stem flow, as in Figure 6.6 below, but with multi-stemmed vegetation this is very difficult.

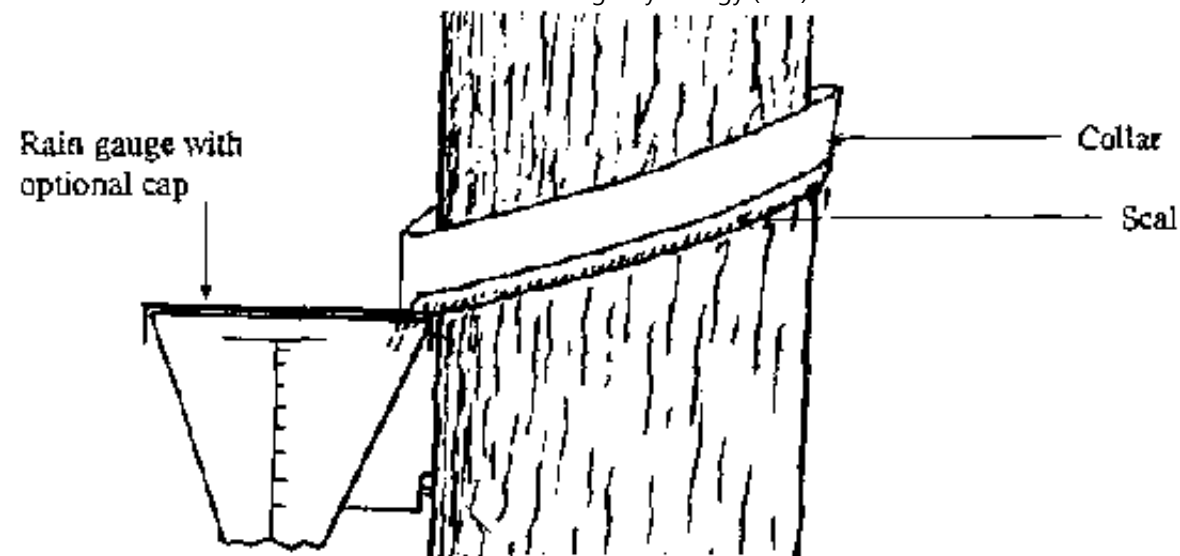


Figure 6.6: Stemflow Measurement on trees

In Figure 6.6, in addition to free-standing gauges under the canopy, a peripheral collector is wrapped around the trunk to direct flow into a single rain gauge that is covered.

Empirical work has led to estimates of losses of 10 - 20% of seasonal rainfall and deduced storage capacities of 0.8 to 1.5 mm of rain per storm. Equation (6.1) describes an empirical interception relation and Table 6.3 gives examples for various crops for a 25 mm rainfall.

$$I = (S_i + Et_r) (1 - e^{-kP}) \text{ where (6.1)}$$

I = total interception

S_i = storage capacity per unit of the area

E = evaporation rate

t_r = duration of rainfall

P = amount of rain $k = 1/ (S_i + Et_r)$

e = base of natural logs.

In terms of runoff studies, the situation regarding interception is even more complex. It is usually lumped with rainfall storage due to ponding and infiltration for runoff modelling purposes, where it is assigned a purely notional value.

Table 6.3 Interception Losses from a 25 mm Rainfall

Crop	Height (m)	Interception (mm)
Maize	1.8	0.8
Cotton	1.2	8.4
Tobacco	1.2	1.8
Small grains	0.9	4.1
Meadow Grass	0.3	2.0
Alfalfa	0.3	2.8

6.3 Catchment size, slope and topography

6.3.1 Catchment Size and Land Slope

Catchment size is an important influence on absolute values of runoff amount and peak flows and is an essential parameter in runoff formulae that predict these hydrological characteristics. The determination of catchment size will be

straightforward in most cases. Runoff plots are usually bounded by bunds or galvanised metal sheets that prevent runoff from outside the proscribed catchment area. Natural catchments will usually be defined by clear patterns of drainage and topographies that show the limits of a catchment area. In some cases these details will be available from topographic maps, in others aerial photography may be the most suitable source of information. In general, the size of a catchment that is monitored will be limited by the practicalities of the natural or artificial controls that can be used as flow measuring sections, the aims of the project and the resources that can be invested in obtaining runoff data. Catchment size is not a good indicator of percent runoff; influences such as land use, soil type and slope are more important, but in terms of absolute values catchment size is very important. It is unfortunate that a simple proportional reduction or increase of runoff cannot be deduced from the size of a catchment, even where catchment conditions are ostensibly the same (see the section on slope and microtopography below). To illustrate the difficulties in making assumptions on runoff proportion and catchment size, Table 6.4 gives percent runoff for large catchments, $R^2 = 0.12$ and is not significant.

River catchment	Gauge site	Area (km ²)	Obs. period	Ay Annual Rainfall (mm)	Runoff (mm)	Runoff (%)
Mahalapshwe	Madiba	840	'70-82	518	13	2.5
Lotsane	Palapye	3815	'70-81	542	4	0.7
Kolobeng	Weir	120	'78-83	555	33	5.9
Metsmot/haba	Merwa Rd	3400	'76-83	460	3	0.7
Metsmot/haba	Tharuga	982	'77-83	465	6	1.3
Motloutse	Tobane	8400	'70-82	536	13	2.4
Notwane	Gaborone	3960	'60-82	545	8	1.5
Shashe	Dam	3630	'73-82	562	19	3.4
Tati	Weir	570	'63-80	503	52	10.3
Ntsho	Weir	800	'63-79	494	46	9.3
Shashe	Mooko	2500	'63-78	497	33	6.6
Shashe	Lower	7810	'71-81	555	26	4.9

Table 6.4: Relation Between Catchment Area and Runoff

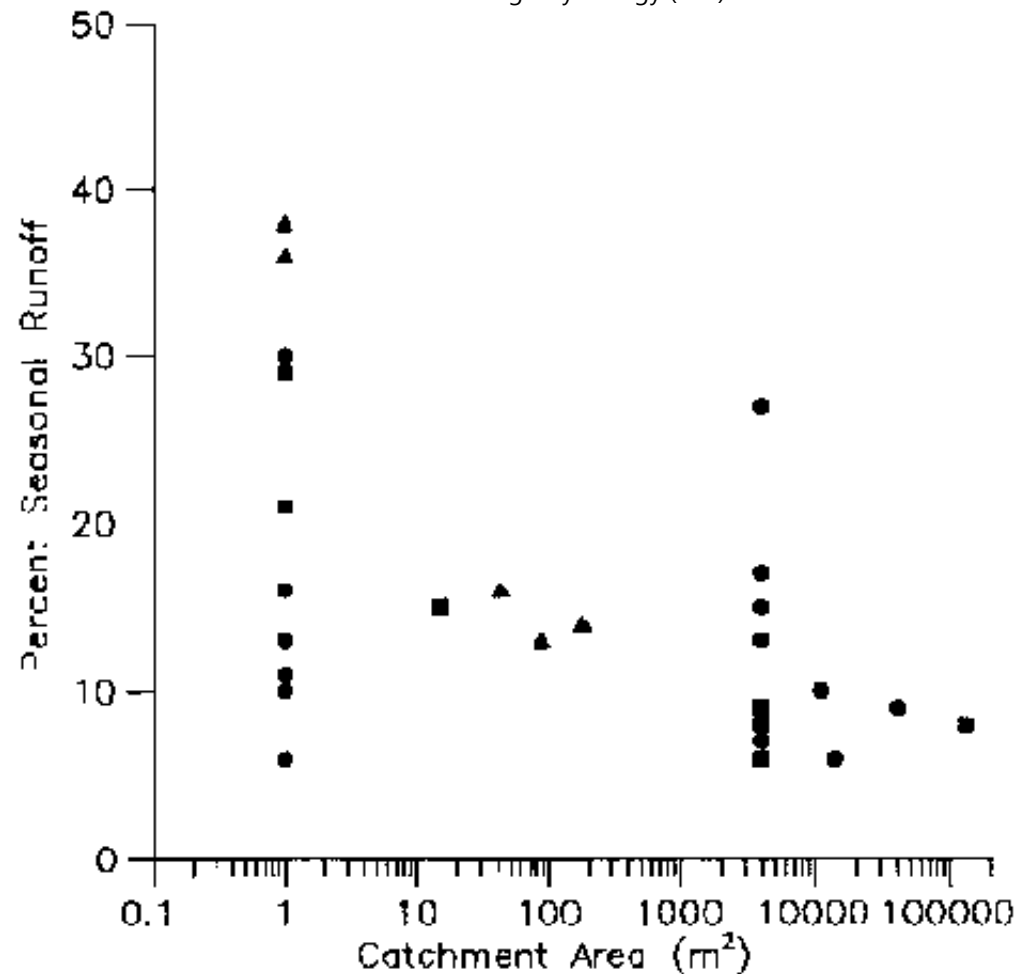


Figure 6.7: Catchment Size versus Runoff from Experimental Plots

The scale of these catchments is larger than is often studied for agrohydrological research, but Figure 6.7 shows a graph of catchment size versus percent runoff, the data for which were obtained from experimental plots and catchments sited in and around farmers' fields. These plots are divided into three groups with similar catchment conditions, to remove any influence that different conditions could exert on runoff. The conditions are crop (squares); rangeland (triangles) and

fallow (circles). The R² of the analyses were 0.108, 0.066 and 0.602 respectively and none of the relations were significant.

Suitable Catchment Sizes for Runoff Plots

a. Plots Representing Farmers' Field Conditions

In many cases, it is important to collect data on the actual losses of rainfall, as runoff, from farmers' fields. These data show whether such runoff is important and if so, provide the information to design preventative measures. Observations of runoff which do not involve actual measurement are notoriously misleading and anecdotal evidence to estimate runoff amounts should not be used. Runoff channels and other evidence do not provide accurate information on volumes and frequencies and no decisions should be made on the basis of their observation

It is important at the outset of runoff plot experimentation, to define the most appropriate size of plot. This size will depend on several factors, but the most important is that it should be representative of actual field conditions. The use of very small plots has several advantages; many replicates can be built, they are easy and cheap to instrument, and they occupy only a small portion of any research area. It is unlikely, however, that a plot that is only 20 square metres in extent, for example, can be used to represent the runoff regime of a farmer's field. The actual dimensions and shape of the any runoff plot are best determined by the aims of the research agenda, the finance and equipment that are available, the remoteness of the site etc., but it is essential that the following considerations be made:

- The plot should include representative field topography, so that within the plot, the overall land slope of the field should be included. Slopes influence the velocity of runoff and will affect opportunities for it to infiltrate and overwhelm ploughed ridges. Because runoff velocity increases by the square root of slope, small differences in slope between plots will not lead to large differences in runoff velocity or amount. Low overall land slopes greatly increase the storage capacity of ploughed ridges and bunds (see chapter 7 on water harvesting for details), thereby reducing the possibility of runoff.**

- Within the plot, the microtopography (the small-scale ups and downs and ploughed ridges and furrows) of the field should be included. This is especially important in flat areas where microtopographical features may have local slopes greatly in excess of the overall land slope and may be very important in inducing runoff. The redistribution of this local runoff (which may constitute net runoff from the field) will be determined by the size, pattern and distribution of microtopography. This can exist as basins and mounds or ridges and channels, the former could be expected to impede runoff, the latter to assist its passage to the field margins.**

- Ploughed ridges and furrows will inevitably leave the contour at some point and encourage water movement to low-lying areas. This should be taken into account when plots are being planned and runoff should not be impeded by the artificial boundaries of the plot.**

- Another important reason to include representative microtopography is its potential to indicate changes in soil texture and nutrient status.**

Differences in infiltration rates, water holding capacity, soil depth and soil chemical characteristics may be present, resulting in a local variation of runoff production and crop performance. The inclusion of microtopography within runoff plots will not only influence the physical processes of runoff, but will also allow agronomic sampling procedures to assess more accurately, the effect that these have on crops.

- It is important to note that although in land-levelled fields natural microtopography may not be evident, residual soil variability will still be present and may have an important influence on crop growth. Plots that are used to measure runoff from farmers' fields should cover at least 10% of the total area, more where fields are less than 5 ha in extent. A 30 cm H flume will have an adequate capacity to cope with flows from plots of around 0.5 to 1 hectare. Plot length should exceed 80 m where field-scale runoff is to be defined and plots should be representative of field slope and topographic conditions. They should be ploughed and planted according to the farmer's usual methods. Where similar plots are used to measure runoff from naturally vegetated areas, a representative cover should be included. Very bare plots of 0.5 hectare may be expected to give flows close to the capacity of a 30 cm H flume and a larger instrument may be preferred.

b. Within Field (Small-Scale) Runoff Plots

Plots built to estimate runoff on small-scale water harvesting and tillage schemes are much simpler than those built to represent farmers' field conditions. They are usually smaller in dimension than any microtopography that may be present.

In these instances, it is usually not difficult to place plots to measure runoff on any slope that is desired. Edge effects can be influential and it is important that boundaries do not channel runoff to the collection tank in an unrealistic manner. Rain falling directly into impermeable gutters, drains, etc. should be taken into account.

Runoff will exploit very small elevation differences and sheet flow is quickly converted into channel flow. If the aim of the experimentation is to promote the even redistribution of runoff to the crop rooting zone, this is an important fact to note.

Ploughed ridges and furrows play an important part in influencing runoff in these circumstances and dead furrows may be a consequence of ploughing technique. They can store a considerable amount of runoff (typically about 500 litres or 0.5 m³ per 10 m length) and their location can make a significant difference to runoff measurement, especially for small runoff events.

It should be noted that such small plots may not behave as on the research station if they are transferred and installed as extensive systems on farmers' field, where pronounced microtopography may exist. The importance of placing runoff plots in full knowledge of the effect of microtopography on runoff measurement cannot be overstated.

In the first case (location Figure 6.8) average seasonal percent runoff from the mounds was measured as 29.0 %, while the runoff from the crop plot (marked on Figure 6.8) and which measures 100 m × 40 m, was only 4.5 % on average, over three seasons. Slopes of the microtopography were about 5%, of the large plot

about 0.5%.

In the second case (location Figure 6.9), local runoff due to microtopography, from the ridges to the channels, was in excess of 15% whereas average runoff from four, 100 m × 40 m plots located on farmers' fields, but not shown in Figure 6.8, ranged from 1.7% to 4.5% over three seasons. Slopes from ridges to channels ranged from about 3-8%,

large plots slopes were approximately 1%. If, in such cases, the results of runoff measurement from the small plots were extrapolated to estimate net runoff values from the whole field, they would lead to a gross over-estimation.

In practical terms this over-estimation might lead to the supposition that the prevention of runoff was of paramount importance and costly (to the farmer in terms of labour input for reward from increased yields) control measures might be implemented. Where rainfall amounts are regarded as marginal for crop production, these results might also suggest that additional supplementary water should be obtained by water harvesting. The apparent runoff efficiencies of 15 - 29% indicate a high runoff efficiency, and it might be expected that an extra 100 - 125 mm per season could be provided on the basis of a 1:1 crop to water harvesting area ratio. The actual runoff efficiencies of around 2 - 4% for the larger plots show that this is not the case and 10 mm might represent the realistic supplement that would be available for crops (ratio 1:1), unless the harvesting to crop area ratio was very large.

Figure 6.10 shows a typical simple installation for the measurement of runoff from field microtopography.

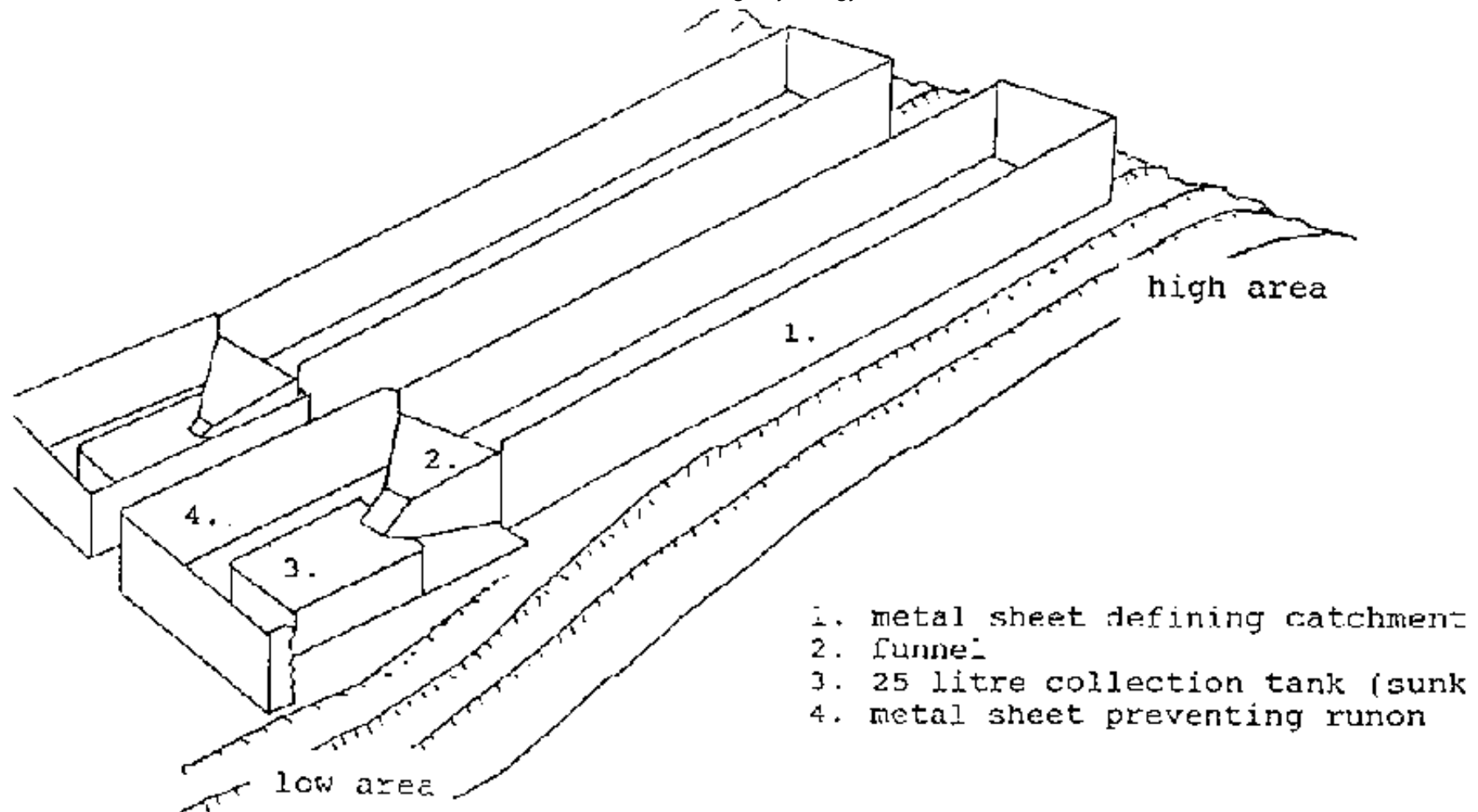


Figure 6.10: Installation to Measure Runoff from Field Microtopography

c. Natural Catchments

Natural catchments are usually larger than those that are artificially defined for the purposes of runoff measurement. They frequently include areas with different land slopes, soil textures, vegetation and microtopography. In areas with abrupt changes in geology, different densities of stream networks are often exhibited.

Natural catchments are, therefore, more difficult to characterise than artificially bounded catchments. For the purposes of study they may have to be divided into subcatchments each with a more homogeneous nature. Runoff may then be measured at locations to include each of these relatively homogeneous areas.

6.4 Field orientation

Field orientation is particularly important with regard to water conservation measures that may be attempted on agricultural land. Fields are often defined according to convenience, exploiting useful land marks such as the position of roads and access, rivers and natural features. They are rarely oriented with runoff losses and methods of runoff prevention in mind. Figure 6.11 shows a typical semi-arid agricultural landscape.

A study of the photograph and the features of drainage shows that most fields are oriented so that one corner is at the highest topographic elevation. Few of the boundaries are parallel or at right angles to the overall land slope and the natural drainage. In practical terms, this means that when a farmer ploughs, he or she will always plough such that ridges and furrows provide channels that encourage runoff. To plough along the contour would necessitate a start in the highest or lowest corner, and ploughing for very short distances. The length of ploughing would gradually increase until the full diagonal width of the field was attained, then the distances would decrease until the farmer eventually reached the opposite corner from where he or she had started. This would be a very difficult and inefficient exercise from the viewpoint of ploughing, but cultivation would be on the contour, disregarding local variations, and would inhibit the natural flow direction of runoff. If contour bunding were to be practiced, similar difficulties

would be encountered.

The boundaries set for fields, in areas of agricultural activity where the effects of urbanisation are small, are often those of roads and tracks. The directions of these roads and tracks are not often exactly along the contour. They remove natural vegetation, cross natural drainage systems, redirect runoff and concentrate it into ditches, under culverts and bridges. This leads to the disruption of the natural drainage, the concentration of flow and in many cases, serious problems of soil erosion.

The problems of field orientation are complex. They involve land ownership, the freedom of access and many other social issues, as well as a consideration of the physical environment and the behaviour of drainage. The field studies of most projects will be sited upon land that is already allocated and used for farming, so few opportunities for the implementation of new allocations will exist. However, the influence of field orientation is an important factor to note when field sites are being selected and where the opportunity exists, serious consideration should be given to the siting of new fields with a favourable aspect to natural drainage. The problems of contour cultivation and the effects of local microtopography on such practices are discussed in more detail in chapter 7, Water Harvesting.

6.5 Antecedent soil moisture conditions

Antecedent soil moisture conditions strongly influence the rate at which rainfall infiltrates into the soil and contribute to the processes of runoff production. Soil moisture levels at any time are the result of a combination of several factors, mainly: the time elapsed since the last rainfall; the rainfall amount and intensity;

the climatic conditions that have prevailed since rainfall; the type and stage of development of vegetation and soil texture and depth. Soil moisture levels can be highly variable both between and within periods of a particular meteorological activity. A high degree of spatial variability of soil moisture conditions may also be encountered.

Soil moisture levels can be estimated by accounting procedures that balance the infiltration of rainfall against losses by drainage and evapotranspiration. The calculation of evapotranspiration (Et) by different methods is discussed in Chapter 8. A commonly used accounting procedure derives an Antecedent Precipitation Index, by the application of an estimated factor for Et losses on previous rainfall. It is generally assumed that the rate of reduction of the soil moisture reserves is logarithmic, the rate falling as the availability of water decreases. The mechanisms by which antecedent soil moisture effects runoff are highly variable from soil to soil, but the general assumption equates higher proportions of runoff with higher levels of soil moisture. This reflects the behaviour of infiltration rates under increasingly moist conditions. Figure 6.12 illustrates changes in antecedent soil moisture according to rainfall.

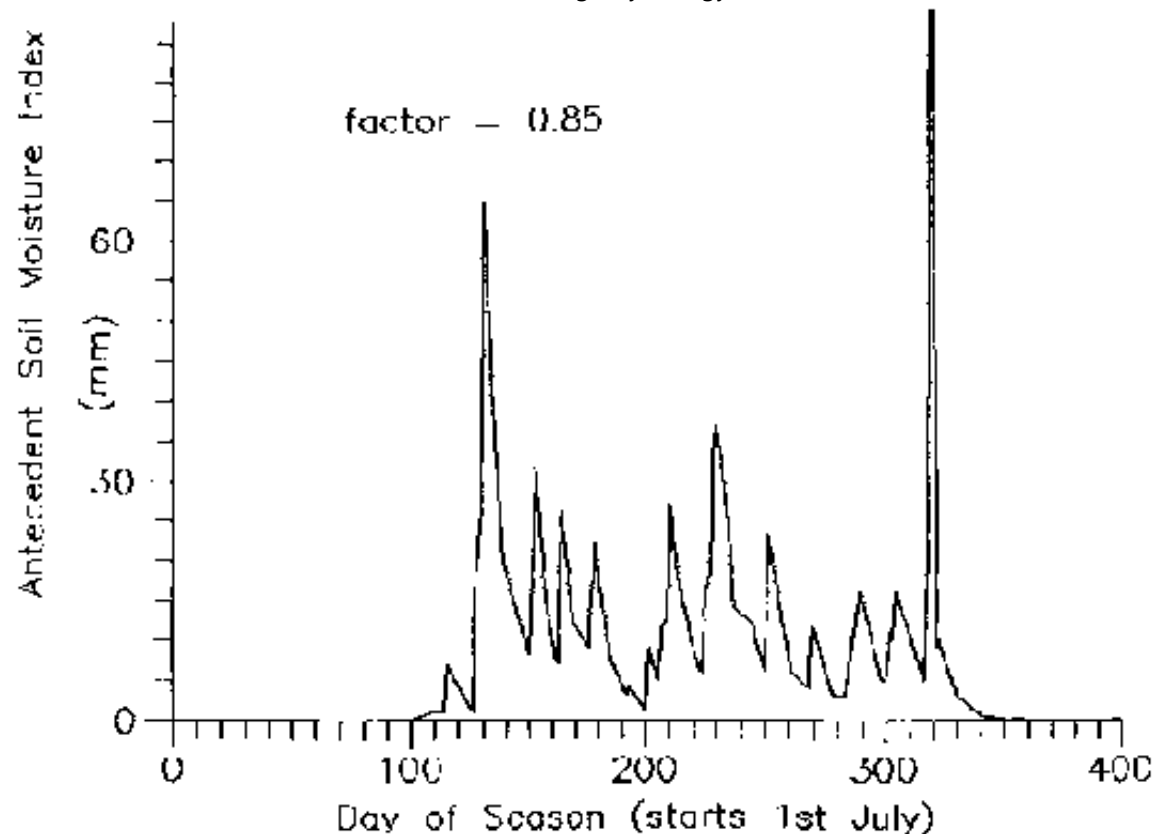


Figure 6.12: Change of Antecedent Soil Moisture Levels as shown by the Antecedent Precipitation index

It is important early in a study to determine the precision to which antecedent soil moisture needs to be measured or calculated. In situ measurements can be time-consuming and calculations of E_t usually necessitate the collection of a wide range of meteorological information (see Chapter 8). General indicators of soil moisture status may be adequate in some instances, but for use in, for example, regression analysis against event runoff data, estimates of actual values are necessary.

6.6 Other catchment influences

a. Geology

Among the other influences on runoff that can be important is the geological nature of the area under study. It affects runoff in three main ways:

1. Lithology. Particular rock types that are exposed at the surface of the ground can have a profound influence on runoff, but generally large areas of exposed rock are not common. Impermeable rock surfaces such as granite, gneisses, shales etc. can produce very high percentages of surface flow and may be locally important as sources of runoff. In semi-arid areas, and where these rocks are highly fractured, they not only lead to rapid runoff from their impermeable surfaces, but can also provide ground water that prolongs stream flow beyond the normally short period of flash-flooding. Permeable lithologies such as limestone and porous sandstone can limit surface flow to brief periods, only attained when ground water levels are extremely high. Perennial springs may occur where they overlie impermeable layers. According to its permeability, the geology of an area will determine surface drainage density, stream channel length and catchment shape.

2. Soils. The most widespread effect on runoff, of geology, is through the type of soil that it engenders. Granite, sandstones and quartzites produce sandy soils with relatively few nutrients and high rates of infiltration. Shales and basic igneous rocks usually give rise to relatively impermeable clays, though the humidity of the climate will determine the processes of weathering and erosion, and the type of soil that is subsequently formed.

3. Topography. Topography is also the result of geology and climate which determine land form, slopes and local microtopography.

b. Stream Density

Stream density is an index of the concentration of a drainage network within a catchment area. It will not be an important factor when small runoff plots are studied, but is used frequently as an independent variable in regression analysis for natural catchments. It should be noted that larger runoff plots, for instance those greater than about 0.5 hectare especially where they constitute a portion of a large field or a natural catchment, may well exhibit a stream network, though in arid and semi arid regions stream flow will be ephemeral. Such networks exhibit themselves as microtopography and may be influential in determining the runoff efficiency of the catchment, as they intercept sheet flow and channel it to the catchment outlet. The measurement of discharge by sheet flow alone is more difficult to achieve, as it is prone to retention by vegetation, ponding and subsequent infiltration. In origin, stream density is closely related to structural geology, Ethology, slope and climate. In general terms the greater the density of a stream network, the greater the percentage runoff for any given rainfall, because stream channels conduct runoff efficiently they lead to high, sharp peaks and rapid recessions. Relatively complicated systems of stream hierarchy are used to derive a stream density index in hydrological analysis, but they are somewhat beyond the scope of this book. A simple but appropriate index that can be used for regression analysis is length of stream per unit area (e.g. km km⁻²), though the correlation of such a stream index with other catchment characteristics must be considered before use in regression.

c. Human Factors and Agriculture

Human influences on runoff can be very great and they work at many scales. The

wholesale destruction of huge forested areas has led to disastrous flooding and extreme environmental degradation. A list of the most important influences that affect the amount of runoff from the land may include:

**Urbanisation
Deforestation
Overgrazing
Dam building
Canalisation
Reduction of flood plains
Draining of marshes and swamps**

Arable agriculture appears less harmful, perhaps, but its effect on the hydrological nature of much of the world's surface is profound.

Equipment costs

All costs of locally made equipment are approximate. The costs of raw materials and especially labour are highly variable from country to country, but a good idea of cost magnitude can be gained from the figures quoted below. The costs of manufactured equipment are based on 1993 prices. Shipping, agents' fees and fluctuations in exchange rate cannot be taken into account.

Item	Typical Approximate Cost in \$ US	
2 - 3 Seat light aircraft	per hour	300 - 500
4 - 5 Seat with global navigation	per hour	400 - 700
(sufficient to cover 2 -3 sites within about 30 km radius)		

Wheel point apparatus	1	30 - 60
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